# Mansions Family - Rockwall, Texas

Rainfall Intensity determined from attached graph given Time of Concentration (Tc)

Use the graphical hydrograph method to determine the volume of stormwater storage needed to compensate for increased runoff due to development.

Q = c\*I\*A

I. Determination of Allowable Release Rate - Existing Site

Total site area

Detained Time of Concentration (T<sub>c</sub>)

Detained runoff coefficient

Max. Allowable release rate

Area of site draining undetained

Runoff from undetained area

Total off-site area passed through

Time of Concentration (T<sub>c</sub>)

Off-site pass-through rate

Runoff coefficient

Rainfall intensity for 10 year storm (T<sub>c</sub>=20 min)

Area of site draining through detention pond

Rainfall intensity for 10 year storm (T<sub>c</sub>=10 min)

Rainfall intensity for 10 year storm (T<sub>c</sub>=15 min)

Total Allowable release rate from detention pond

II. Required Storage Calculations, Return Period = 10 years

Runoff coefficient for developed conditions

Undetained Time of Concentration (T<sub>c</sub>)

or Duration and the Return Period of the storm.

where c = 0.35, I = 5.80 in./hr.

Use c = 0.75 for muli-family development

Use the Rational Method to determine maximum rate of runoff Use the Rational Method to determine maximum rate of runoff

c = Runoff Coefficient I = Rainfall Intensity (in/hr) A = Drainage Area (acres)

Rainfall Intensity determined from attached graph given Time of Concentration (Tc) or Duration and the Return Period of the storm.

Use the graphical hydrograph method to determine the volume of stormwater storage

needed to compensate for increased runoff due to development.

Graphical Hydrograph Method for Stormwater Detention

Mansions Family - Rockwall, Texas

2 Year Detention Calculations

Allowable release rate is based upon 20 minute, 2 year storm event for undeveloped flows For Existing Conditions:

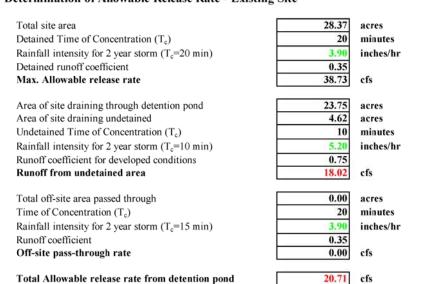
For Proposed Conditions: Use c = 0.75 for muli-family development

## I. Determination of Allowable Release Rate - Existing Site

II. Required Storage Calculations, Return Period = 2 years

Q = c\*I\*A

Purpose:



## **Graphical Hydrograph Method for Stormwater Detention**

Where: c = Runoff Coefficient

inches/hr

Allowable release rate is based upon 20 minute, 10 year storm event for undeveloped flows

I = Rainfall Intensity (in/hr)

A = Drainage Area (acres)

#### **Graphical Hydrograph Method for Stormwater Detention** Mansions Family - Rockwall, Texas 10 Year Detention Calculations 25 Year Detention Calculations Use the graphical hydrograph method to determine the volume of stormwater storage

needed to compensate for increased runoff due to development.

Use the Rational Method to determine maximum rate of runoff c = Runoff Coefficient I = Rainfall Intensity (in/hr)

A = Drainage Area (acres) Rainfall Intensity determined from attached graph given Time of Concentration (Tc)

or Duration and the Return Period of the storm. Allowable release rate is based upon 20 minute, 25 year storm event for undeveloped flows where c = 0.35, I = 6.75 in./hr.

Use c = 0.75 for muli-family development

#### I. Determination of Allowable Release Rate - Existing Site Total site area Detained Time of Concentration (T<sub>c</sub>) Rainfall intensity for 25 year storm (T<sub>c</sub>=20 min) inches/hr Detained runoff coefficient Max. Allowable release rate Area of site draining through detention pond Area of site draining undetained Undetained Time of Concentration (T<sub>c</sub>) Rainfall intensity for 25 year storm (T<sub>c</sub>=10 min) inches/hr Runoff coefficient for developed conditions Runoff from undetained area Total off-site area passed through Time of Concentration (T<sub>c</sub>) Rainfall intensity for 25 year storm (T<sub>c</sub>=15 min) inches/hr

## II. Required Storage Calculations, Return Period = 25 years

Total Allowable release rate from detention pond

#### Graphical Hydrograph Method for Stormwater Detention Mansions Family - Rockwall, Texas

50 Year Detention Calculations Use the graphical hydrograph method to determine the volume of stormwater storage

needed to compensate for increased runoff due to development. Use the Rational Method to determine maximum rate of runoff

c = Runoff Coefficient I = Rainfall Intensity (in/hr) A = Drainage Area (acres)

Rainfall Intensity determined from attached graph given Time of Concentration (Tc) or Duration and the Return Period of the storm.

Allowable release rate is based upon 20 minute, 50 year storm event for undeveloped flows where c = 0.35, I = 7.40 in./hr.

Use c = 0.75 for muli-family development

Total site area	28.37
Detained Time of Concentration (T <sub>c</sub> )	20
Rainfall intensity for 50 year storm (T <sub>c</sub> =20 min)	7.40
Detained runoff coefficient	0.35
Max. Allowable release rate	73.48
Area of site draining through detention pond	23.75
Area of site draining undetained	4.62
Undetained Time of Concentration (T <sub>c</sub> )	10
Rainfall intensity for 50 year storm (T <sub>c</sub> =10 min)	9.00
Runoff coefficient for developed conditions	0.75
Runoff from undetained area	31.19
Total off-site area passed through	0.00
Time of Concentration (T <sub>c</sub> )	20
Rainfall intensity for 50 year storm (T <sub>c</sub> =15 min)	7.40
Runoff coefficient	0.35
Off-site pass-through rate	0.00

#### Actual Metered release rate from detention pond II. Required Storage Calculations, Return Period = 50 years

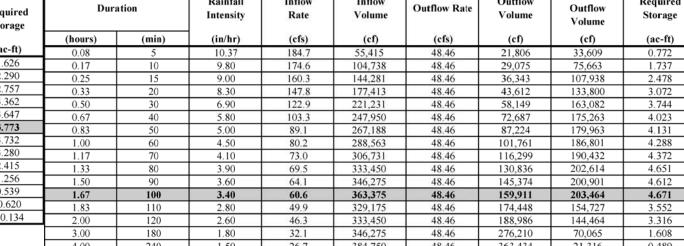
Total Allowable release rate from detention pond

Detained Time of Concent	ration (T <sub>c</sub> )		20	minutes	
Rainfall intensity for 100 y	ear storm (T <sub>c</sub> =20 r	nin)	8.30	inches/hr	
Detained runoff coefficient			0.35		
Max. Allowable release r	ate		82.41	cfs	
Area of site draining throu	gh detention pond		23.75	acres	
Area of site draining under	ained		4.62	acres	
Undetained Time of Conce	entration (T <sub>c</sub> )		10	minutes	
Rainfall intensity for 100 y	ear storm (T <sub>c</sub> =5 m	in)	9.80	inches/hr	
Runoff coefficient for deve	loped conditions		0.75		
Runoff from undetained a	area		33.96	cfs	
Total off-site area passed the	hrough		0.00	acres	
Time of Concentration (Tc	)		20	minutes	
Rainfall intensity for 100 y	ear storm (T <sub>c</sub> =20 r	nin)	8.30	inches/hr	
Runoff coefficient			0.35		
Off-site pass-through rate	e		0.00	cfs	
Total Allowable release r	ate from detention	n pond	48.46	cfs	
II. Required Storage Calc	ulations, Retur	n Period = 10	0 years		
			991;		
	T . C				0.40

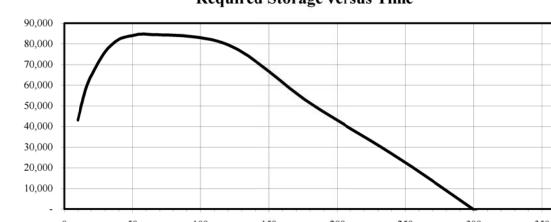
Pariship							T		I									ı							ı			1								г
	Dura	ation				Outflow Rate	1			Dura	tion				Outflow Rate				Dur	ation				Outflow Rate			21041111011	Dui	ration	1 1			Outflow Rate		Outflow	Required Storage
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	(hours)	(min)	(in/hr)	(cfs)	(cf)	(cfs)	(cf)	(cf)	(ac-ft)	(hours)	(min)	(in/hr)	(cfs)	(cf)	(cfs)	(cf)	(cf)	(ac-ft)	(hours)	(min)	(in/hr)	(cfs)	(cf)	(cfs)	(cf)	(cf)	(ac-ft)	(1, )	(		(-8-)	2.6	(-6)	(.6	volume	( 6)
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	0.17	10	5.20	92.6	55,575	20.71	12,424	43,151	0.991	0.17	10	7.20	128.3	76,950	32.64	19,586	57,364	1.317	0.17	10	8.20	146.1	87,638	38.61	23,167	64,471	1.480	(nours)	(min)	(In/nr)	(cis)	(61)	(cis)	(61)	(61)	(ac-it)
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	0.25	15	4.50	80.2	72,141	20.71	15,530	56,610	1.300	0.25	15	6.50	115.8	104,203	32.64	24,482	79,721	1.830	0.25	15	7.40	131.8	118,631	38.61	28,958	89,673	2.059	0.17	10	9.00	160.3	96,188	42.29	25,376	,	1.020
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	0.33	20	3.90	69.5	83,363	20.71	18,636	64,726	1.486	0.33	20	5.80	103.3	123,975	32.64	29,379	94,596	2.172	0.33	20	6.75	120.2	144,281	38.61	34,750	109,531	2.514	0.25	15	8.20	146.1	131,456	42.29	31,720	99,736	2.290
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	0.50	30	3.15	56.1	100,997	20.71	24,848	76,148	1.748	0.50	30	4.80	85.5	153,900	32.64	39,172	114,728	2.634	0.50	30	5.55	98.9	177,947	38.61	46,333	131,614	3.021	0.33	20	7.40	131.8	158,175	42.29	20,001	120,111	2.757
1.00 50 2.00 35.6 128,250 20.1 43,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 50 4.02 128,272 143,485 84,765 1946 1.00 60 3.50 62.3 187,185 128,272 143,485 84,765 1946 1.00 60 3.50 62.3 187,185 128,272 143,485 84,765 1946 1.00 60 3.50 62.3 187,185 128,272 143,485 84,765 1946 1.00 60 3.50 62.3 187,185 128,272 143,485 84,765 1946 1.00 60 3.50 62.3 187,185 128,272 143,485 84,765 1946 1.00 60 3.50 62.3 187,185 128,272 143,485 144,637 3.32 142,335 144,637 3.32 142,335 144,637 3.32 142,335 144,637 3.32 142,335 144,637 3.32 142,335 144,637 3.32 142,345 144,637 3.32 142,345 144,637 3.32 142,345 144,637 3.32 142,345 144,637 3.32 144,637 3.32 144,637 144,6	0.67	40	2.65	47.2	113,288	20.71	31,061	82,227	1.888	0.67	40	4.05	72.1	173,138	32.64	48,965	124,173	2.851	0.67	40	4.70	83.7	200.925	38.61	57.917	143.008	3.283	0.50	30	6.15	109.5	197,184	42.29	50,752	146,432	3.362
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	0.83	50	2.27	40.4	121,303	20.71	37,273	84,030	1.929	0.83	50	3.50	62.3	187,031	32.64	58,758	128,274	2,945	0.83	50	4.02	71.6	214.819	38.61	69,500	145.319	3.336	0.67	40	5.20	92.6	222,300	42.29	63,440	158,860	3.647
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1.00	60	2.00	35.6	128,250	20.71	43,485	84,765	1.946	1.00	60	3.00	53.4	192 375	32.64	68.551	123.824	2.843	1.00	60	3.52	62.7	225 720	38.61	81.083	- 10,000	3 320	0.83	50	4.50	80.2	240,469	42.29	76,128	164,341	3.773
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	2.00	120	1.25	22.3	160.313	20.71	,	79.555	1.826	2.00	120	1.80	32.1	230.850	32.64	127 308	103 542	2 377	2.00	120	2.20	39.2	282 150	38.61	150 583	131.567	3.020	1.00	60	3.92	69.8	251,370	42.29	88,816	162,554	3.732
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	3.00	180	0.88	15.7	169 290	20.71	118 030	51 260	1 177	3.00	180	1.00	24.9	269 325	32.64	186,066	83 259	1 911	3.00	180	1.65	20.4	317.419	38.61	150,505	07 335	2 235	2.00	120	2.40	42.8	307,800	42.29	164,944	142,856	3.280
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4.00	240	0.71	12.6	182 115	20.71	110,000	26.812	0.616	4.00	240	1.15	20.5	207,325	32.64	244 823	50.152	1.511	4.00	240	1.00	24.0	246 275	30.01	220,003	56,602	1.201	3.00	180	1.80	32.1	346,275	42.29	241,072	105,203	2.415
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5.00	300	0.71	10.7	102,115	20.71		20,012	-0.005		200	0.00	16.6	294,973	32.04	202 591	5 400	0.124	4.00	240	1.33	24.0	340,273	30.01	207,000	0.625	0.221	4.00	240	1.45	25.8	371.925	42.29	317,200	54,725	1.256
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	6.00	300	0.00	0.7	200,070	20.71	,	20.550	0.003	0.00	300	0.93	10.0	298,181	32.04	303,381	-5,400	-0.124	5.00	300	1.15	20.5	2003712	38.61	507,005	9,035	0.221	5.00	300	1.30	23.2	416.813	42.29	393.328	23.485	0.539
$lackbox{12.00} \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ $	0.00	300	0.02	9.3	200,070	20.71	152 101	-29,778	-0.684	0.00	300	0.80	14.3	507,000	32.64	362,338	-54,538	-1.252	6.00	360	0.96	17.1	369,360	38.61	,	-59,223	-1.360	6.00	360	1 15	20.5	442 463	42.29	469 456	-26 993	-0.620
	12.00	/20	0.30	5.3	230,850	20.71	453,484	-222,634	-5.111	12.00	720	0.47	8.4	361,665	32.64	714,884	-353,219	-8.109	12.00	720	0.55	9.8	423,225	38.61	845,584	-422,359	-9.696	12.00	720	0.63	11.2	194 785	12.29	926 223	-441.438	-10.134

Runoff coefficient

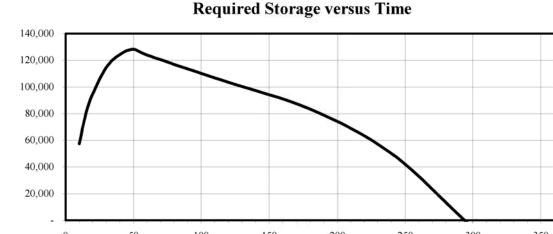
Off-site pass-through rate



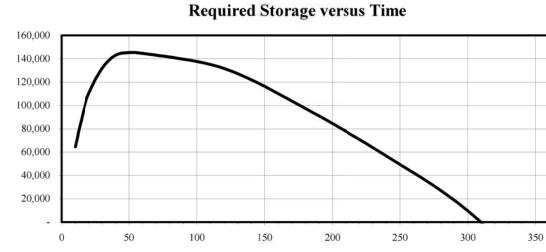
#### **Stormwater Detention Calculations Required Storage versus Time**



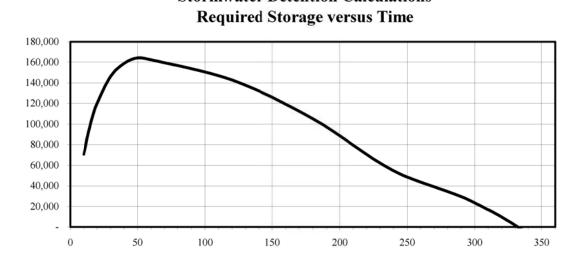
## **Stormwater Detention Calculations**



## **Stormwater Detention Calculations**



## **Stormwater Detention Calculations**

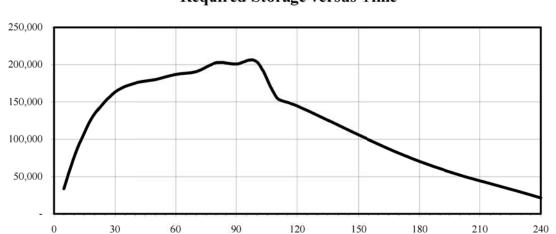


#### III. Determine Approximate Pond Area

Assumptions: Assumed pond depth is Actual pond area = 120% of calculated area for landscaping, freeboard, slopes, etc.

Pond volume required Calculated pond surface area Effective pond area

#### **Stormwater Detention Calculations** Required Storage versus Time



## **Pond Volume Calculations**

**Method:** Use the average-end area method to determine volume of storage of the pond and determine the 100-year storm high water level

Where: Volume = (1/2)\*(Area of top contour + Area of bottom contour)\*depth increment

543.47 feet High Water Elevation High Water Elevation + 1' freeboard 544.47 feet 540.00 feet Invert elevation of outlet structure

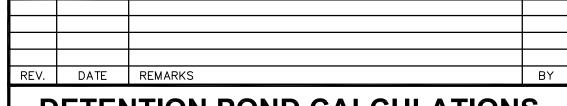
Calculated 100-yr storage 203,464 cubic feet

## **Calculations:**

_			_	1000 Mg	<b></b> 1					
			Increm	ental	To	otal				
	Elevation	Area	Volu	me	Vol	ume				
	(feet)	(sq. ft.)	(cubic ft.)	(ac. ft.)	) (ac. ft.) (cub					
	540.0	49,919.0	0.0	0.000	0.000	0				
	540.5	52,332.0	25,562.8	0.587	0.587	25,563				
	541.0	54,745.0	26,769.3	0.615	1.201	52,332				
	541.5	57,241.0	27,996.5	0.643	1.844	80,329				
	542.0	59,737.0	29,244.5	0.671	2.515	109,573				
	542.5	62,014.0	30,437.8	0.699	3.214	140,011				
	543.0	64,291.0	31,576.3	0.725	3.939	171,587				
	543.5	66,586.5	32,719.4	0.751	4.690	204,306				
	544.0	68,882.0	33,867.1	0.777	5.468	238,174				

## RECORD DRAWING

RECORD INFORMATION PROVIDED BY CONASTER CONSTRUCTION, FORT WORTH, TEXAS **ELEVATION VERIFICATION PERFORMED BY SURVEY CONSULTANTS, INC., PLANO, TEXAS** 



# **DETENTION POND CALCULATIONS**

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