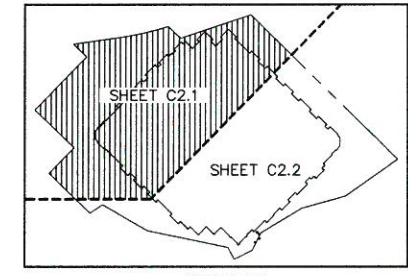


Finished Floor Elevations

Building Level 4	= 555.83
Building Level 3	= 545.33
Building Level 2	= 534.83
Building Level 1	= 524.33
Garage Level -1	= 513.33
Garage Level -2	= 503.00



NOTE FROM FALKOFSKE: This plan was developed directly from the Cross Engineering Consultants, Inc. Grading Plan dated Jan. 9, 2020 and last revised on May 20, 2020, sheet C2.1. This plan is for the sole purpose of showing what retaining wall sections to use where and to comment on the site as necessary with regards to the retaining walls. Contractor shall review final Civil Plans for other notes with regard to the site prior to constructing Masonry Stone Retaining Walls.

REVISION  
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CITY OF ROCKWALL ENGINEERING DEPT.

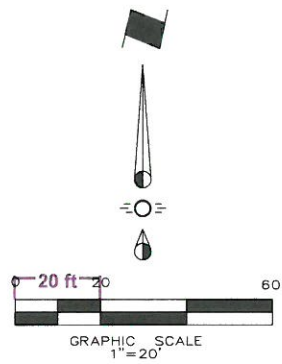
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CITY  DATE 5-27-21

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MASONRY RETAINING WALLS - SITE PLAN MARKUP HARBOR HILLS SUMMER LEE DRIVE ROCKWALL, TEXAS			
WALLCO RETAINING WALLS, INC. 4800 S.E. LOOP 820 FORT WORTH, TEXAS 76140			
5th Look Rev 1 8/19/21 Tum.B.		JOB NO. 151.20 SP1	

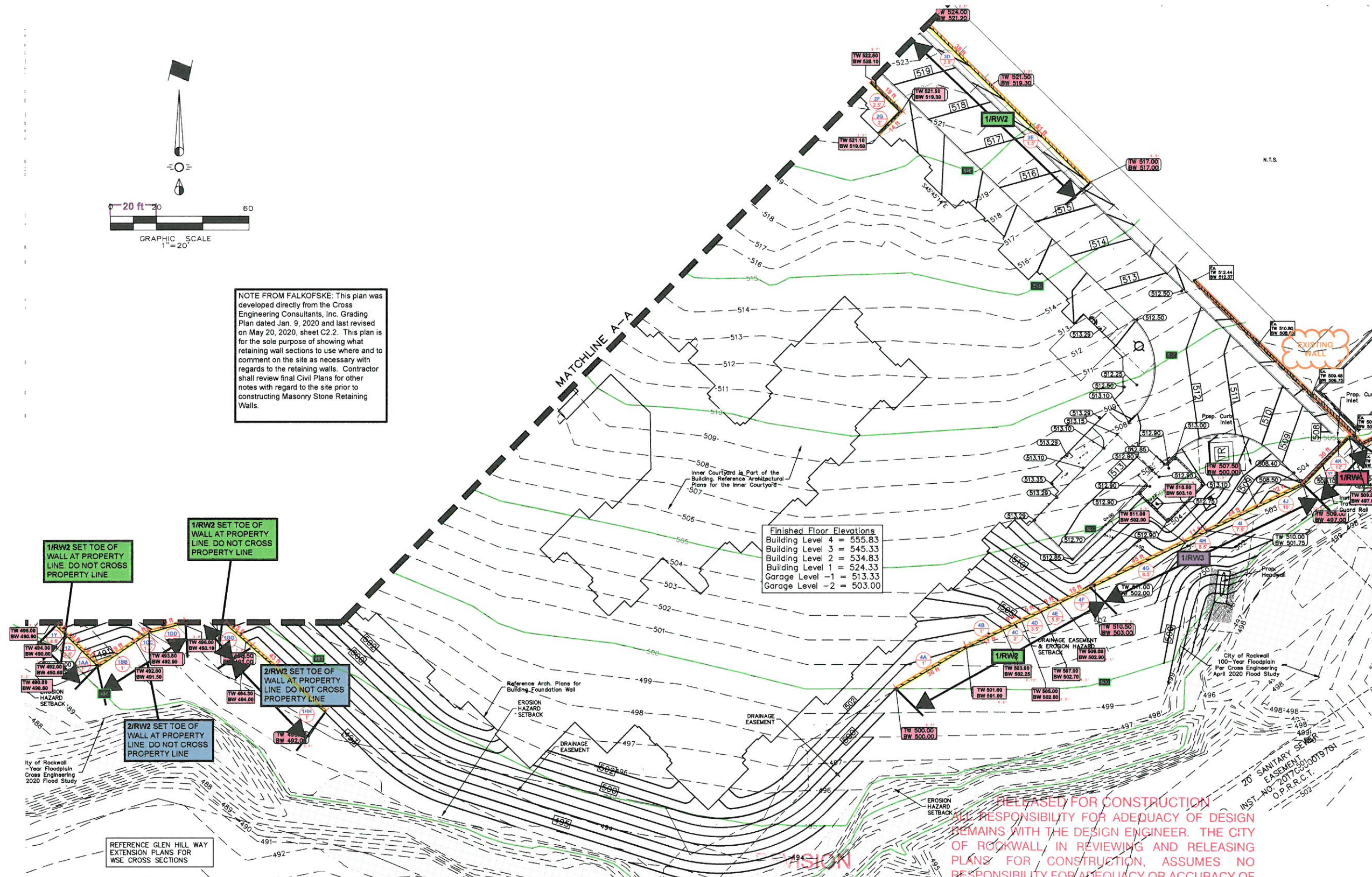
DATE	BY	DES.	CHK.	NO.	DATE	BY
03-20-20	AMB	RLB	EC	1		
03-20-20	RL	DRN	EC	2		
03-20-20	AMB	CHK.	RL	3		





NOTE FROM FALKOFSKE: This plan was developed directly from the Cross Engineering Consultants, Inc. Grading Plan dated Jan. 9, 2020 and last revised on May 20, 2020, sheet C2.2. This plan is for the sole purpose of showing what retaining wall sections to use where and to comment on the site as necessary with regards to the retaining walls. Contractor shall review final Civil Plans for other notes with regard to the site prior to constructing Masonry Stone Retaining Walls.

Finished Floor Elevations  
Building Level 4 = 555.83  
Building Level 3 = 545.33  
Building Level 2 = 534.83  
Building Level 1 = 524.33  
Garage Level -1 = 513.33  
Garage Level -2 = 503.00



DATE	BY	DES.	CHK.	NO.	DATE	BY
03-20-20	AMB	RLB				
03-20-20	RL	EC				
03-20-20	AMB	RL				
03-20-20						

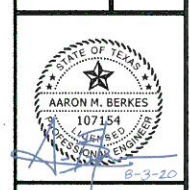
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Structural Engineering Consultants  
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MASONRY RETAINING WALLS - SITE PLAN MARKUP  
HARBOR HILLS  
SUMMER LEE DRIVE  
ROCKWALL, TEXAS

WALLCO RETAINING WALLS, INC.  
4800 S.E. LOOP 820  
FORT WORTH, TEXAS 76140



JOB NO. 151.20  
SP2

SEP 14 2021

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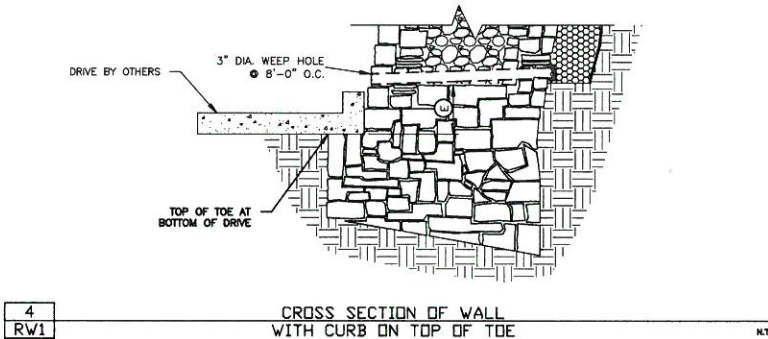
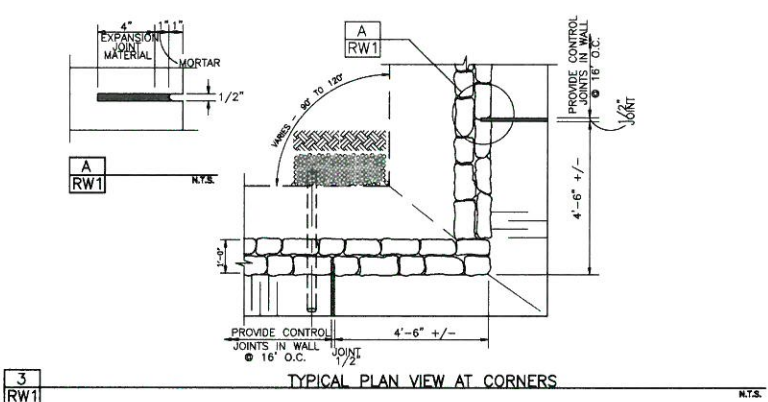
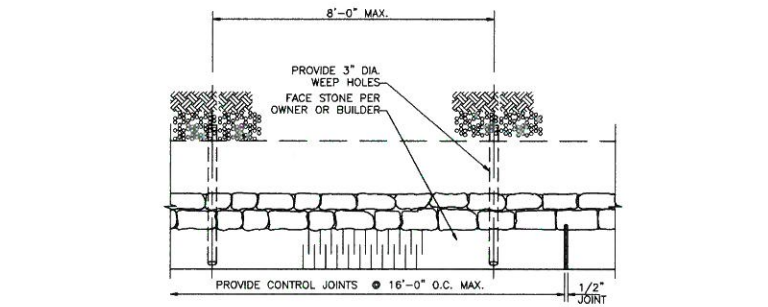
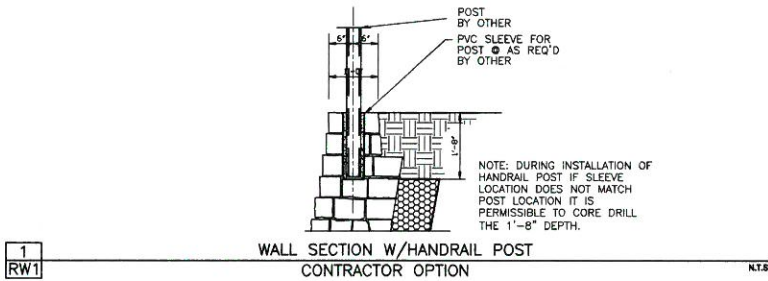
CITY  5-27-21  
DATE

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Rev 1  
8/19/21  
Tim B.









# GENERAL NOTES

## 1. Design

### 1.1. Design Codes

International Building Code, 2015 Edition

### 1.2. Geotechnical Report

Firm: RONE ENGINEERING, Inc.  
Report No.: 16-21064 Dated: May 12, 2016  
Allowable Bearing Capacity: 1500 psf

### 1.3. Design Parameters

#### Soil Parameters:

Soil Type*	Friction Angle	Cohesion (psf)	Unit Weight (pcf)
Retained Backfill (On site clay)	26 deg	0 psf	120 pcf
Foundation Soils (1500 psf)	26 deg	0 psf	120 pcf

\*See materials below for a description of each Soil Type.

#### Factors of Safety:

External Stability	
a. Minimum Factor of Safety Against Base Sliding (Static Condition)	1.5
b. Minimum Factor of Safety Against Overturning	2.0
c. Minimum Factor of Safety Against Global Stability	1.5
d. Minimum Factor of Safety for Bearing Capacity	3.0

#### Design Loading:

Lateral earth pressures are calculated using Coulombs Lateral Earth Pressure Theory. Designs have been performed to accept loading per the proposed loading conditions based on the Civil Grading Plans. A live loading of 250 psf has been used for all walls supporting areas subject to firelane loading.

Retaining walls should not have solid fence (such as wood fence) placed on top of wall other than that shown on these plans. Retaining walls shall not have additional surcharge placed above wall other than that shown on these plans. Retaining walls shall not have slope at base or top of wall that exceed that which is shown on these plans. The retaining walls noted above require special design.

## 2. Materials

### 2.1. Soil Types

- Retained Backfill
  - On site clayey soils
  - Properly compacted on-site fill soils, verification by others.
- Foundation Soils ( Allowable Bearing = 1500 psf min)
  - Bearing on Stiff Natural Undisturbed Clayey or Sandy Soils or Compacted and Tested Fill Soils
  - Friction Angle between Base of Wall and Soil - 17 deg
  - Bearing in fill soils. Fill soils supporting the retaining walls shall be placed in accordance with the recommendations for the fill placement per the geotechnical report.
- Drainage Material
  - Free draining granular backfill, clean, non-plastic, relatively well-graded.

### 2.2. Dimension Stone

- Average Density of masonry wall varies from 135pcf to 145pcf.
- Stone size varies from 4" to 18".
- Face stone shall be coordinated between contractor and owner/developer.
- Recycled concrete 4" to 18" may be used in place of dimension stone, contractors option.

### 2.3. Rebar/Welded Wire Fabric (If Required)

- All steel reinforcement shall be new billet steel conforming to ASTM A-615, Grade 60 with  $f_y=60ksi$ .
- All reinforcement shall not have deleterious material on it.
- All welded wire fabric shall have minimum  $f_y=60ksi$  and be hot dip galvanized.

### 2.4. Drainage Materials

- Weep pipes shall be PVC or corrugated HDPE pipe.
- Drainage zone shall be separated from retained backfill by miraf 140N filter fabric or approved equal.

### 2.5 Portland Cement Mortar for Retaining Wall Construction.

The portland cement mortar used for construction of the masonry stone retaining walls shall be provided with the following proportions per cubic yard of concrete. The portland cement mortar supplier shall provide "batch tickets" clearly indicating that the appropriate amount of materials are provided in each truck load. The batch tickets shall clearly indicate the amount batched, the date, the project name and shall be provided to Falkofsky Engineering, Inc. for review, documentation, and file.

Contents	Amount per cubic yard	Specific Gravity	Volume ft <sup>3</sup>
Type 1 Portland cement:	451 lbs	3.15	2.29
Type F Fly Ash	113 lbs	2.93	0.62
Fine Aggregate (sand):	2746 lbs	2.59	16.99
Potable Water	367 lbs	44 Gallons	5.88
Sika Air (or equivalent)	(AS REQ'D) oz	4.5%	1.22
			27.0 Total

Note: the portland cement mortar supplier material weights may vary slightly based on the specific gravity of the materials used.

Concrete retarders may be used at the discretion of the masonry wall contractor. A greater amount of retarder is typically used during hot periods and a less amount of retarder is typically used during cool weather.

Please note that the above proportions will provide a portland cement mortar with a compressive strength of about  $f_c = 2500$  psi. Falkofsky Engineering, Inc. does not require any concrete testing provided the above proportions are verified by way of the "batch tickets".

## 3. Construction

### 3.1 Preparation Work

- Prior to grading or excavation of the site, confirm the location of the retaining walls and all underground features, including utility location within the area of construction. Ensure surrounding structures are protected from effects of wall excavation, and construction.
- Coordinate installation of underground utilities and other improvements with wall installation.

## 3.2 Excavation

- If a mortared footing is over-excavated, then the dimension stone shall be placed mortared. If a dry stone footing is over excavated, then the dimension stone does not need to be mortared.
- Fill over-excavated area in front of the wall footing with compacted on site soils before the wall construction exceeds 4 feet in height.
- In areas where the walls are installed in a cut, the required excavation shall extend horizontally to the extent of the width of the retaining wall. The wall may be built to the cut. If the wall is over cut, then soil shall either be compacted or the drainage zone may be widened.

## 3.3 Wall Construction

- The wall shall be constructed to the dimensions as shown on these plans. Front leads, back leads, and string lines shall be set for each wall. Care shall be taken to install the mortar zones the correct thickness, and to place drainage behind the wall as required.
- Control joints shall be installed at a maximum of 16'-0" o.c. per these plans.
- Weep pipes shall be placed at 8'-0" o.c. max.
- Face rock type shall be coordinated between the architect, owner, and retaining wall contractor.

## 3.4 Retained Backfill Placement

- Retained backfill shall be placed per the recommendations of the geotechnical engineer, but should not be less than 93% Standard Proctor Maximum Dry Density (ASTM D698).
- Fill should be placed in maximum 8" thick compacted lifts.
- Large compaction equipment (equipment heavier than 7,500 lb) shall remain a minimum of 1.5x the height of the wall away from the back of the wall for a period of 2 weeks from the time of construction.
- After a period of 2 weeks from the time of construction large compaction equipment may be used behind the wall but shall stay a minimum of 5'-0" away from the back of the wall.
- Soil placed within 5'-0" of the back of the wall shall be placed using handheld compaction equipment.
- If the wall is in a cut situation the wall may be built up to the cut. If the wall is overcut the drainage zone may be widened to the cut or compacted fill may be placed between the drainage zone and the cut.

## 3.6 Retaining Wall Performance, Maintenance, and Other Comments

- Control joints are provided in the retaining wall to allow for minor movements due to settlement and shrink swell of the soils. Some cracking may occur in the face of the retaining wall. This cracking, if minor (less than 3/8"), may be cosmetically repaired as desired.
- The retaining walls are designed to allow surface water to flow over the tops of the retaining walls. Care should be taken during and after construction to not allow water to pond behind the retaining walls, as this can have a negative impact on the stability of the retaining walls.
- If downspouts are located near the back of the retaining wall they should either be plumbed through the retaining wall to drain below the wall or collected and tied into the storm sewer system. Perforated subsurface pipes shall not be used behind the retaining walls.
- Positive drainage over the top of the walls shall be maintained throughout the life of the structure. If swales are placed behind the wall they shall remain clean and free draining. If water is found to be ponding in the swale it shall be fixed to allow water to freely drain as soon as possible.
- Any broken sprinklers behind the retaining wall shall be turned off and repaired as soon as possible.

## 3.7 Cold Weather Construction of Retaining Walls

### Construction Requirements for temperatures between 40°F and 32°F:

- Water and aggregates used in mortar shall not be heated above 140°F.
- Mortar sand or mixing water shall be heated to produce mortar temperatures between 40°F and 120°F at the time of mixing.

### Construction Requirements for temperatures between 32°F and 25°F:

- The guidelines above for construction requirements for temperatures between 40°F and 32°F and the following shall be met.
- The mortar temperature shall be maintained above freezing until used in masonry stone retaining wall.
- Visible ice and snow shall be removed from the top surface of existing foundations and masonry to receive new construction. These surfaces shall be heated to above freezing, using methods that do not result in damage.
- Newly constructed masonry shall be completely covered with weather-resistive membrane for 48 hours after being completed.

### Construction Requirements for temperatures between 25°F and 20°F:

- The guidelines above for construction requirements for temperatures between 40°F and 32°F, the construction guidelines for temperatures between 32°F and 25°F, and the following shall be met.
- Masonry (raw stone) surfaces under construction shall be heated to 40°F.
- Wind breaks or enclosures shall be provided when the wind velocity exceeds 15 miles per hour.
- Newly constructed masonry shall be completely covered with weather-resistive insulating blankets, or equal protection, for 48 hours after being completed.

The above procedures comes from sections 2104.3.2.1, 2104.3.2.2, 2104.3.2.3, 2104.3.3.3, and 2104.3.3.4 of the International Building Code, and is in compliance with Masonry Standards Joint Committee recommendations for cold weather construction of masonry structures.

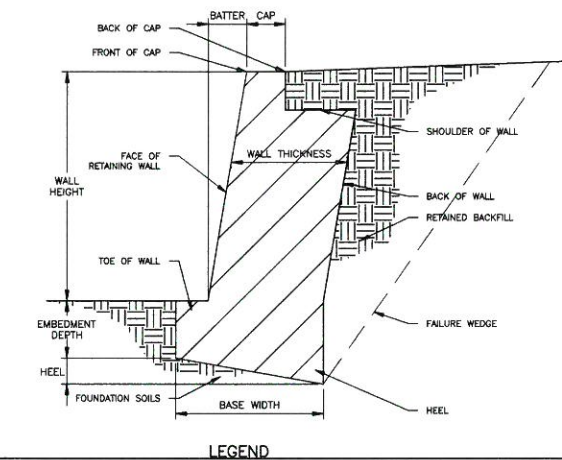
## 4. Construction Observations

### 4.1 Construction Observations by Falkofsky Engineering, Inc.

- Falkofsky Engineering, Inc. will perform construction observation, but only as a means of verification of the contractors quality control performance.
- Falkofsky Engineering, Inc. will act as the Special Inspector for this project. Contractor shall contact Falkofsky Engineering to set up inspections, at least 1 day before construction starts.
- All required materials testing shall be performed by an approved materials testing laboratory.
- Falkofsky Engineering, inc. is not responsible for means, methods, and material furnished by the retaining wall contractor.

### 4.2 Construction Observations by Others

- Construction observations as required by the city shall be coordinated by the contractor.



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DATE

5-27-21

5th Look  
Rev 1  
8/19/21  
Tim B.

JOB NO. 151.20

RW1



MASONRY RETAINING WALLS - NOTES & STANDARD DETAILS  
HARBOR HILLS  
SUMMER LEE DRIVE  
ROCKWALL, TEXAS  
WALCO RETAINING WALLS, INC.  
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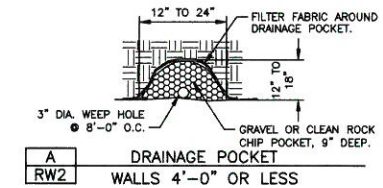
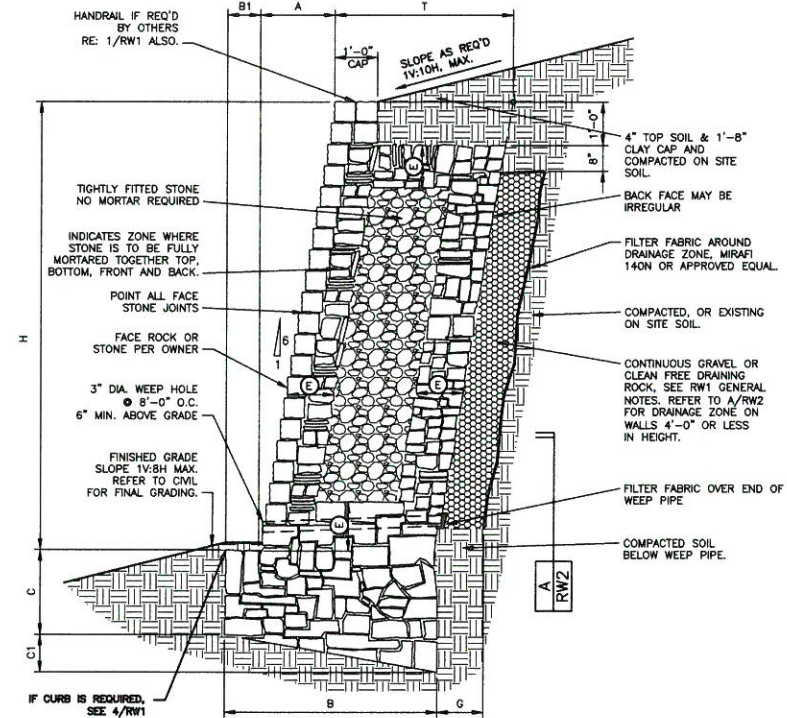
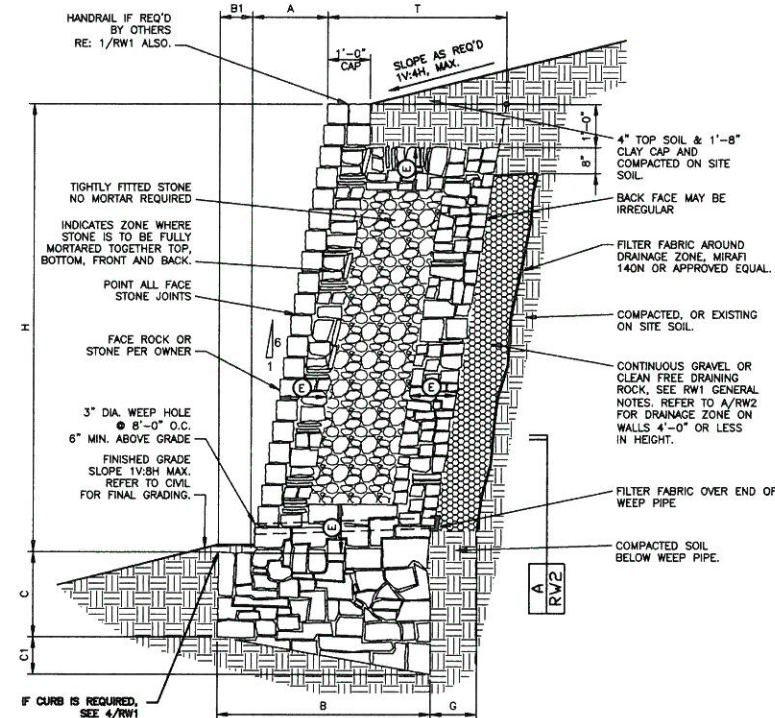
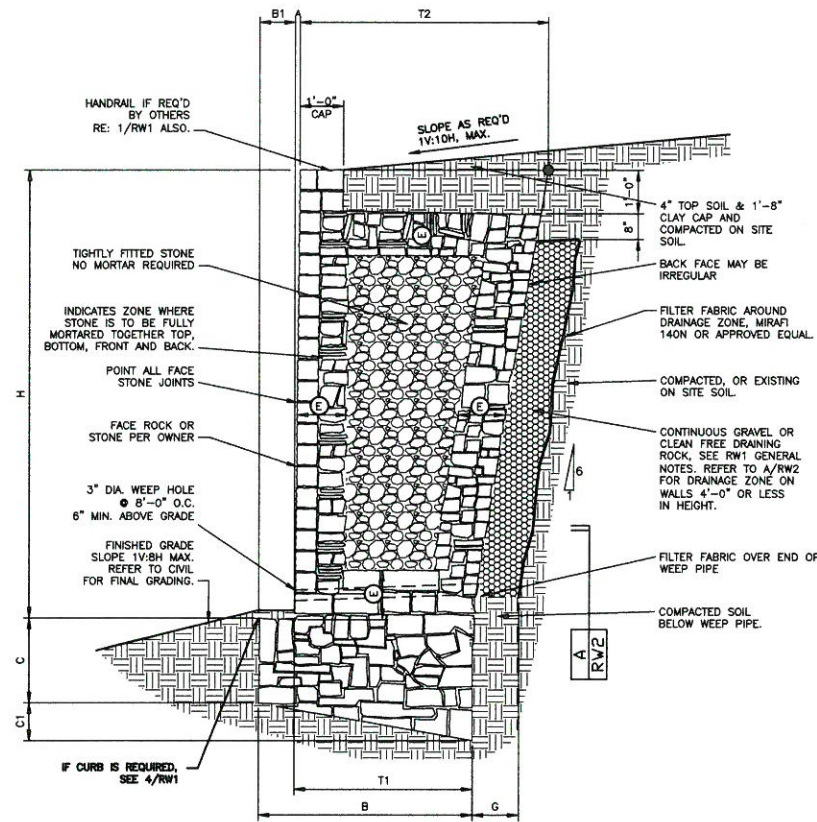
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DATE	BY	DES.	DRN.	CHK.	NO.	DATE	REVISION	BY
03-28-20	AMB		RL	AMB				





MASONRY WALL SCHEDULE									
1500 psf - BEARING CAPACITY (SEE GENERAL NOTES SHEET RW1)									
WALL HEIGHT H	BASE WIDTH B	TOE B1	DRIVE DEPTH (FOOT) C1	DRIVE DEPTH (FEET) C2	BATTER A	THICKNESS OF WALL T	THICKNESS OF WALL T2	THICKNESS OF WALL T3	BEARING CAPACITY
1'-0"	1'-0"	0'-0"	0'-0"	0'-2"	1/4"	1'-0"	1'-2"	SEE A/RW2	1500 psf
2'-0"	1'-2"	0'-2"	0'-0"	0'-3"	1/2"	1'-0"	1'-4"	SEE A/RW2	1500 psf
3'-0"	1'-6"	0'-3"	0'-0"	0'-4"	3/4"	1'-0"	1'-8"	SEE A/RW2	1500 psf
4'-0"	2'-1"	0'-5"	1'-0"	0'-5"	1"	1'-0"	2'-4"	SEE A/RW2	1500 psf
5'-0"	2'-9"	0'-7"	1'-3"	0'-6"	0'-1 1/4"	0'-8"	2'-2"	3'-0"	1'-0"
6'-0"	3'-5"	0'-10"	1'-6"	0'-8"	0'-1 1/2"	0'-10"	2'-7"	3'-7"	1'-0"
7'-0"	4'-0"	1'-0"	1'-6"	0'-9"	0'-1 3/4"	0'-10"	3'-0"	4'-2"	1'-0"
8'-0"	4'-10"	1'-4"	2'-3"	0'-11"	0'-2"	1'-0"	3'-8"	4'-10"	1'-0"

USE THIS SCHEDULE FOR 3/RW2

MASONRY WALL SCHEDULE									
1500 psf - BEARING CAPACITY (SEE GENERAL NOTES SHEET RW1)									
WALL HEIGHT H	BASE WIDTH B	TOE B1	DRIVE DEPTH (FOOT) C1	DRIVE DEPTH (FEET) C2	BATTER A	THICKNESS OF WALL T	THICKNESS OF WALL T2	THICKNESS OF WALL T3	BEARING CAPACITY
1'-0"	1'-0"	0'-0"	0'-0"	0'-2"	1/4"	1'-0"	1'-2"	SEE A/RW2	1500 psf
2'-0"	1'-4"	0'-2"	0'-0"	0'-3"	1/2"	1'-0"	1'-4"	SEE A/RW2	1500 psf
3'-0"	1'-8"	0'-3"	0'-0"	0'-4"	3/4"	1'-0"	1'-8"	SEE A/RW2	1500 psf
4'-0"	2'-2"	0'-4"	1'-0"	0'-5"	1"	1'-0"	2'-4"	SEE A/RW2	1500 psf
5'-0"	3'-3"	0'-5"	1'-3"	0'-6"	0'-1 1/4"	0'-8"	2'-2"	3'-0"	1'-0"
6'-0"	4'-0"	0'-7"	1'-6"	0'-8"	0'-1 1/2"	0'-10"	2'-7"	3'-7"	1'-0"
7'-0"	4'-10"	0'-9"	1'-9"	0'-10"	0'-1 3/4"	0'-10"	3'-0"	4'-2"	1'-0"
8'-0"	5'-8"	0'-10"	2'-3"	0'-11"	0'-2"	1'-0"	3'-8"	4'-10"	1'-0"
9'-0"	6'-10"	0'-11"	2'-6"	0'-11"	0'-2 1/4"	1'-0"	4'-10"	5'-8"	1'-0"
10'-0"	7'-7"	0'-12"	3'-0"	0'-12"	0'-2 1/2"	1'-0"	5'-8"	6'-10"	1'-0"
11'-0"	8'-5"	0'-13"	3'-6"	0'-13"	0'-3"	1'-0"	6'-10"	7'-7"	1'-0"

USE THIS SCHEDULE FOR 2/RW2

MASONRY WALL SCHEDULE									
1500 psf - BEARING CAPACITY (SEE GENERAL NOTES SHEET RW1)									
WALL HEIGHT H	BASE WIDTH B	TOE B1	DRIVE DEPTH (FOOT) C1	DRIVE DEPTH (FEET) C2	BATTER A	THICKNESS OF WALL T	THICKNESS OF WALL T2	THICKNESS OF WALL T3	BEARING CAPACITY
1'-0"	1'-0"	0'-0"	0'-0"	0'-2"	1/4"	1'-0"	1'-2"	SEE A/RW2	1500 psf
2'-0"	1'-2"	0'-2"	0'-0"	0'-3"	1/2"	1'-0"	1'-4"	SEE A/RW2	1500 psf
3'-0"	1'-6"	0'-3"	0'-0"	0'-4"	3/4"	1'-0"	1'-8"	SEE A/RW2	1500 psf
4'-0"	2'-1"	0'-5"	1'-0"	0'-5"	1"	1'-0"	2'-4"	SEE A/RW2	1500 psf
5'-0"	2'-9"	0'-7"	1'-3"	0'-6"	0'-1 1/4"	0'-8"	2'-2"	3'-0"	1'-0"
6'-0"	3'-5"	0'-10"	1'-6"	0'-8"	0'-1 1/2"	0'-10"	2'-7"	3'-7"	1'-0"
7'-0"	4'-0"	1'-0"	1'-6"	0'-9"	0'-1 3/4"	0'-10"	3'-0"	4'-2"	1'-0"
8'-0"	4'-10"	1'-4"	2'-3"	0'-11"	0'-2"	1'-0"	3'-8"	4'-10"	1'-0"
9'-0"	5'-8"	0'-10"	2'-6"	0'-11"	0'-2 1/4"	1'-0"	4'-10"	5'-8"	1'-0"
10'-0"	6'-10"	0'-11"	2'-9"	0'-12"	0'-2 1/2"	1'-0"	5'-8"	6'-10"	1'-0"
11'-0"	7'-7"	0'-12"	3'-0"	0'-12"	0'-2 1/2"	1'-0"	6'-10"	7'-7"	1'-0"

USE THIS SCHEDULE FOR 1/RW2

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5-27-21

3  
RW2 TYPICAL VERTICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL  
BEARING IN CLAYEY OR SANDY SOILS 3/4" = 1'-0"

2  
RW2 TYPICAL WALL SECTION  
BEARING IN CLAYS  
MAX. SLOPE ABOVE WALL 1V:4H  
MAX. SLOPE BELOW WALL 1V:8H

1  
RW2 TYPICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL  
1V:8H MAX SLOPE BELOW WALL  
BEARING IN CLAYEY OR SANDY SOILS 1/2" = 1'-0"

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JOB NO. 151.20

RW2

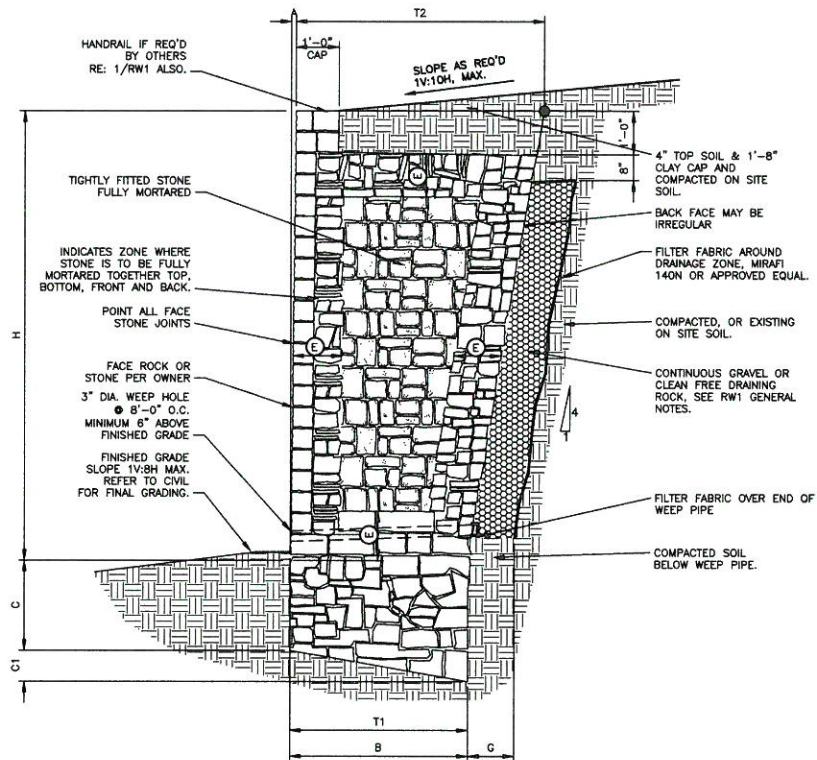
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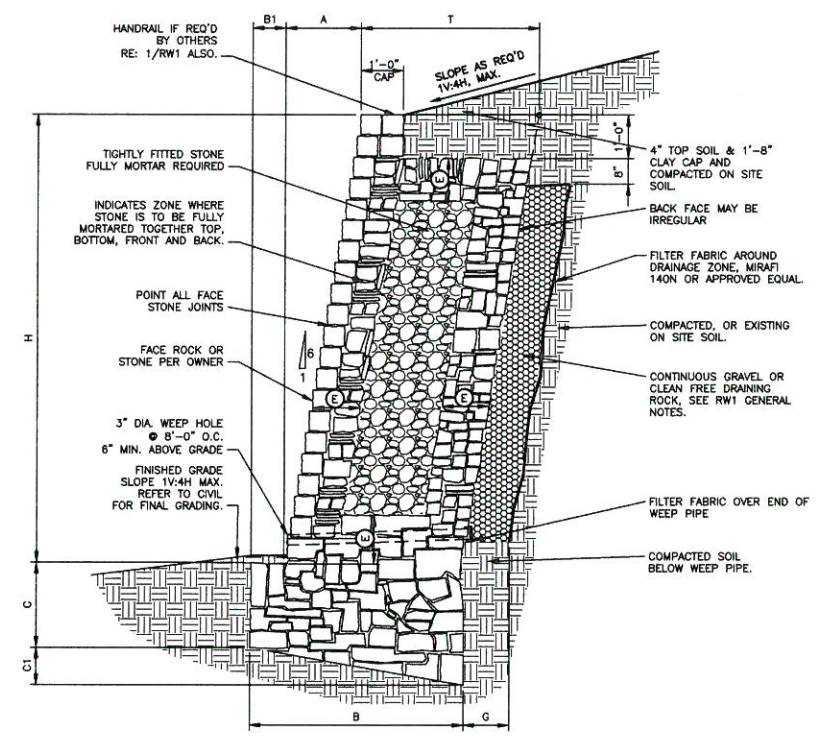




MASONRY WALL SCHEDULE									
1500 psf - BEARING CAPACITY (STIFF NATURAL UNDISTURBED SOILS OR COMPACTED AND TESTED SOILS SEE GENERAL NOTES SHEET RW1)									
WALL HEIGHT H	BASE WIDTH B	BASE DEPTH (FOOT) C	BASE DEPTH (FEET) C1	BATTERY A	THICKNESS OF WALL T1	THICKNESS OF WALL T2	DRAINAGE ZONE THICKNESS G	BEARING CAPACITY	
4'-0"	2'-0"	1'-0"	1'-0"	0'-1"	2'-0"	3'-0"	1'-0"	1500 psf	
5'-0"	2'-6"	1'-3"	1'-3"	0'-1 1/4"	2'-6"	3'-6"	1'-0"	1500 psf	
6'-0"	3'-0"	1'-6"	1'-6"	0'-1 1/2"	3'-0"	4'-6"	1'-0"	1850 psf	
7'-0"	3'-6"	1'-9"	1'-9"	0'-1 1/2"	3'-6"	5'-3"	1'-0"	2150 psf	
WALL DESIGN CRITERIA									
BEARING Q <sub>a</sub>	SLOPE TOP S	SLOPE BOT S1	ACTIVE PRESSURE P <sub>a</sub>	PASSIVE PRESSURE P <sub>p</sub>	FRICITION ANGLE BASE δ	SLOPE OF BACK OF WALL J	BURDWARE d		
1500PSF	5.71 deg	7.13 deg	28 deg	26 deg	17 deg	104.04 deg	0 pcf		

USE THIS SCHEDULE FOR 2/RW3

2  
RW3 TYPICAL VERTICAL WALL SECTION - 1V:10H MAX SLOPE ABOVE WALL BEARING IN CLAYEY OR SANDY SOILS 3/4" = 1'-0"



MASONRY WALL SCHEDULE									
1500 psf - BEARING CAPACITY (STIFF NATURAL UNDISTURBED SOILS OR COMPACTED AND TESTED SOILS SEE GENERAL NOTES SHEET RW1)									
WALL HEIGHT H	BASE WIDTH B	TOE B1	BASE DEPTH (FOOT) C	BASE DEPTH (FEET) C1	BATTER A	WALL THICKNESS T	THICKNESS OF WALL T1	DRAINAGE ZONE THICKNESS G	BEARING CAPACITY
5'-0"	3'-6"	0"	3'-0"	0'-8"	0'-10"	0'-9"	3'-11"	1'-0"	1500 psf
6'-0"	4'-3"	0"	3'-6"	0'-9"	1'-0"	1'-0"	3'-9"	1'-0"	
7'-0"	5'-2"	0"	4'-0"	0'-10"-11"	1'-2"	1'-0"	4'-7"	1'-0"	
8'-0"	6'-1"	0"	4'-6"	1'-1"	1'-4"	1'-3"	5'-5"	1'-0"	
9'-0"	7'-3"	0"	5'-0"	1'-3"	1'-6"	1'-3"	6'-8"	1'-0"	
10'-0"	8'-1"	0"	5'-6"	1'-5"	1'-8"	1'-6"	7'-3"	1'-0"	2000 psf
11'-0"	9'-0"	1'-0"	7'-0"	1'-7"	1'-10"	1'-6"	8'-0"	1'-0"	
WALL DESIGN CRITERIA									
BEARING Q <sub>a</sub>	SLOPE TOP S	SLOPE BOT S <sub>1</sub>	ACTIVE PRESSURE P <sub>a</sub>	PASSIVE PRESSURE P <sub>p</sub>	FRICITION ANGLE BASE δ	SLOPE OF BACK OF WALL J	SURCHARGE d		
1500psf	14 deg	14 deg	26 deg	26 deg	17 deg	99.46 deg	0 psf		

USE THIS SCHEDULE FOR 1/RW3

1  
RW3 TYPICAL WALL SECTION - 1V:4H MAX SLOPE ABOVE WALL 1V:4H MAX SLOPE BELOW WALL BEARING IN CLAYEY SOILS 1/2" = 1'-0"

REVISION  
SEP 14 2021

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CITY 3J DATE 5-27-21

5th Look  
Rev 1  
8/19/21  
Tim B.

DATE	BY	DES.	CHK.	NO.	DATE	REVISION	BY
03-28-20	AMB						
03-28-20	RL						
03-28-20	AMB						

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WALCO RETAINING WALLS, INC.  
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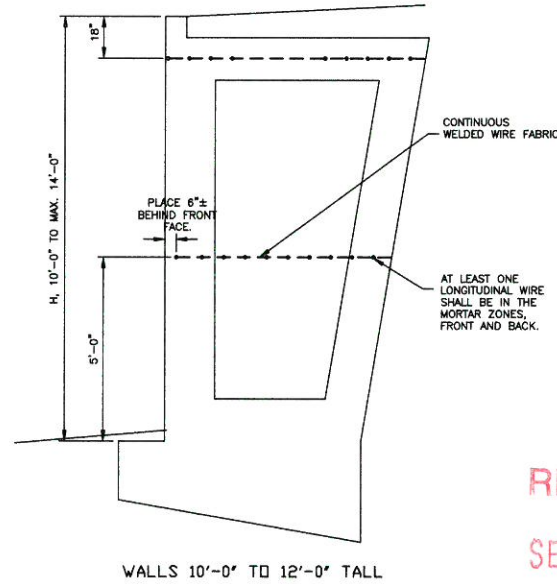
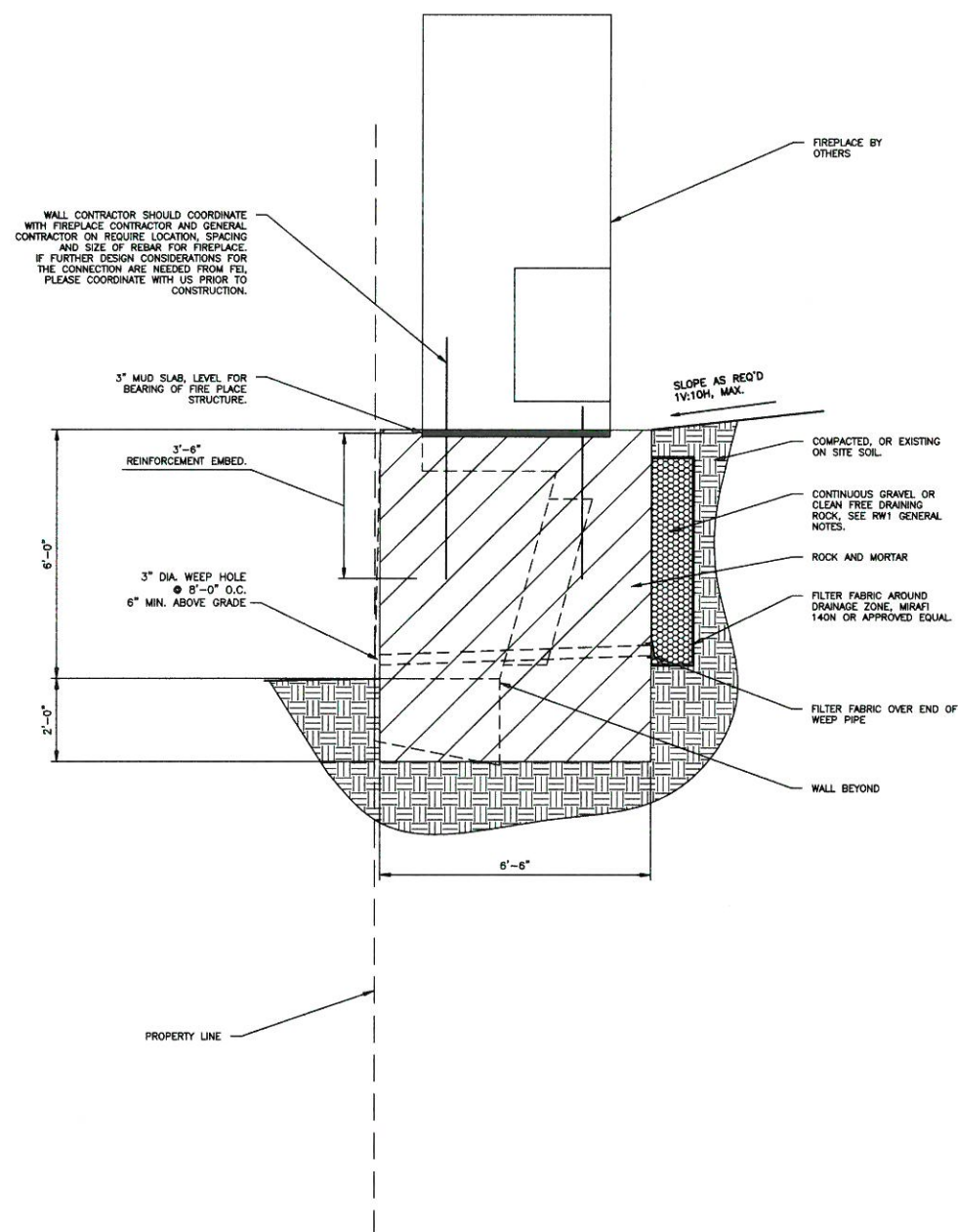
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107154  
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5-26-20

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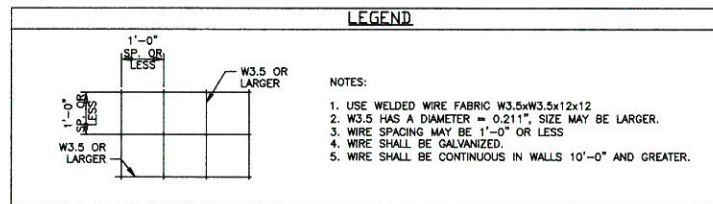
**RW3**



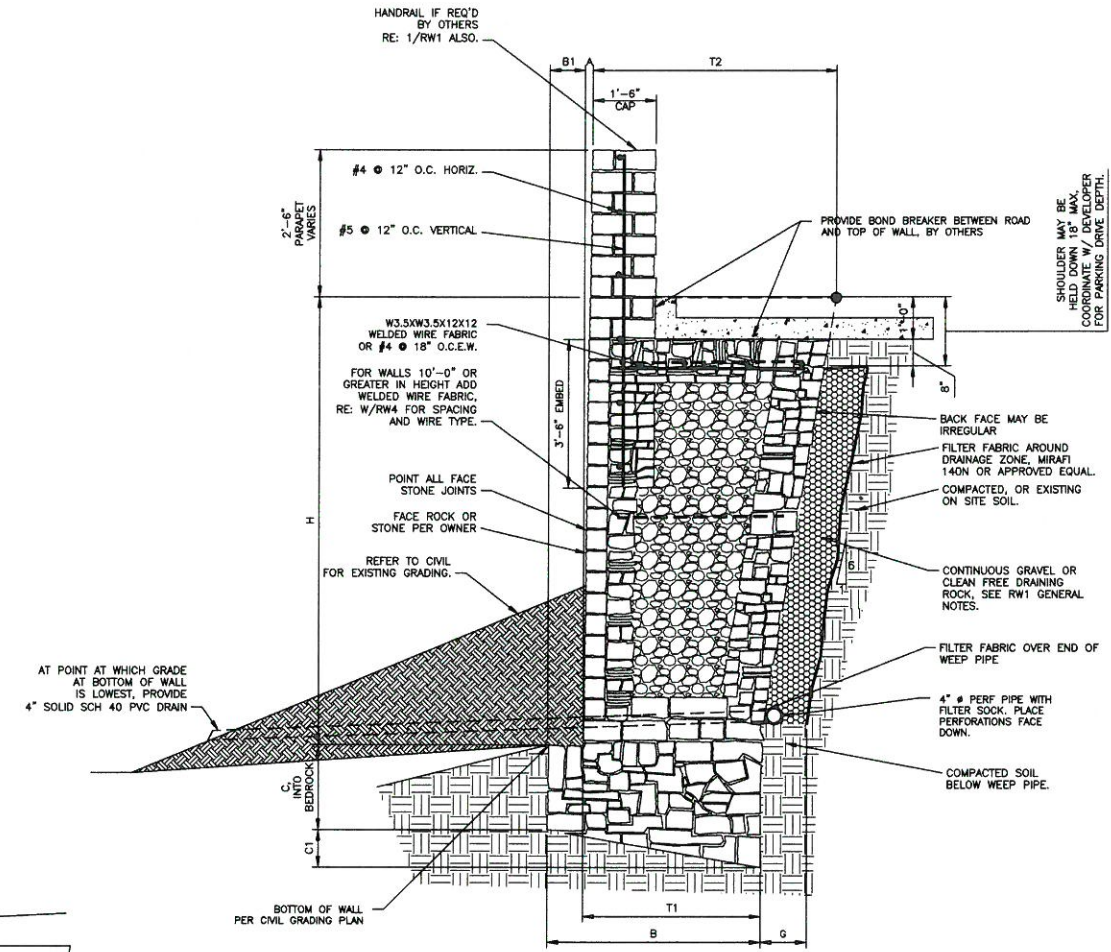


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SEP 14 2021

CITY OF ROCKWALL ENGINEERING DEPT.



W  
RW4 WIRE SPACING FOR WALLS - VERTICAL FACE WALLS  
10'-0" AND GREATER



MASONRY WALL SCHEDULE										
1500 psf - MINIMUM CAPACITY (EXCEPT WHERE NOTED OTHERWISE) BASED ON COMPACTED AND REDUCED SOILS (SEE GENERAL NOTES FOR SOIL DATA)										
TOTAL HEIGHT H	SOIL TYPE	TOE B1	BASE DEPTH (FOT) C	HEEL DEPTH (FOT) C1	CHUTE A	MINIMUM WALL THICKNESS OF WALL T1	MINIMUM WALL THICKNESS OF WALL T2	MINIMUM WALL THICKNESS OF WALL T3	MINIMUM WALL THICKNESS OF WALL T4	MINIMUM WALL THICKNESS OF WALL T5
10'-0"	7'-1"	1'-10"	4'-0"	1'-3"	0'-2 1/2"	8'-3"	6'-11"	1'-0"	3050 psf	
11'-0"	7'-0"	2'-0"	4'-0"	1'-4"	0'-2 3/4"	8'-3"	7'-4"	1'-3"	3150 psf	
12'-0"	8'-1"	2'-2"	4'-0"	1'-6"	0'-3"	8'-3"	7'-11"	1'-3"	3200 psf	
WALL DESIGN CRITERIA										
DESIGN SO	SLOPE TOP	SLOPE BOT	ADDED FRICTION	ADDED FRICTION	ADDED FRICTION	SLOPE OF BACK OF WALL	CURBWEIGHT			
1500PSF	0 deg	7.13 deg	20 deg	20 deg	17 deg	90.46 deg	250 psf			

USE THIS SCHEDULE FOR 1/RW4

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DATE

1  
RW4 TYPICAL VERTICAL WALL SECTION - 250 PSF FIRELANE SURCHARGE  
1V/8H MAX SLOPE BELOW WALL  
BEARING IN CLAYEY OR SANDY SOILS

3/4" = 1'-0"  
5th Look  
Rev 1  
8/19/21  
Tim B.

DATE	BY	DES.	DRN.	CHK.	INL.	DATE	REVISION	BY
03-20-20	AMB							
03-20-20	RL							
03-20-20	AMB							
03-20-20	RL							

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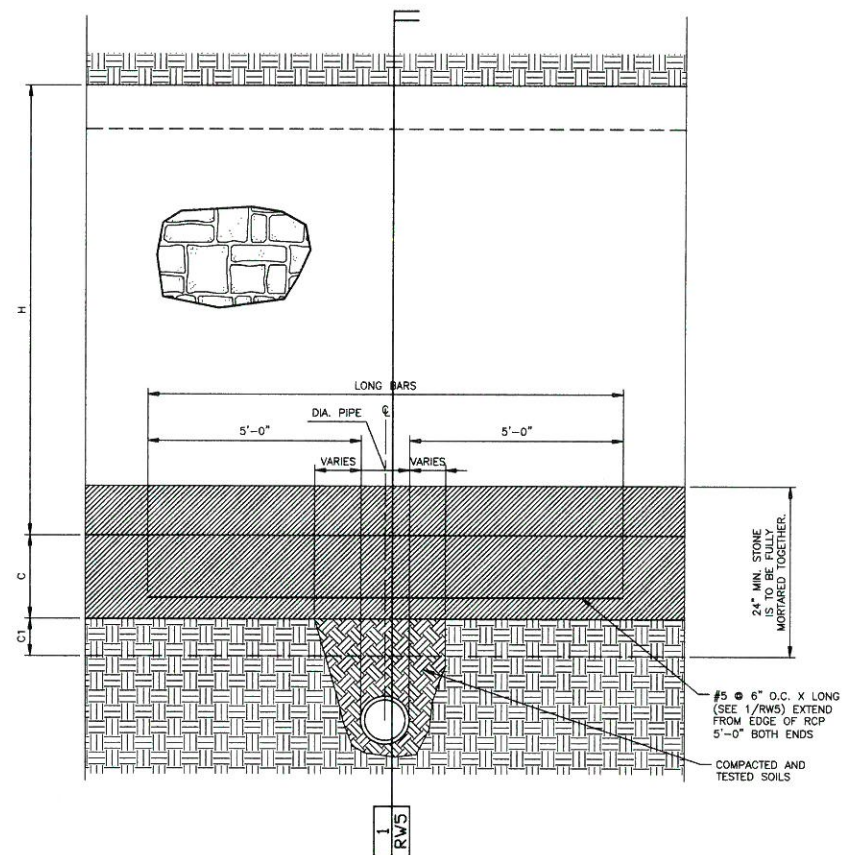
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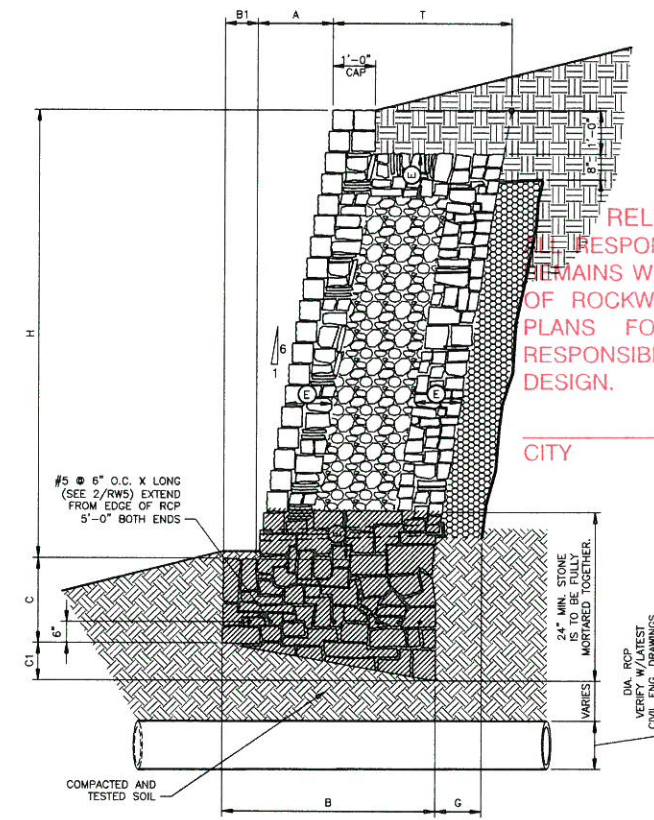
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107154  
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RW4





2  
RW5 WALL ELEVATION W/RCP BELOW WALL



1  
RW5 WALL SECTION W/RCP BELOW WALL

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CITY SJ DATE 5-27-21

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SEP 14 2021

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T.L.M.B.



JOB NO. 151.20

RW5

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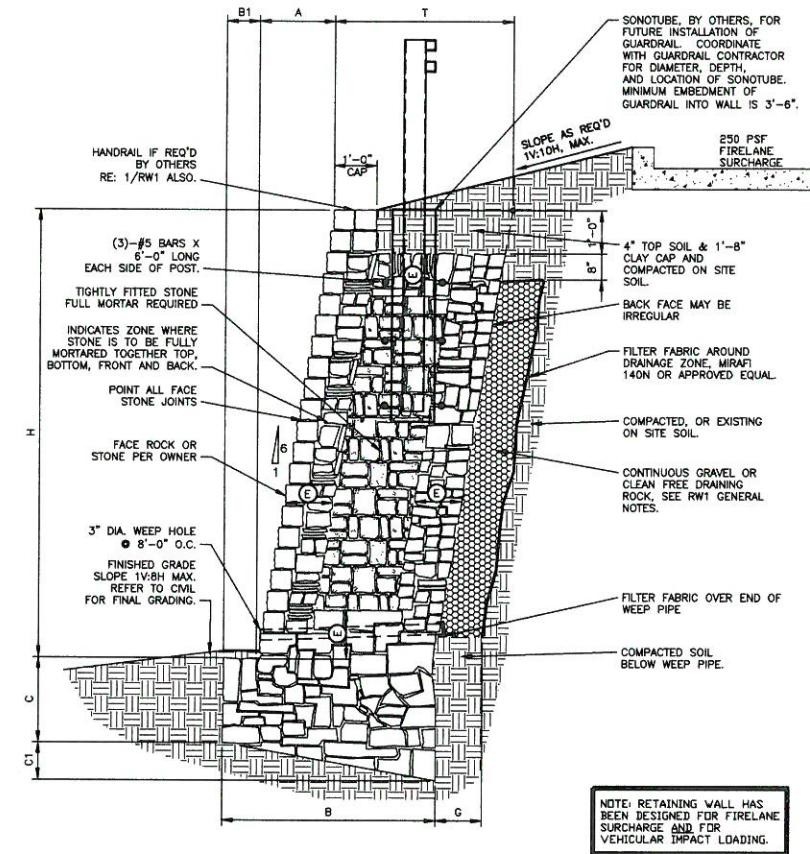


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DES.	DATE	BY	NO.	DATE	REVISION	BY
DES.	03-20-20	AMB				
DRN.	03-20-20	EJC				
CHK.	03-20-20	AMB				





MASONRY WALL SCHEDULE									
1400 psf - STRENGTH CAPACITY (DO NOT EXCEEDS UNWEIGHTED DEAD AND SUPERIMPOSED LOADS AND GENERAL NOTES APPLY)									
WALL HEIGHT H	BASE WIDTH B	TOP WIDTH B1	BASE DRAIN C	BASE DRAIN D	BASE DRAIN E	BASE DRAIN F	BASE DRAIN G	BASE DRAIN H	STRENGTH CAPACITY
6'-0"	5'-3"	1'-6"	2'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1'-0"	1500 psf
7'-0"	5'-7"	1'-6"	2'-0"	1'-1"	1'-2"	1'-2"	1'-2"	1'-2"	1500 psf
8'-0"	6'-4"	1'-6"	2'-0"	1'-2"	1'-4"	1'-4"	1'-4"	1'-4"	1500 psf
9'-0"	7'-0"	1'-6"	2'-3"	1'-3"	1'-6"	1'-6"	1'-6"	1'-6"	1500 psf
10'-0"	7'-6"	1'-6"	2'-6"	1'-4"	1'-8"	1'-8"	1'-8"	1'-8"	1500 psf
11'-0"	8'-0"	1'-6"	2'-6"	1'-5"	1'-10"	1'-10"	1'-10"	1'-10"	2000 psf
WALL DESIGN CRITERIA									
WINDSPEED G	SLOPE TOP	SLOPE BOT	WIND PRESSURE **	WIND PRESSURE **	WIND PRESSURE **	WIND PRESSURE **	WIND PRESSURE **	WIND PRESSURE **	
1500PSF	0 deg	7.13 deg	20 deg	20 deg	17 deg	99.46 deg	250 psf		

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CITY                      DATE 5-27-21

1  
RW6 TYPICAL WALL SECTION - 250PSF FIRELANE SURCHARGE  
1V:10H MAX SLOPE ABOVE WALL, 1V:8H MAX SLOPE BELOW WALL  
BEARING IN CLAYEY SOILS

DATE	BY	DES.	CHK.	NO.	DATE	REVISION	BY
05-26-20	AMB						
05-26-20	RL						
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RW6