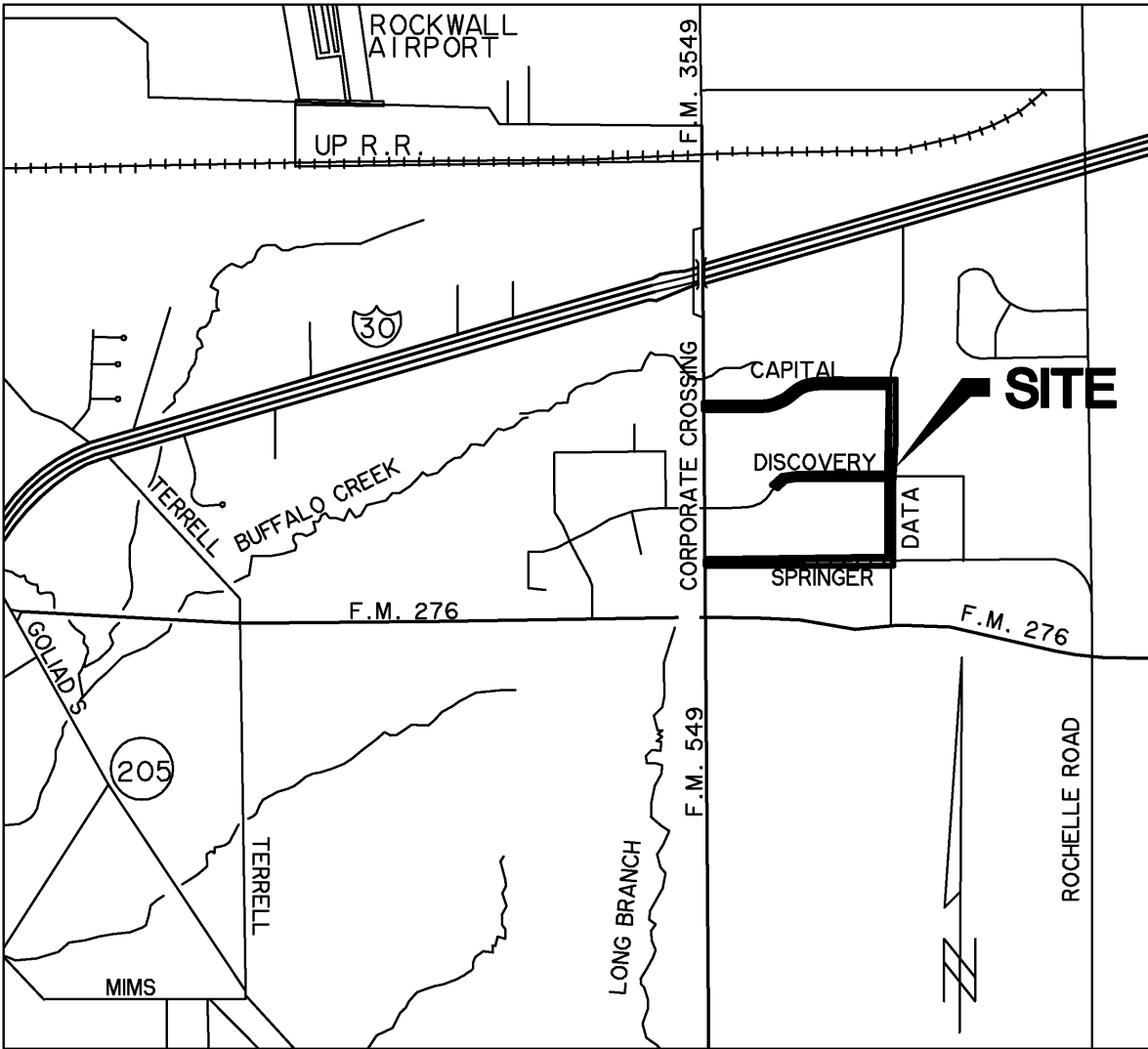


CITY OF ROCKWALL, TEXAS

PAVING, DRAINAGE, WATER & SEWER IMPROVEMENTS TO SERVE ROCKWALL TECHNOLOGY PARK PHASE IV

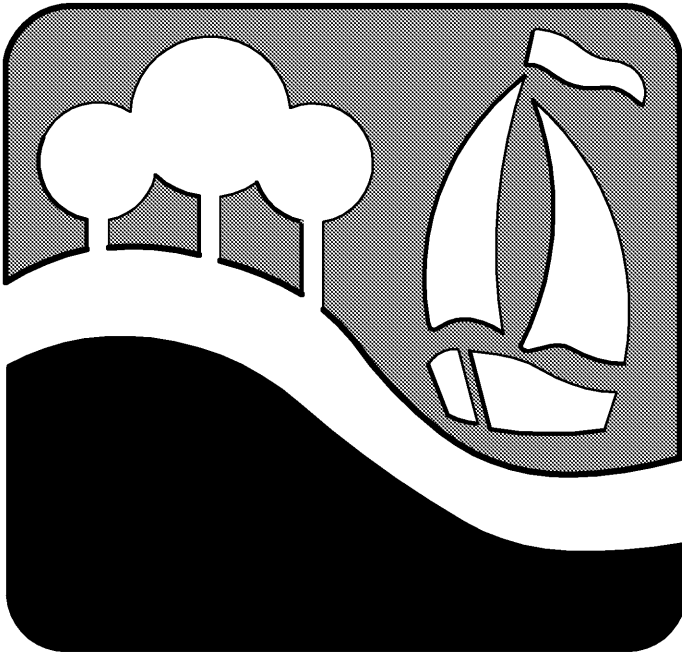
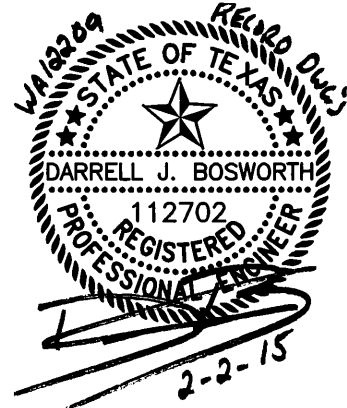
INDEX OF DRAWINGS



VICINITY MAP
N.T.S

REVISIONS 4/30/14

ADDED SHEETS S101 AND S201 TO CONSTRUCTION PLANS



"THE NEW HORIZON"

PREPARED BY:
WIA WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

FEBRUARY, 2014

DAVID SWEET - MAYOR

COUNCIL MEMBERS:

DAVID WHITE - Mayor Pro Tem
JIM PRUITT
BENNIE DANIELS
DENNIS LEWIS
SCOTT MILDER
MIKE TOWNSEND

RICK CROWLEY - City Manager

NOTES:

- ALL REFERENCES TO "CITY" SHALL MEAN "CITY OF ROCKWALL".
- ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE CITY OF ROCKWALL AND NORTH TEXAS COUNCIL OF GOVERNMENT STANDARD SPECIFICATIONS, 3RD ADDITION.
- CITY OF ROCKWALL STANDARD DETAIL SHEETS INCORPORATED HEREIN BY REFERENCE.
- STREET LIGHTS ARE NOT PART OF THIS PROJECT AND WILL BE INSTALLED IN THE FUTURE BY OTHERS

A001	COVER SHEET
A100	RIGHT-OF-WAY BEDDICATION PLAT (1 OF 2)
A101	RIGHT-OF-WAY BEDDICATION PLAT (2 OF 2)
A201	GENERAL NOTES
A202	GENERAL NOTES
A203	TOPOGRAPHIC LEGEND
P001	TYPICAL SECTIONS
P002	TYPICAL SECTIONS FOR SPRINGER ROAD
P003	PAVING DETAILS AND CURB DETAILS
P004	PAVING DETAILS
P101	DISCOVERY BLVD. PAVING PLAN AND PROFILE STA. 8+30.57 TO 16+00
P102	DISCOVERY BLVD. PAVING PLAN AND PROFILE STA. 16+00 TO 24+00
P103	DISCOVERY BLVD. PAVING PLAN AND PROFILE STA. 24+00 TO 28+29.59
P104	SPRINGER ROAD PAVING PLAN AND PROFILE STA. 0+00 TO 8+00
P105	SPRINGER ROAD PAVING PLAN AND PROFILE STA. 8+00 TO 16+00
P106	SPRINGER ROAD PAVING PLAN AND PROFILE STA. 16+00 TO 24+00
P107	SPRINGER ROAD PAVING PLAN AND PROFILE STA. 24+00 TO 32+00
P108	DATA DRIVE PAVING PLAN AND PROFILE STA. 1+59.12 TO 9+00 (ALTERNATE BID)
P109	DATA DRIVE PAVING PLAN AND PROFILE STA. 9+00 TO 14+00 (ALTERNATE BID)
P110	DATA DRIVE PAVING PLAN AND PROFILE STA. 15+23 TO 24+00
P111	DATA DRIVE PAVING PLAN AND PROFILE STA. 24+00 TO 31+25.98
P112	CAPITAL BOULEVARD PAVING PLAN AND PROFILE STA. 0+00 TO 8+00
P113	CAPITAL BOULEVARD PAVING PLAN AND PROFILE STA. 8+00 TO 16+00
P114	CAPITAL BOULEVARD PAVING PLAN AND PROFILE STA. 16+00 TO 24+00
P115	CAPITAL BOULEVARD PAVING PLAN AND PROFILE STA. 24+00 TO 28+20.86
P116	DRIVEWAYS SOUTH OF SPRINGER ROAD
P117	DRIVEWAYS SOUTH OF SPRINGER ROAD
P200	SIGNAGE LAYOUT
P201	STRIPING PLAN FOR DISCOVERY BLVD & DATA DRIVE
P202	GUARDRAIL LAYOUT
P301	TXDOT COMBINATION RAIL TYPE C221 (1 OF 3)
P302	TXDOT COMBINATION RAIL TYPE C221 (2 OF 3)
P303	TXDOT COMBINATION RAIL TYPE C221 (3 OF 3)
P304	TXDOT METAL BEAM GUARDRAIL GF (31) - II
P305	TXDOT SINGLE GUARDRAIL TERMINAL SGT (7) 31 - II
P306	TXDOT METAL BEAM GUARDRAIL GF (31) DAT - II
T101	TRAFFIC CONTROL PLAN
T201	TXDOT TRAFFIC CONTROL PLAN TOP (1-1) - 12
T202	TXDOT BARRICADE & TEMPORARY SIGN NOTED BG (4) - 13
T203	TXDOT BARRICADE & SIGN SUPPORT BG (5) - 13
T204	TXDOT BARRICADE & CONSTRUCTION TYPICAL SIGN SUPPORT BG (5) - 13
S101	STREET LIGHT LAYOUT
S201	STREET LIGHT DETAILS
G101	DITCH OUT GRADING FOR CULVERT 'U'
G102	DITCH OUT GRADING FOR CULVERT 'V'
G103	DITCH OUT GRADING FOR CULVERT 'W'
G104	GRADING SOUTH OF SPRINGER TO LATERAL 'C-16'
D101	OVERALL DRAINAGE AREA MAP
D102	OVERALL DRAINAGE AREA MAP
D103	OVERALL DRAINAGE AREA MAP
D201	DRAINAGE AREA CALCULATIONS
D202	100 YR INLET CALCULATIONS
D203	STORM DRAIN HYDRAULIC CALCULATIONS
D204	STORM DRAIN HYDRAULIC CALCULATIONS
D205	STORM DRAIN HYDRAULIC CALCULATIONS
D206	STORM DRAIN CULVERT HYDRAULIC CALCULATIONS
D301	STORM DRAIN PLAN AND PROFILE LINES 'E', & 'I' (DATA DRIVE/SPRINGER ROAD)
D302	STORM DRAIN PLAN AND PROFILE LINE 'M' (DATA DRIVE)
D303	STORM DRAIN PLAN AND PROFILE LINE 'T' (DATA DRIVE)
D304	STORM DRAIN PLAN AND PROFILE CULVERTS 'U' & 'W' (DATA DRIVE, CAPITAL BOULEVARD)
D305	STORM DRAIN PLAN AND PROFILE CULVERT 'V' & LINE 'X' (CAPITAL BOULEVARD)
D306	STORM DRAIN PLAN AND PROFILE LINE 'Y' (CAPITAL BOULEVARD)
D307	STORM DRAIN PLAN AND PROFILE LINE 'Z' STA 0+00 TO STA 8+00 (CAPITAL BOULEVARD)
D308	STORM DRAIN PLAN AND PROFILE LINE 'Z' STA 8+00 TO STA 11+31.50 (CAPITAL BOULEVARD)
D309	STORM DRAIN PLAN AND PROFILE LATERAL 'C16' STA 0+00 TO STA 6+80.16 (SPRINGER ROAD)
D310	STORM DRAIN PLAN AND PROFILE LATERAL 'C3.2' STA 0+00 TO STA 0+38.11 (SPRINGER ROAD)
D401	STORM DRAIN LATERALS LINES 'A', 'B', & 'C' (DISCOVERY BLVD, SPRINGER RD)
D402	STORM DRAIN LATERALS LINES 'C', 'D', & 'M' (SPRINGER RD, DATA DRIVE)
D403	STORM DRAIN LATERALS LINES 'P' & 'T' (DATA DRIVE)
D404	STORM DRAIN LATERALS LINE 'X' & 'Y' (CAPITAL BOULEVARD)
D405	STORM DRAIN LATERALS LINE 'Z' (CAPITAL BOULEVARD)
D501	TXDOT DETAIL CONCRETE HEADWALLS (PW-0)
D502	TXDOT DETAIL CONCRETE WINGWALLS (PW-1)
D503	TXDOT DETAIL TYPE M J-BOX WITH ACCESS (MH-M)
D504	TXDOT SINGLE BOX CULVERTS (SCC-7-1)
D505	TXDOT SINGLE BOX CULVERTS (SCC-7-2)
D506	TXDOT MULTIPLE BOX CULVERTS (MC-5-20-1)
D507	TXDOT MULTIPLE BOX CULVERTS (MC-5-20-2)
D508	TXDOT MULTIPLE BOX CULVERTS (MC-6-16-1)
D509	TXDOT MULTIPLE BOX CULVERTS (MC-6-16-2)
D510	TXDOT HORIZONTAL INLET TYPE H WITH GRATE (IL-H-G-1)
D511	TXDOT HORIZONTAL INLET TYPE H WITH GRATE (IL-H-G-2)
D512	N.C.T.C.O.G. CURB INLET DETAILS
U101	WATER LINE 'B' LAYOUT
U102	WATER LINE 'D' LAYOUT STA. 0+00 TO STA. 8+00
U103	WATER LINE 'D' LAYOUT STA. 8+00 TO STA. 20+50
U104	WATER LINE 'D' LAYOUT STA. 20+50 TO STA. 32+78.18
U201	SANITARY SEWER 'C' PLAN-PROFILE STA. 0+00 TO STA. 9+50
U202	SANITARY SEWER 'C' PLAN-PROFILE STA. 9+50 TO STA 18+00
U203	SANITARY SEWER 'C' PLAN-PROFILE STA. 18+00 TO STA 26+50
U204	SANITARY SEWER 'C' PLAN-PROFILE STA. 26+50 TO STA 33+45.90
E901	STORM WATER POLLUTION PREVENTION PLAN NOTES
E902	STORM WATER POLLUTION PREVENTION PLAN NOTES
E903	STORM WATER POLLUTION PREVENTION PLAN NOTES
E904	EROSION CONTROL WORK SHEETS
E101	EROSION CONTROL PLAN
E102	EROSION CONTROL PLAN
E103	EROSION CONTROL BMP DESCRIPTIONS
E201	EROSION CONTROL DETAILS
E202	EROSION CONTROL DETAILS

**RECORD
DRAWING
02/02/2015**

TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

DATE: 01-21-2015
W.A. No. I2209

TIME 16:28 FILE: A201-GENNOTES 1-12209.dwg

PAVEMENT & JOINT SEALING NOTES

- ALL CONCRETE FOR PAVEMENT SHALL BE CLASS "F" AND HAVE A MINIMUM 4,200 PSI COMPRESSIVE STRENGTH AT 28 DAYS WITH 4 TO 6 PERCENT AIR ENTRAINMENT UNLESS OTHERWISE NOTED. PAVEMENT MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE APPLICABLE SECTIONS OF THE "STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, 3RD EDITION" PREPARED BY THE NORTH CENTRAL TEXAS COUNCIL OF GOVERNMENTS. SLIP FORMED CONCRETE SHALL HAVE A MAXIMUM SLUMP OF THREE INCHES. HAND-PLACED CONCRETE SHALL HAVE A MAXIMUM FIVE-INCH SLUMP. ALL REINFORCEMENT SHALL BE CHAIRED. 6.5 SACK MIN. FOR MACHINE PLACE OR 7.0 SACK MIN. FOR HAND PLACE.
- THE JOINTING SHALL CONFORM TO THE LOCATIONS AND DETAILS SHOWN ON THESE PLANS. SPECIFIC SAWED CONTRACTION OR CONSTRUCTION JOINT LOCATIONS ARE NOT SHOWN. THE CONTRACTOR SHALL SUBMIT A LAYOUT INDICATING THE SAWED JOINT LOCATIONS TO BE REVIEWED AND APPROVED BY THE ENGINEER. ISOLATION JOINTS SHALL BE PROVIDED AT ALL MANHOLE RIMS, LIGHT STANDARDS AND OTHER SIMILAR INSTALLATIONS. EXPANSION JOINT LOCATIONS HAVE BEEN INDICATED ON PAVING AND DIMENSIONAL CONTROL PLANS.
- PROVIDE SAWED JOINTS AT MAXIMUM 15-FOOT SPACING FOR EIGHT-INCH CONCRETE. DO NOT PLACE SAWED JOINT LONGITUDINALLY ALONG LOW POINT OR AT GUTTER LINE. SAWING OF JOINTS SHALL BEGIN AS SOON AS CONCRETE HAS HARDENED SUFFICIENTLY TO PERMIT SAWING WITHOUT EXCESSIVE RAVELING. COMPLETE ALL SAWED JOINTS BEFORE UNCONTROLLED SHRINKAGE CRACKING OCCURS.
- DO NOT PLACE SAND OR SELECT FILL BENEATH CONCRETE PAVEMENT, SIDEWALKS, DRIVE APPROACHES OR HANDICAP RAMPS FOR LEVEL UP COURSE. UTILIZE COMPACTED NATIVE MATERIALS.
- BACKFILL ALL CURBS TO EDGE OF SUBGRADE WITH ON-SITE CLAY SOILS. COMPACT TO 95% TO 100% OF STANDARD PROCTOR DENSITY AT OR ABOVE OPTIMUM MOISTURE CONTENT.
- CONTRACTOR SHALL SAW-CUT TIE-INS AT EXISTING CURBS AS NECESSARY TO INSURE SMOOTH TRANSITIONS. CONTRACTOR SHALL SAW-CUT AND TRANSITION TO MEET EXISTING PAVEMENT AS NECESSARY TO INSURE POSITIVE DRAINAGE. (TYP. ALL INTERSECTIONS)
- ALL EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN PAVED AREAS SHALL BE SEALED IN ACCORDANCE WITH THESE SPECIFICATIONS AND THE JOINT SEALING MANUFACTURERS RECOMMENDATIONS.
- CLEAN ALL JOINTS PRIOR TO PLACEMENT OF JOINT SEALING MATERIAL IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.
- PROVIDE BACKER RODS FOR JOINTS WITHOUT PRE-MOLDED JOINT MATERIAL IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. INSTALL CERA-ROD MANUFACTURED BY W.R. MEADOWS OR EQUAL.
- EXPANSION AND ISOLATION JOINT MATERIAL TO BE PRE-MOLDED EXPANSION JOINT MATERIAL AS RECOMMENDED BY JOINT SEALING MANUFACTURER WITH JOINT CAP TO PROTECT SEALANT RESERVOIR.
- TYPICALLY, JOINT SEALING MATERIAL IS PLACED BELOW SURFACE OF CONCRETE TO NEAR FULL LEVEL. CERTAIN PRODUCTS SUCH AS SOFT SEAL ARE RECOMMENDED TO BE PLACED TO FULL LEVEL. REFER TO MANUFACTURERS RECOMMENDATIONS.
- THE CONTRACTOR SHALL CONSTRUCT ALL DRIVEWAY APPROACHES IN CONFORMANCE WITH APPLICABLE CITY STANDARD ORDINANCES AND REQUIREMENTS. CONTRACTOR SHALL CONFIRM APPLICABLE DRIVEWAY OR ACCESS PERMITS HAVE BEEN OBTAINED PRIOR TO CONSTRUCTION.
- ALL DIMENSIONS ARE TO BACK OF CURB, UNLESS NOTED OTHERWISE.
- ALL COORDINATES ARE TO BACK OF CURB, UNLESS NOTED OTHERWISE.
- ALL EDGE OF PAVEMENT WITH NO CURB SHALL BE THICKENED EDGE.
- NOTIFY CITY INSPECTOR 72 HOURS BEFORE WORK BEGINS OR EARLIER IF REQUIRED BY PERMITS.

WALKWAY, MARKING, AND SIGNAGE NOTES

- ALL PEDESTRIAN WALKWAYS UTILIZED FOR DISABLED ACCESS ROUTE SHALL

- CONFORM TO LOCAL, STATE, AND FEDERAL REGULATIONS INCLUDING THE "STATE OF TEXAS PROGRAM FOR THE ELIMINATION OF ARCHITECTURAL BARRIERS", "TEXAS ACCESSIBILITY STANDARDS" (TAS) AND THE "AMERICANS WITH DISABILITIES ACT OF 1990" (ADA).
- THE CONTRACTOR SHALL OBTAIN ALL REQUIRED CITY PERMITS AND NOTIFY THE CITY PRIOR TO CONSTRUCTING PUBLIC SIDEWALKS.
 - UNLESS REQUIRED OTHERWISE BY CITY REGULATIONS, ALL WALKWAYS SHALL BE CONSTRUCTED OF MINIMUM 3,000 PSI CONCRETE AND A MINIMUM CEMENT CONTENT OF 5.5 SACKS PER CUBIC YARD. ALL SIDEWALKS SHALL BE REINFORCED WITH A MINIMUM OF #3 BARS AT 18-INCH CENTERS EACH WAY LOCATED AT THE CENTER OF THE THICKNESS. THE STEEL SHALL BE PLACED ON CHAIR SUPPORTS BEFORE CONCRETE PLACEMENT. IF NECESSARY, DURING CONCRETE PLACEMENT, THE STEEL SHALL BE PULLED UP TO INSURE THE PROPER LOCATION OF REINFORCEMENT.
 - WALKWAYS SHALL BE CONSTRUCTED TO THE LINE AND GRADE INDICATED ON THE PLANS OR THE TYPICAL LOCATIONS SHOWN ON THE PAVING PLANS IN RELATION TO PROPOSED CURB. SEE PAVEMENT NOTE #1 ABOVE.
 - PRIVATE SIDEWALKS SHALL BE CONSTRUCTED ON NATIVE MATERIALS. DO NOT PLACE SAND UNDER PRIVATE SIDEWALKS OR HANDICAP RAMPS FOR LEVEL UP COURSE. PUBLIC SIDEWALKS SHALL BE CONSTRUCTED ACCORDING TO CITY DETAILS.
 - FORMS SET FOR SIDEWALKS SHALL BE TRUE TO LINE AND GRADE AND SHALL PROVIDE A SLOPE OF 1/4 INCH PER FOOT ACROSS THE SIDEWALK UNLESS INDICATED OTHERWISE ON THE PLANS. FORMS SHALL BE SET TO PROVIDE FOR A FULL DEPTH OF CONCRETE INDICATED ON THE PLANS AND FORMS SHALL REMAIN IN PLACE A MINIMUM OF 24 HOURS. UPON REMOVAL OF THE FORM WORK, THE CONTRACTOR SHALL IMMEDIATELY BACKFILL THE EDGES OF THE WALK FOR A MINIMUM OF ONE FOOT (1') EACH SIDE OF THE WALK.
 - 24-INCH BY 3/4-INCH DIAMETER ASPHALT-COATED DOWELS WITH FIVE INCH BY 13/16-INCH DOWEL SLEEVE SHALL BE INSTALLED ON 16-INCH CENTERS, ALONG WITH REDWOOD EXPANSION BOARD AND SEALING COMPOUND AS PER STANDARD EXPANSION JOINT DETAIL SHEET ALONG PERIMETER OF WHEEL CHAIR RAMP AND SIDEWALK.
 - PROVIDE 15-INCH MINIMUM LAP BETWEEN REINFORCING STEEL IN STREET AND REINFORCING STEEL IN WHEEL CHAIR RAMP.
 - SUBGRADE FOR WALKWAYS ABUTTING CURBS, WITHIN PARKING ISLAND AREAS OR BETWEEN THE PARKING AREA AND BUILDING, SHALL BE PLACED ON COMPACTED FILL OR FIRM COMPACTED EXCAVATED GRADE. FILLS FOR SIDEWALKS SHALL CONFORM TO THE SAME REQUIREMENTS AS CONTROLLED DENSITY FILLS IN PARKING AREAS WITH THE COMPACTED MATERIAL EXTENDING A MINIMUM 18 INCHES BEYOND THE WALKWAY.
 - FOR WALKWAYS SIX FEET IN WIDTH OR LESS, GROOVED OR SAWED CONTRACTION JOINTS SHALL BE MADE AT UNIFORM INTERVALS EQUAL TO THE WIDTH OF THE SIDEWALK. ON WALKWAYS GREATER THAN SIX FEET IN WIDTH, CONTRACTION JOINTS SHALL BE SAWED. CONTRACTION JOINTS SHALL ONLY BE SEALED WHERE CONCENTRATED RUNOFF OCCURS IN PARKING AREAS, ENTRANCES AND WALKWAYS AT THE BUILDING. SEAL PARKING LOT CONCENTRATED RUNOFF AREAS SAME AS PARKING PAVEMENT. SEAL WALKWAYS WITHIN 50 FEET OF BUILDING WITH "DECK-O-SEAL" AS MANUFACTURED BY W.R. MEADOWS OR EQUAL.
 - CONCRETE FINISH SHALL BE BROOMED FOR ALL WALKWAYS LESS THAN SIX FEET IN WIDTH AND MINOR ACCESS ROUTES GREATER THAN EIGHT FEET IN WIDTH. ALL HANDICAP ACCESS RAMPS SHALL HAVE SURFACE TEXTURE FINISH COMPLYING WITH ADA AND TAS GUIDELINES 302 AND 405.4.
 - JOINT SEALING MATERIAL FOR WALKWAY AND EXPANSION JOINTS IN THE INTERNAL PARKING AREAS AND EXTERNAL OPEN AREAS SHALL BE "HI SPEC" MANUFACTURED BY W.R. MEADOWS OR EQUAL.
 - CLEAN ALL JOINTS IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATION PRIOR TO SEALING.
 - ALL SIGNS, PAVEMENT MARKINGS AND OTHER TRAFFIC CONTROL DEVICES SHALL CONFORM TO THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES.
 - ALL PAVEMENT MARKINGS SHALL BE FOUR INCHES WIDE, COLOR WHITE UNLESS INDICATED OTHERWISE ON THE DRAWINGS. STRIPING TO BE TWO COATS OF PAINT. SECOND COAT TO THE APPLIED IMMEDIATELY PRIOR TO OBTAINING A CERTIFICATE OF OCCUPANCY.
 - A MINIMUM CLEARANCE OF TWO (2) FEET SHALL BE MAINTAINED BETWEEN THE FACE OF CURB AND ANY PART OF A TRAFFIC SIGN.
 - CONTRACTOR SHALL FURNISH AND INSTALL ALL PAVEMENT MARKINGS AS SHOWN ON THE PLANS.
 - CONTRACTOR SHALL COORDINATE INSTALLATION OF ALL SIGNS, PAVEMENT MARKINGS AND OTHER TRAFFIC CONTROL DEVICES WITH OTHER CONTRACTORS ON THE SITE.

TESTING

- REFER TO PROJECT GEOTECHNICAL RECOMMENDATIONS FOR FREQUENCY OF CONCRETE TESTING AND TEST METHODS. ALL CONCRETE SHALL BE TESTED. IF TESTING IS NOT ADDRESSED IN GEOTECHNICAL RECOMMENDATIONS PROVIDE AS PER NCTCOG ITEM 303.7.3 AND ITEM 702.2.4.

UTILITY NOTES

- THIS SHEET IS FOR SANITARY SEWER, WATER LINE AND STORM DRAINAGE CONSTRUCTION ONLY. DO NOT USE FOR GRADING CONSTRUCTION.
- ALL PIPE LENGTHS ARE HORIZONTAL DISTANCES AND ARE APPROXIMATE.
- CONTRACTOR SHALL PROVIDE ALL THE MATERIALS AND APPURTENANCES NECESSARY FOR THE COMPLETE INSTALLATION OF THE UTILITIES. ALL PIPE AND FITTINGS SHALL BE INSPECTED BY THE CITY PRIOR TO BEING COVERED. THE INSPECTOR MUST ALSO BE PRESENT DURING PRESSURE TESTING AND DISINFECTION OF MAINS AND HIS SIGNATURE OF APPROVAL IS REQUIRED.
- ALL WORK SHALL COMPLY WITH ALL APPLICABLE CODES, REGULATIONS AND/OR LOCAL STANDARDS IMPOSED BY LOCAL UTILITY AND THE CITY.
- CONTRACTOR SHALL MAKE ARRANGEMENTS WITH THE LOCAL UTILITY AUTHORITY FOR CONNECTION TO THE EXISTING MAINS.
- ALL FIRE HYDRANTS ARE SIX-INCH DIAMETER WITH A 6-INCH DIAMETER LINE AND A SIX-INCH DIAMETER SHUT OFF VALVE. FIRE HYDRANTS SHALL BE SET SUCH THAT NOZZLE CONNECTIONS FACE THE FIRE LANE. FIRE HYDRANTS SHALL BE SET MAX 8 FEET FROM BACK OF CURB, OR TWO FOOT INSIDE RIGHT-OF-WAY UNLESS OTHERWISE NOTED.
- ALL WATER LINES SHALL HAVE A MINIMUM COVER OF 48 INCHES ABOVE TOP OF PIPE, UNLESS NOTED OTHERWISE.
- CONTRACTOR SHALL ADJUST LOCATION OF PROPOSED WATER LINES AS REQUIRED TO AVOID CONFLICTS WITH STORM SEWER OR OTHER UTILITIES.
- THRUST BLOCKS SHALL BE PROVIDED AT ALL "TEES, ELBOWS AND BENDS" OF SUFFICIENT SIZE TO COMPLY WITH MINIMUM STANDARDS OF N.F.P.A.-24 FOR EXISTING SOIL CONDITIONS.
- ALL GATE VALVES TO BE PROVIDED WITH CAST IRON BOXES. SIZE OF GATE VALVE (WHERE TAP IS MADE INTO EXISTING WATER LINE) WILL BE DETERMINED BY THE WATER DEPARTMENT.
- SHOULD LATENT SOIL CONDITIONS NECESSITATE, CONTRACTOR SHALL INSTALL SPECIAL SUPPORTS FOR PIPING AND/OR APPURTENANCES INCLUDING THE REMOVAL OF UNSUITABLE MATERIAL AND BACKFILLING WITH GRAVEL OR OTHER MATERIAL. CONTRACTOR SHALL PERFORM ANY SUCH WORK AS DIRECTED BY THE CIVIL ENGINEER AND/OR SOILS ENGINEER AT NO ADDITIONAL COST TO THE OWNER.
- THE SITE UTILITY CONTRACTOR SHALL COOPERATE AND WORK WITH OTHER CONTRACTORS ON THE SITE.
- ALL MANHOLES OVER FIVE FEET IN DEPTH SHALL HAVE A STANDARD ECCENTRIC CONE.
- ALL MATERIALS SHALL BE U.L. LISTED AND FACTORY MUTUAL APPROVED UNLESS DIRECTED OTHERWISE BY THE ENGINEER.
- EXISTING UTILITIES AND UNDERGROUND FACILITIES INDICATED ON THESE PLANS ARE BASED ON REFERENCE INFORMATION SUPPLIED BY VARIOUS OWNERS OF THE FACILITIES. THE ENGINEER DOES NOT ACCEPT THE RESPONSIBILITY FOR THE GRAPHICAL REPRESENTATION OF THE UTILITIES SHOWN, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES AND UNDERGROUND FACILITIES, BOTH HORIZONTALLY AND VERTICALLY, PRIOR TO CONSTRUCTION, TO TAKE NECESSARY PRECAUTIONS IN ORDER TO PROTECT ALL FACILITIES ENCOUNTERED, AND TO NOTIFY THE ENGINEER PROMPTLY OF ALL CONFLICTS OF THE WORK WITH EXISTING FACILITIES. THE CONTRACTOR SHALL PRESERVE AND PROTECT ALL EXISTING UTILITIES FROM DAMAGE DURING CONSTRUCTION. ANY DAMAGE BY THE CONTRACTOR TO EXISTING UTILITIES SHALL BE REPAIRED BY THE CONTRACTOR AT HIS EXPENSE.
- UTILITY CONTRACTOR SHALL VERIFY WITH LOCAL AND STATE AUTHORITIES THAT ALL EXISTING STREET LIGHT AND TRAFFIC SIGNAL WIRES HAVE BEEN LOCATED PRIOR TO CONSTRUCTION.
- ALL TRENCHES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE OCCUPATIONAL SAFETY AND HEALTH ACT OF 1970 AND THE STANDARDS THEREIN AND APPLICABLE STATE AND LOCAL REGULATIONS.
- CONTRACTOR SHALL REFER TO SITE GEOTECHNICAL REPORT FOR RECOMMENDATIONS ON COMPACTING AND BACKFILLING TRENCHES. IF NO TRENCH COMPACTION RECOMMENDATIONS ARE PROVIDED, TRENCHES BENEATH OR WITHIN FIVE FEET OF PAVEMENT SHALL BE COMPACTED TO 95% OF STANDARD PROCTOR DENSITY AT A MOISTURE CONTENT BETWEEN OPTIMUM TO FIVE PERCENT ABOVE OPTIMUM. TRENCHES OUTSIDE OF PAVED AREAS SHALL BE COMPACTED TO A MINIMUM 90% OF STANDARD PROCTOR

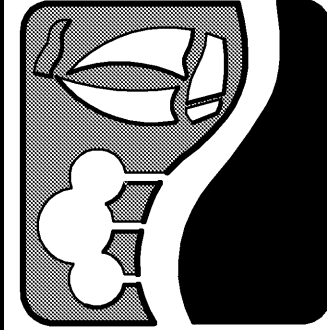
- DENSITY AT A MOISTURE CONTENT BETWEEN OPTIMUM TO FIVE PERCENT ABOVE OPTIMUM.
- TRENCHES SHALL BE TESTED FOR COMPACTION AT A MINIMUM OF ONE TEST PER 300 LINEAR FEET PER LAYER.
 - ALL STORM DRAIN PIPE TO BE CLASS III REINFORCED CONCRETE PIPE (RCP) UNLESS OTHERWISE NOTED IN PLANS.
 - ALL CONSTRUCTION SHALL COMPLY WITH THE CITY STANDARD CONSTRUCTION DETAILS AND THE "STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION, 3RD EDITION" PREPARED BY THE NORTH CENTRAL TEXAS COUNCIL OF GOVERNMENTS.
 - CONTRACTOR SHALL ADJUST ALL EXISTING AND PROPOSED SURFACE UTILITIES TO MATCH FINISHED GRADES.
 - ALL WATER MAINS SHALL BE C900 PVC, CLASS 200 DR-14.
 - CONCRETE ENCASEMENT SHALL BE INSTALLED WHERE SHOWN IN THE PLANS OR AS DIRECTED BY THE ENGINEER OR THE INSPECTOR. THIS WORK SHALL BE SUBSIDIARY TO THE COST OF THE UTILITY AND NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE.
 - BLUE EMS DISCS SHALL BE INSTALLED EVERY 250' AND AT EVERY CHANGE IN DIRECTION, VALVE, AND SERVICE.
 - GREEN EMS DISCS SHALL BE INSTALLED AT EVERY MANHOLE, CLEAN OUT, AND SERVICE.
 - RAVEN EPOXY COATING, OR APPROVED EQUAL, SHALL BE INSTALLED IN ALL NEW MANHOLES AND IN EXISTING MANHOLES BEING MODIFIED.
 - PIPE USED FOR WASTEWATER COLLECTION SYSTEMS SHALL BE PVC PIPE CONFORMING TO THE STANDARD SPECIFICATIONS FOR CONSTRUCTION. THE WASTEWATER PIPELINE SHALL BE SDR-35, AND SHALL HAVE A MINIMUM EARTH COVER OF THREE FEE (3'). FOR DEPTHS OF TEN FEET (10') OR GREATER, THE WASTEWATER PIPELINE SHALL BE A MINIMUM OF SDR-26.

SPECIAL NOTES

- CONTRACTOR SHALL TAKE SPECIAL CARE NOT TO INSTALL A JOINT LONGITUDINALLY WITHIN THE CENTERLINE OF A LOW POINT SWALE.
- ALL REFERENCES TO THE "CITY" SHALL REFER TO "THE CITY OF ROCKWALL".
- SEE SHEETS E001-E003 FOR EROSION CONTROL NOTES.

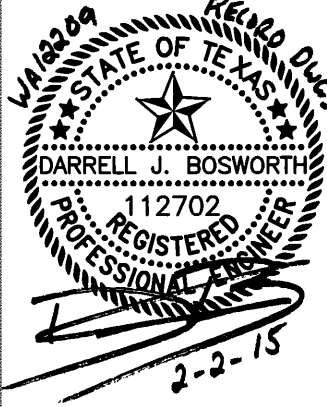
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TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.



ROCKWALL
TECHNOLOGY
PARK
PHASE IV

GENERAL NOTES



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WIER & ASSOCIATES, INC.
LAST SHEET EDIT
DATE: 02-14-2014
WA# 12209
SHEET NO.
A201

PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS

701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

GENERAL GRADING & DRAINAGE NOTES

- EXISTING UTILITIES AND UNDERGROUND FACILITIES INDICATED ON THESE PLANS ARE BASED ON REFERENCE INFORMATION SUPPLIED BY VARIOUS OWNERS OF THE FACILITIES. THE ENGINEER DOES NOT ACCEPT THE RESPONSIBILITY FOR THE GRAPHICAL REPRESENTATION OF THE UTILITIES SHOWN. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL EXISTING UTILITIES AND UNDERGROUND FACILITIES, BOTH HORIZONTALLY AND VERTICALLY, PRIOR TO CONSTRUCTION, TO TAKE NECESSARY PRECAUTIONS IN ORDER TO PROTECT ALL FACILITIES ENCOUNTERED, AND TO NOTIFY THE ENGINEER PROMPTLY OF ALL CONFLICTS OF THE WORK WITH EXISTING FACILITIES. THE CONTRACTOR SHALL PRESERVE AND PROTECT ALL EXISTING UTILITIES FROM DAMAGE DURING CONSTRUCTION. ANY DAMAGE BY THE CONTRACTOR TO EXISTING UTILITIES SHALL BE REPAIRED BY THE CONTRACTOR AT HIS EXPENSE. EXISTING TOPOGRAPHIC INFORMATION SHOWN IS BASED ON IN-FIELD SURVEY PREPARED BY WIER & ASSOCIATES, INC. DURING THE MONTHS OF JUNE TO AUGUST 2013. (EXCLUDES BELOW GRADE PUBLIC UTILITY LOCATIONS PROVIDED BY UTILITY COMPANY AS DESCRIBED ABOVE.)
- NEW FINISHED CONTOURS SHOWN ARE TOP OF PAVING IN AREAS TO RECEIVE PAVEMENT AND TOP OF TOPSOIL IN AREAS TO BE SEEDED.
- AREAS OUTSIDE OF PAVING LIMITS SHOWN TO BE SEEDED SHALL RECEIVE MINIMUM FOUR (4) INCHES OF TOPSOIL (OR TO DEPTH INDICATED ON LANDSCAPE ARCHITECT PLANS). THIS TOPSOIL TO BE PLACED AND LEVELED BY THE GRADING CONTRACTOR. THIS MATERIAL MAY BE STOCKPILED DURING STRIPPING OPERATIONS.
- ROUGH GRADING ELEVATIONS SHALL BE AS FOLLOWS:
 - FOUR INCHES BELOW FINISHED CONTOURS IN SEEDED AREAS.
 - THE DEPTH OF PAVEMENT, TYPICALLY SIX TO EIGHT INCHES, BELOW FINISHED CONTOURS IN PAVED AREAS, UNLESS OTHERWISE NOTED.
- GRADING CONTRACTOR SHALL NOTIFY AND COOPERATE WITH ALL UTILITY COMPANIES OR FIRMS HAVING FACILITIES ON OR ADJACENT TO THE SITE BEFORE DISTURBING, ALTERING, REMOVING, RELOCATING, ADJUSTING, OR CONNECTING TO SAID FACILITIES. CONTRACTOR SHALL PAY ALL COSTS IN CONNECTION WITH THE ALTERATION OF OR RELOCATION OF THE FACILITIES. CONTRACTOR SHALL RAISE OR LOWER TOPS OF EXISTING MANHOLES AS REQUIRED TO MATCH FINISHED GRADES IN CONFORMANCE WITH CITY STANDARDS.
- GRADING CONTRACTOR SHALL COOPERATE AND WORK WITH ALL OTHER CONTRACTORS PERFORMING WORK ON THIS PROJECT TO INSURE PROPER AND TIMELY COMPLETION OF THIS PROJECT.
- THE GRADING CONTRACTOR SHALL USE WHATEVER MEASURES ARE REQUIRED TO PREVENT SILT AND CONSTRUCTION DEBRIS FROM FLOWING ONTO ADJACENT PROPERTIES. THIS CAN BE ACCOMPLISHED BY SMALL TEMPORARY SEDIMENT PONDS, SILT FENCES OF STEEL WIRE AND BURLAP OR BARRIERS OF CEDAR TREES AND/OR BALES OF STRAW. CONTRACTOR SHALL COMPLY WITH ALL LOCAL EROSION, CONSERVATION AND SILTATION ORDINANCES. CONTRACTOR SHALL REMOVE ALL TEMPORARY EROSION CONTROL STRUCTURES UPON COMPLETION OF PERMANENT DRAINAGE FACILITIES AND THE ESTABLISHMENT OF A STAND OF GRASS SUFFICIENT TO PREVENT EROSION.
- FOR THE WORK ON THE STATE OR CITY RIGHT-OF-WAY, THE GRADING CONTRACTOR SHALL:
 - NOT STORE MATERIAL, EXCESS DIRT OR EQUIPMENT ON THE SHOULDERS OF PAVEMENT, IN CASE OF MULTI-LANE HIGHWAYS, IN THE MEDIAN STRIPS. THE PAVEMENT SHALL BE KEPT FREE FROM ANY MUD OR EXCAVATION WASTE FROM TRUCKS OR OTHER EQUIPMENT. ON COMPLETION OF THE WORK, ALL EXCESS MATERIAL SHALL BE REMOVED FROM THE RIGHT-OF-WAY.
 - SHALL PROVIDE ALL NECESSARY AND ADEQUATE SAFETY PRECAUTIONS SUCH AS SIGNS, FLAGS, LIGHTS, BARRICADES AND FLAGMEN AS REQUIRED BY THE LOCAL AUTHORITIES AND IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES. THE GRADING CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR AND HOLD HARMLESS THE TEXAS DEPARTMENT OF TRANSPORTATION, THE CITY, AND THE OWNER FROM ANY CLAIMS FOR DAMAGE DONE TO EXISTING

PRIVATE PROPERTY, PUBLIC UTILITIES OR TO THE TRAVELING PUBLIC.

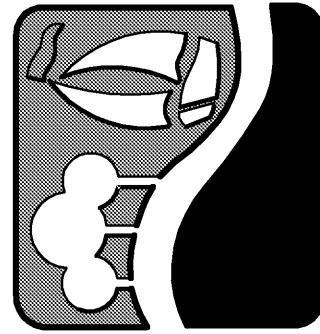
- SHALL COMPLETE THE WORK TO THE SATISFACTION OF THE CITY PUBLIC WORKS DEPARTMENT AND OBTAIN A LETTER FROM THE DEPARTMENT STATING THAT THE WORK UNDER PUBLIC JURISDICTION IS ACCEPTABLE.
- POST NECESSARY BONDS AS REQUIRED BY THE CITY AND/OR STATE.
- GRADING CONTRACTOR SHALL TAKE ALL AVAILABLE PRECAUTIONS TO CONTROL DUST. CONTRACTOR SHALL CONTROL DUST BY SPRINKLING, BY APPLYING CALCIUM CHLORIDE OR BY OTHER METHODS AS DIRECTED BY ENGINEER AND/OR OWNER'S REPRESENTATIVE AT NO ADDITIONAL COST TO OWNER.
- REFER TO PAVING DETAILS FOR TYPE OF PAVING AND BASE TO BE USED.
- GRADING CONTRACTOR IS RESPONSIBLE FOR REMOVING ANY EXISTING STRUCTURES, FENCES, DEBRIS OR TREES REMAINING ON SITE, UNLESS NOTED OTHERWISE ON PLANS AND SHALL COORDINATE WITH GENERAL CONTRACTOR.
- GRADING CONTRACTOR TO COMPLY WITH ALL STATE AND LOCAL SEDIMENT CONTROL AND AIR POLLUTION ORDINANCES OR RULES.
- A QUALIFIED SOILS LABORATORY SHALL DETERMINE THE SUITABILITY OF THE EXISTING SUBGRADE AND EXISTING ON-SITE MATERIAL PRIOR TO BEGINNING ANY FILLING OPERATION.
- UNSUITABLE EXCAVATED MATERIALS AND ALL WASTE RESULTING FROM CLEARING AND GRUBBING SHALL BE DISPOSED OF OFF-SITE BY GRADING CONTRACTOR.
- ALL EXCAVATING IS UNCLASSIFIED AND SHALL INCLUDE ALL MATERIALS ENCOUNTERED.
- BEFORE ANY MACHINE WORK IS DONE, CONTRACTOR SHALL STAKE OUT AND MARK THE ITEMS ESTABLISHED BY THE SITE PLAN. CONTROL POINTS SHALL BE PRESERVED AT ALL TIMES DURING THE COURSE OF THE PROJECT. LACK OF PROPER WORKING POINTS AND GRADE STAKES MAY REQUIRE CESSATION OF OPERATIONS UNTIL SUCH POINTS AND GRADES HAVE BEEN PLACED TO THE OWNER'S SATISFACTION. NO EXTENSION OF TIME WILL BE GRANTED FOR THE ABOVE.
- TEMPORARY EROSION CONTROL DEVICES TO BE INSTALLED PRIOR TO BEGINNING OF GRADING. CONTRACTOR SHALL MAINTAIN ALL TEMPORARY EROSION CONTROL DEVICES AND SHALL REMOVE SILT FROM BERM DITCHES, SILT DAMS AND SILT FENCES AS NEEDED.
- ALL DISTURBED AREAS SHALL BE HYDROMULCH SEEDED UNLESS OTHERWISE NOTED. ALL DISTURBED AREAS WITHIN EXISTING ROAD RIGHT-OF-WAY TO BE SODDED.
- THE CONTRACTOR SHALL PREVENT SOIL STABILIZATION TREATMENT FROM LEAVING THE SITE BY WAY OF STORMWATER RUNOFF WHICH MAY DAMAGE DOWNSTREAM WATER COURSES, LAKES OR PONDS. ANY DAMAGE TO WILDLIFE OR FISH KILLS SHALL BE CORRECTED BY THE CONTRACTOR AT HIS EXPENSE.
- MAINTAIN AS MUCH EXISTING VEGETATION AS POSSIBLE AS WELL AS RE-ESTABLISHING THE GROUND COVER AS EARLY AS POSSIBLE. GRASS BUFFER STRIPS SHALL BE LEFT AROUND THE PERIMETER TO AID IN FILTERING SEDIMENTATION. A DENSITY OF TEMPORARY OR PERMANENT GROUND COVER SUFFICIENT TO PREVENT EROSION SHALL BE ESTABLISHED ON ALL BERMS, SWALES AND SLOPES.
- ALL SITE GRADING AND EARTHWORK CONSTRUCTION SHALL COMPLY TO THE GEOTECHNICAL REPORT RECOMMENDATIONS
- ALL FILL TO BE COMPACTED TO A MIN. 95% USING A SHEEP'S FOOT ROLLER.

SPECIAL NOTES

- CONTRACTOR SHALL TAKE SPECIAL CARE NOT TO INSTALL A JOINT LONGITUDINALLY WITHIN THE CENTERLINE OF A LOW POINT SWALE.
- ALL REFERENCES TO THE "CITY" SHALL REFER TO "THE CITY OF ROCKWALL".
- SEE SHEETS E001-E003 FOR EROSION CONTROL NOTES.

RECORD
DRAWING
02/02/2015

TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.



ROCKWALL
TECHNOLOGY
PARK
PHASE IV

GENERAL NOTES



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DATE: 10-07-2013
WA# 12209

SHEET NO.
A202

PREPARED BY:
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EXISTING TOPOGRAPHIC LEGEND	
	ASPHALT PAVEMENT
	BOLLARD/GUARD POST
	DIMENSION TO BACK OF CURB
	CABLE TV
	CONTROL MONUMENT
	CONCRETE
	EDGE OF ASPHALT PAVEMENT
	ELEC BOX (GROUND)
	ELEC METER
	FIRE HYDRANT
	FIBER OPTIC CABLE
	GAS METER
	GAS MANHOLE
	GAS TEST STATION
	GUY WIRE
	CONCRETE HEADWALL
	IRRIGATION CONTROL VALVE
	IRON ROD FOUND
	IRON ROD SET
	LIGHT POLE
	POWER POLE
	POWER POLE W/LIGHT
	STORM DRAIN MANHOLE
	SPRINKLER HEAD
	SIGN
	SANITARY SEWER MANHOLE
	SANITARY SEWER CLEANOUT
	SOUTH WESTERN BELL TELEPHONE
	TELEPHONE PEDESTAL
	TELEPHONE SWITCH GEAR
	TRAFFIC SIGNAL BOX
	TRAFFIC SIGNAL POLE
	TRAFFIC SIGNAL CONTROLLER
	TRANSFORMER PAD
	WATER METER
	WATER VALVE
	OVERHEAD ELECTRIC LINE
	UNDERGROUND ELECTRIC LINE
	WATER LINE
	SANITARY SEWER LINE
	FIBER OPTIC LINE
	UNDERGROUND TELEPHONE
	OVERHEAD TELEPHONE
	UNDERGROUND GAS
	EXISTING CONCRETE STORM DRAIN LINE
	EXISTING CORRUGATED METAL STORM DRAIN LINE
	EXISTING FLOWLINE
	BARBED WIRE FENCE
	CHAIN LINK FENCE
	WOOD FENCE
	GUARD RAIL / BARRICADE
	EXISTING TREE LINE
	EXISTING TREE

PAVING PLAN LEGEND	
	PROPOSED CONC. PAVING
	BRICK PAVING
	LANDSCAPING
	COMPACTED FILL PER CITY STANDARDS
	EXISTING ASPHALT TO BE REMOVED
	EXISTING CONCRETE TO BE REMOVED
	EXISTING GRAVEL TO BE REMOVED
	DIRECTION OF TRAFFIC
	DIRECTION OF FLOW

GRADING PLAN LEGEND	
	PROPOSED CONTOURS
	EXISTING CONTOURS
	PROPOSED SPOT GRADES
	PROPOSED STORM DRAIN
	FLOW ARROWS
	T/C TOP OF CURB
	T/P TOP OF PAVEMENT
	H.P. HIGH POINT

DRAINAGE PLAN LEGEND	
	DRAINAGE AREA DESIGNATION DRAINAGE AREA (ACRES)
	WATERSHED LIMITS
	MAJOR DRAINAGE AREA DIVIDE
	MAJOR DRAINAGE AREA SUB-DIVIDE
	ZONING BOUNDARY
	FLOW DIRECTION ARROW
	LINE IDENTIFIED IN LINE TABLE
	CURVE IDENTIFIED IN CURVE TABLE
	INDICATES PROPOSED RECESSED CURB INLET
	INDICATES PROPOSED STANDARD CURB INLET
	INDICATES FUTURE INLET
	INDICATES EXISTING INLET
	INDICATES PROPOSED DROP INLET
	INDICATES PROPOSED JUNCTION BOX
	27" RCP PROPOSED STORM DRAIN
	27" RCP FUTURE STORM DRAIN
	PROPOSED SWALE

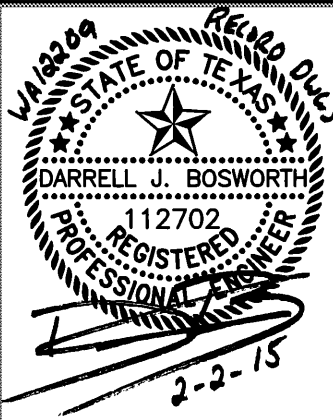
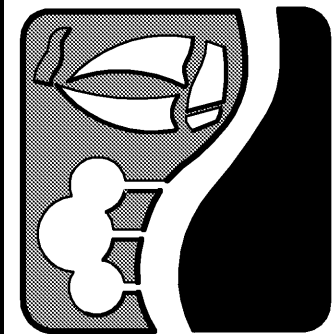
EROSION CONTROL LEGEND	
	LIMITS OF OPERATOR DAY TO DAY OPERATIONAL CONTROL
	PROPOSED SWALE
	INDICATES STABILIZED CONSTRUCTION ENTRANCE
	INDICATES REINFORCED SILT FENCE
	INDICATES ROCK BERM
	INDICATES DROP INLET PROTECTION
	INDICATES PROPOSED INLET TREATMENT
	STONE OVERFLOW STURCTURE
	EXISTING CONTOUR LINE
	640 PROPOSED CONTOUR LINE
	DRAINAGE AREA DIVIDE

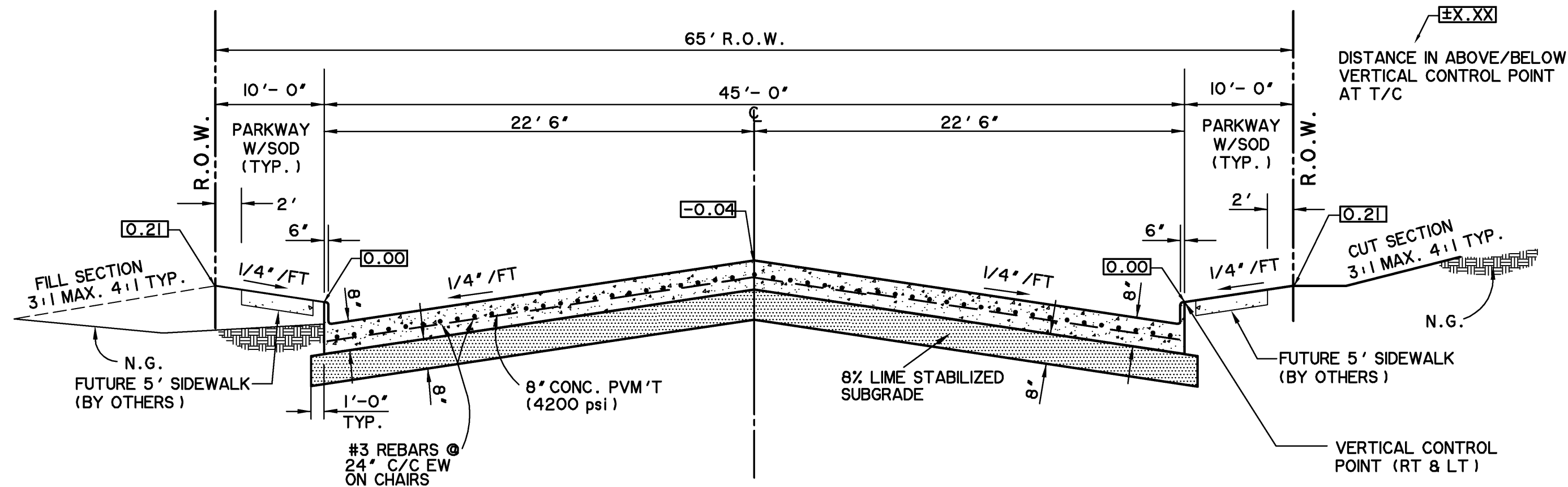
LEGEND	
	DRAINAGE AREA DIVIDE
	PROPOSED INLET
	PROP. STORMDRAIN
	PROP. FIELD INLET
	EXIST. INLET
	FUTURE INLET
	FUTURE STORMDRAIN
	DIRECTION OF FLOW

WATER AND SANITARY SEWER PLAN LEGEND	
	PROPOSED FIRE HYDRANT
	PROPOSED WATER METER
	PROPOSED WATER VALVE
	PROPOSED WATER
	PROPOSED SEWER
	PROPOSED SEWER MANHOLE
	EXISTING WATER LINE
	EXISTING WATER VALVE
	EXISTING WATER METER BOX
	EXISTING FIRE HYDRANT
	EXISTING LIGHT POLE
	EXISTING POWER POLE
	EXIST. SANITARY SEWER MANHOLE
	EXISTING SANITARY SEWER
	EXISTING INLET
	PROPOSED INLET
	FUTURE INLET

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02/02/2015**

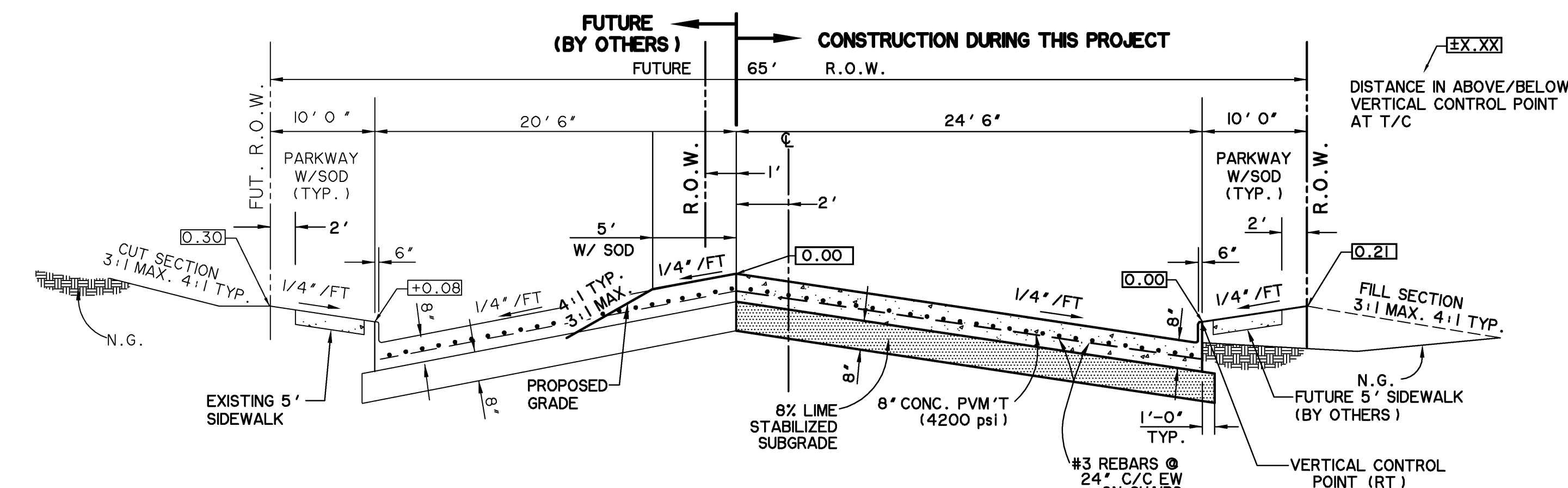
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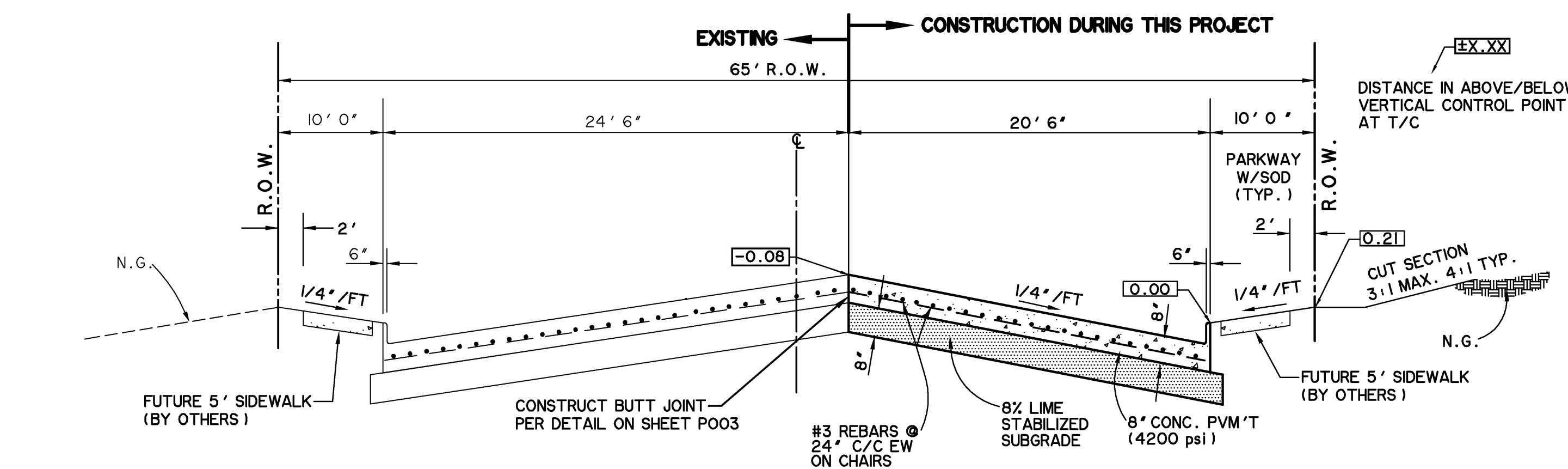
TYPICAL SECTION - 65' R.O.W.

CAPITAL BOULEVARD STA 0+00 TO 12+41.74
DATA DRIVE STA 15+23 TO 31+76
SCALE: NTS



TYPICAL SECTION - 65' R.O.W.

CAPITAL BOULEVARD STA 12+41.74 TO END
SCALE: NTS



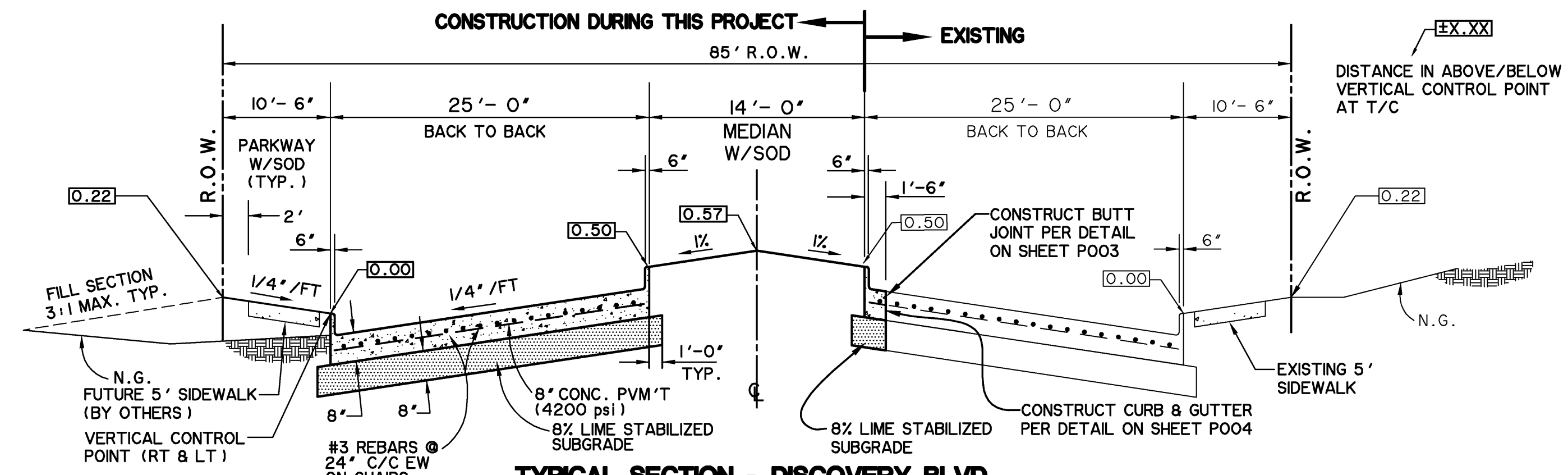
TYPICAL SECTION - 65' R.O.W.

DATA DRIVE STA 2+00 TO STA 14+00
SCALE: NTS

ALTERNATE BID

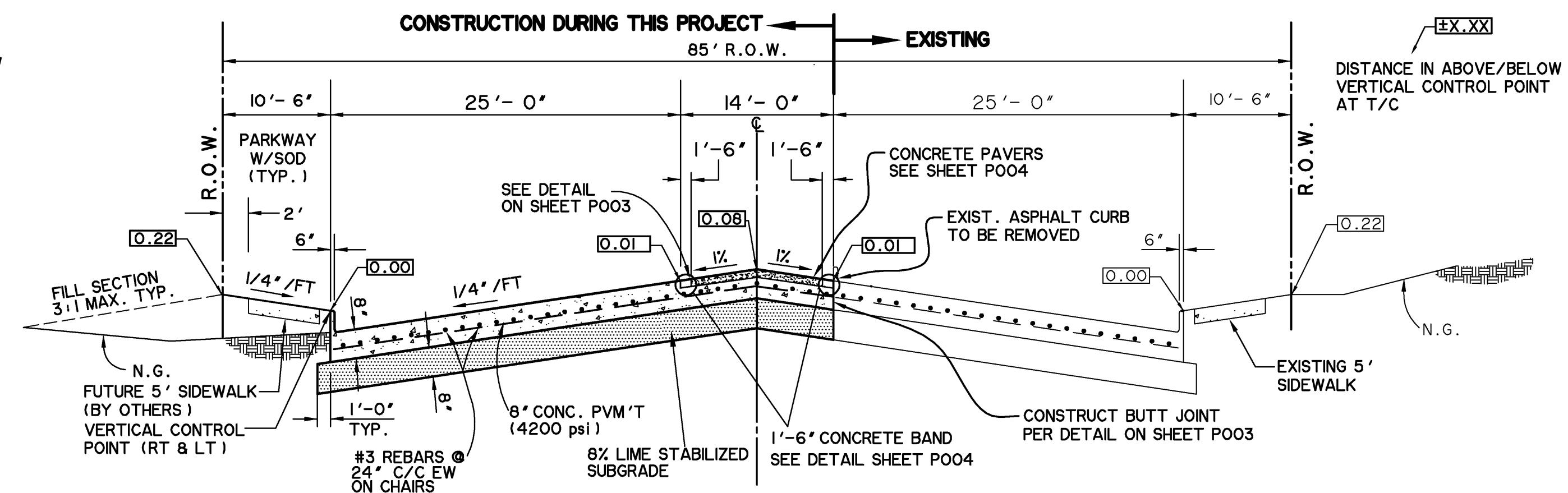
SEE SHEET A201
FOR PAVEMENT NOTES

- NOTES: 1. ALL CONSTRUCTION SHALL CONFORM TO THE CITY OF ROCKWALL STANDARD CONSTRUCTION DETAILS AND NCTCOG 3rd EDITION. TYPICAL SECTIONS ARE REPLACED BY THOSE SHOWN ON THIS SHEET.
2. PLACE MIN. 4" TOPSOIL FROM ON SITE SOURCE IN PARK WAYS, SLOPES, AND DISTURBED AREAS. SODDING WILL BE USED ON ALL PARKWAYS. HYDROMULCH SEED WITH BERMUDA GRASS (PER SPECIFICATIONS) WILL BE USED ON ALL SLOPES AND DISTURBED AREAS. TOPSOIL SHALL BE A SUBSIDIARY ITEM AND NO SEPERATE MEASUREMENT OR PAYMENT SHALL BE MADE.
3. ALL FILLS SHALL BE COMPACTED TO 95% MIN. PROCTOR DENSITY. FILLS SHALL BE PLACED IN 8" MAXIMUM LIFTS, AND COMPACTION SHALL BE ACCOMPLISHED BY THE USE OF A SHEEPS-FOOT ROLLER.
4. 0.50 DENOTES DISTANCE IN DECIMAL FEET ABOVE VERTICAL CONTROL POINT.



TYPICAL SECTION - DISCOVERY BLVD.

CURBED MEDIAN - LANDSCAPE POD
(DISCOVERY BOULEVARD)
STA 11+36.25 TO 28+08.53
SCALE: NTS



TYPICAL SECTION - DISCOVERY BLVD.

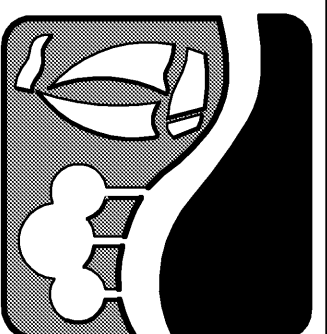
TWO-WAY LEFT TURN
WITH CONCRETE PAVERS
STA 11+36.25 TO 28+08.53
SCALE: NTS

**RECORD
DRAWING
02/02/2015**

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* ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OF DESIGN. *

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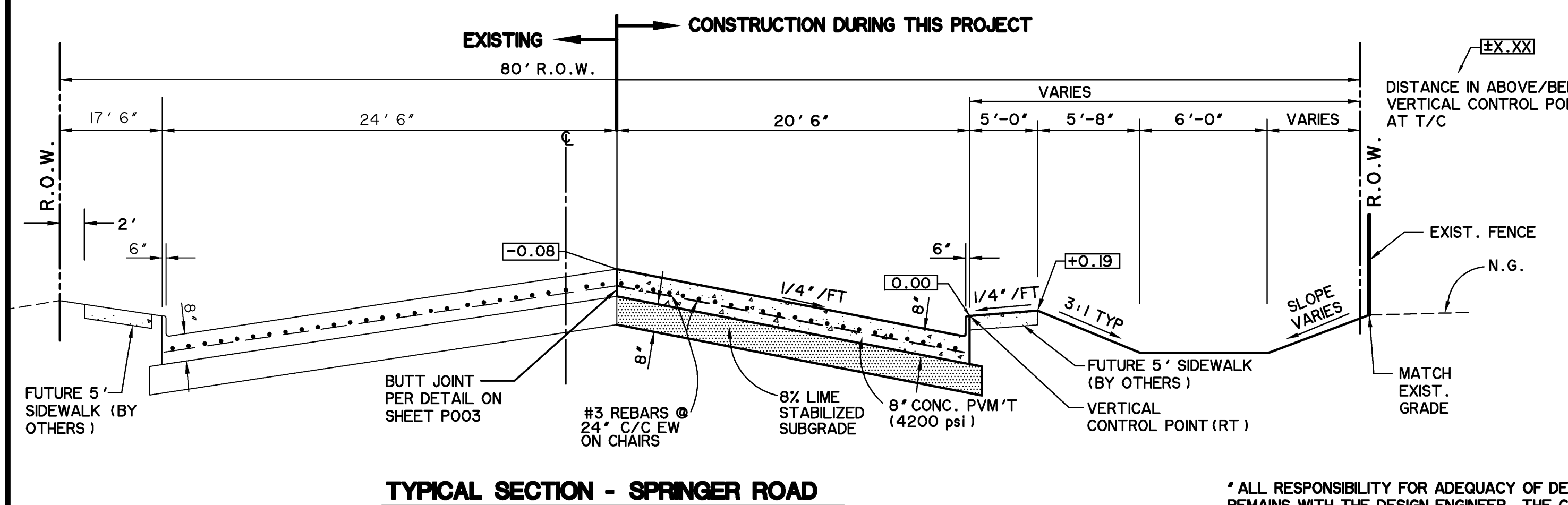
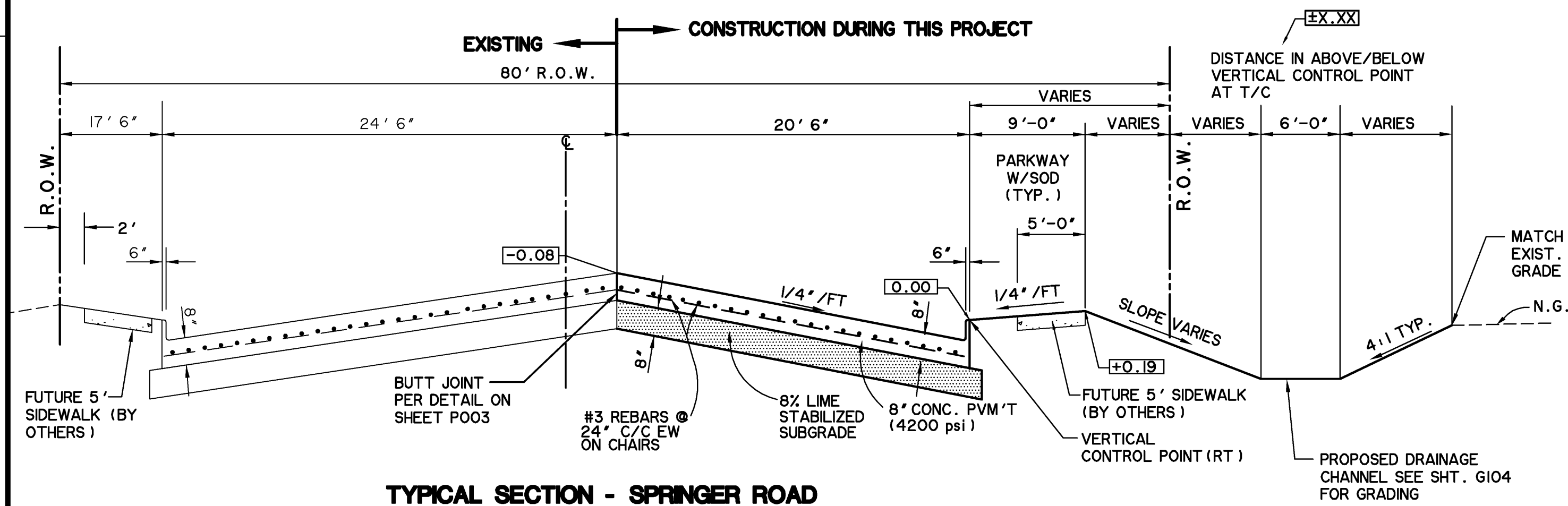
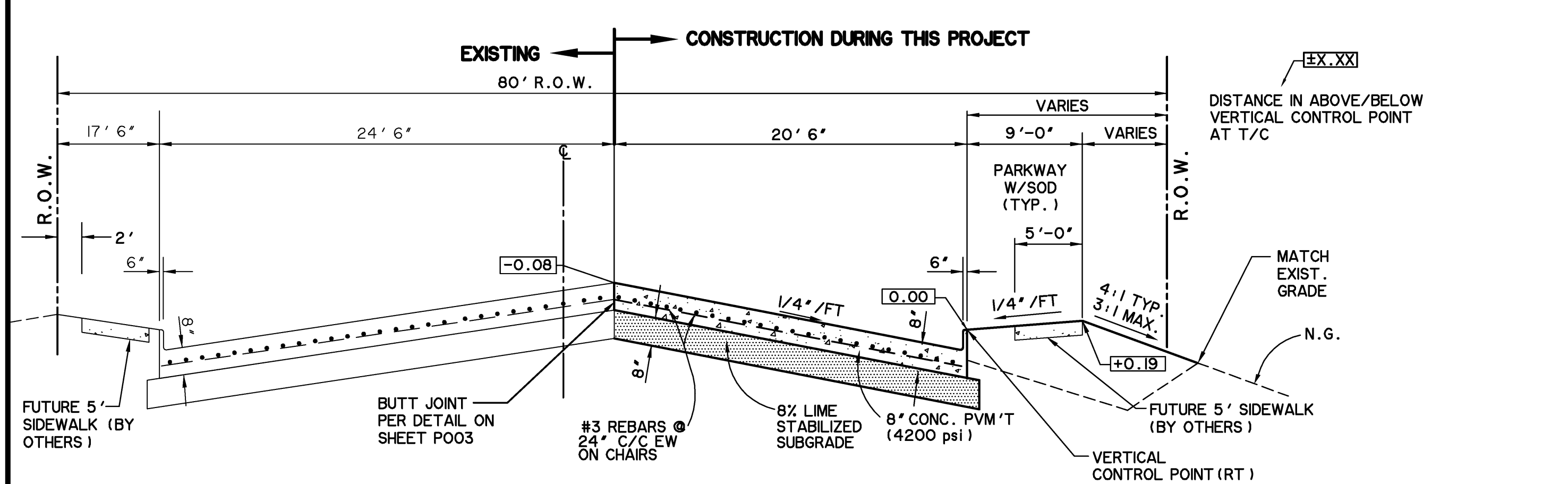
**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**TYPICAL
SECTIONS**

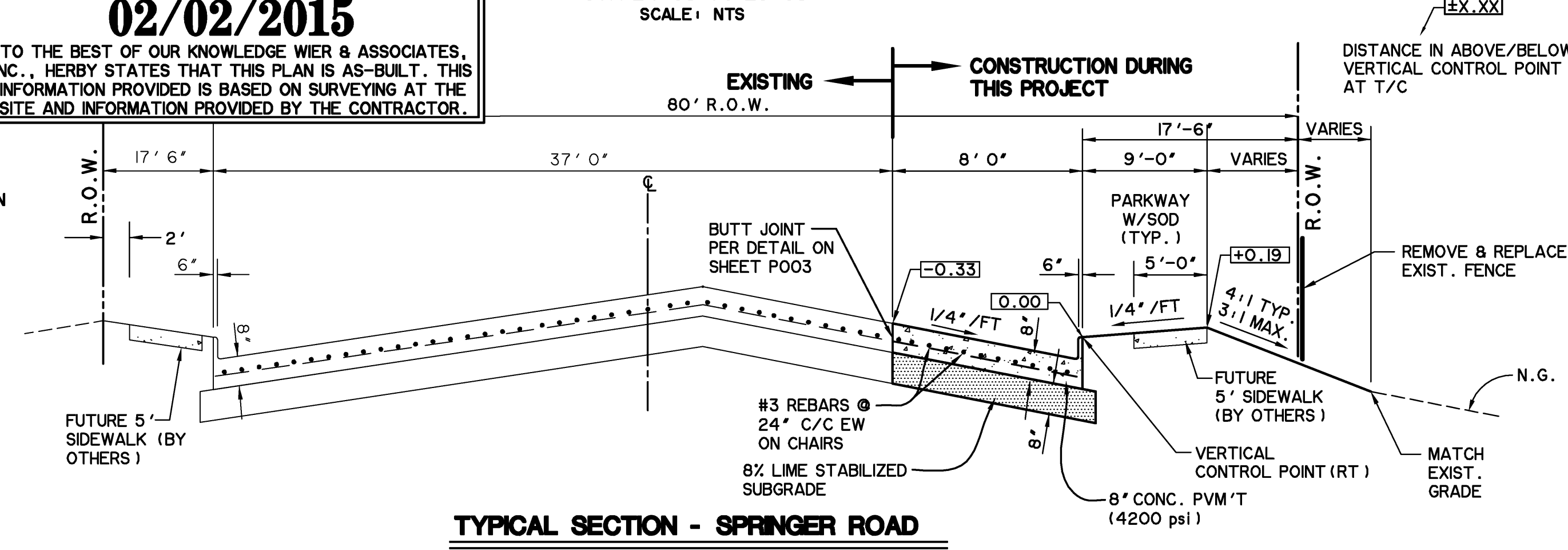
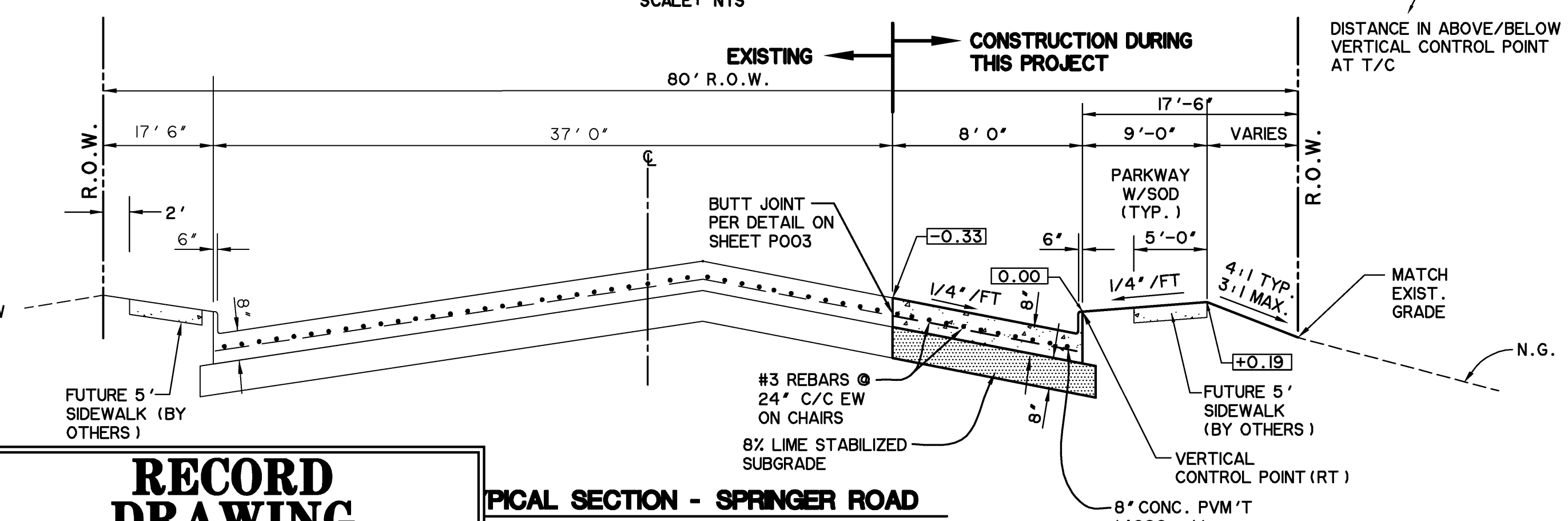
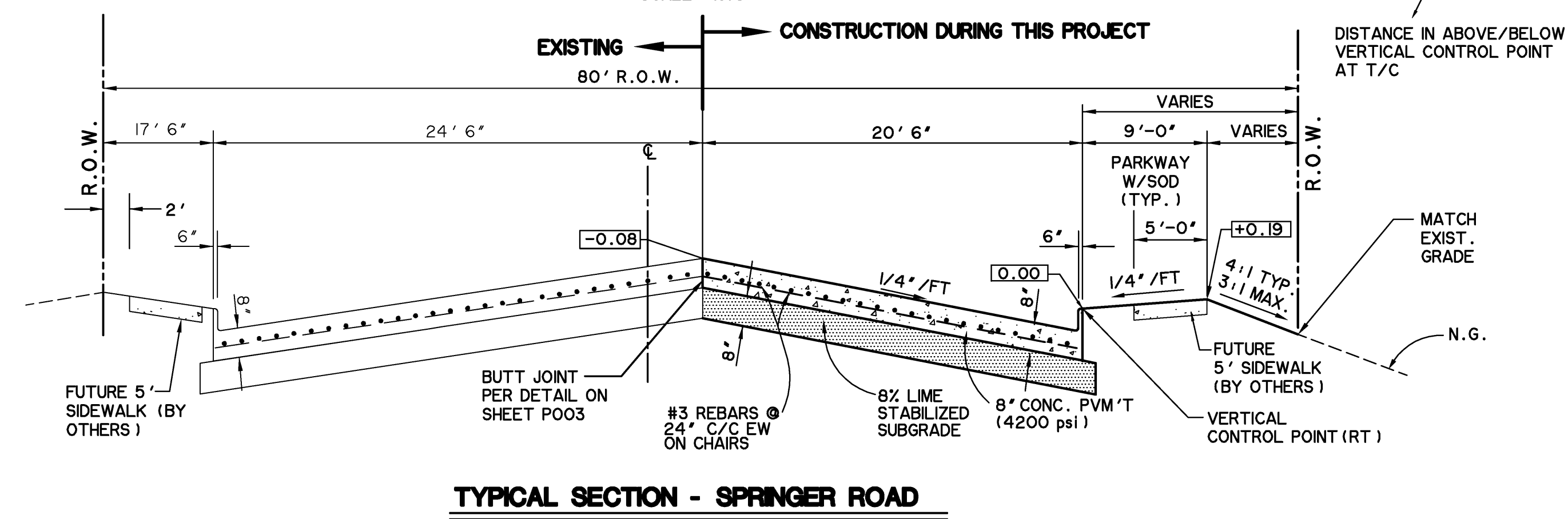
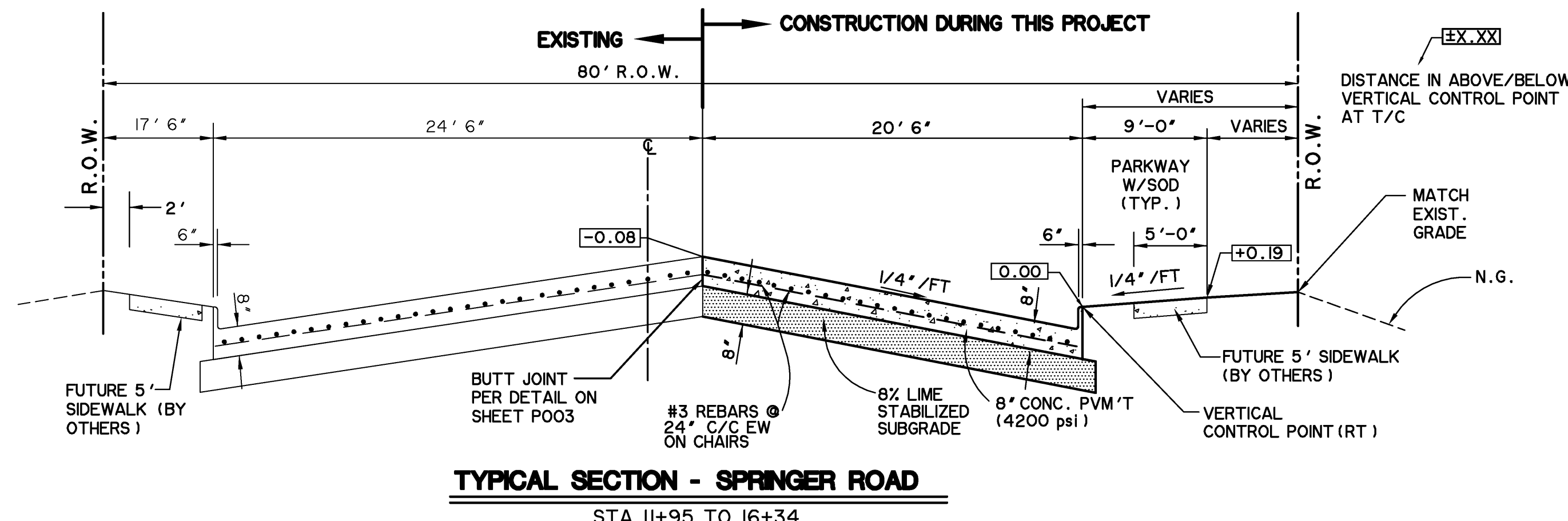


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LAST SHEET EDIT
DATE: 09-26-2013
WA# 12209
**SHEET NO.
P001**

TIME 12:00
FILE: P002-TYPESEC-SPRINGER-12209.dwg



- NOTES: 1. ALL CONSTRUCTION SHALL CONFORM TO THE CITY OF ROCKWALL STANDARD CONSTRUCTION DETAILS AND NCTCOG 3rd EDITION. TYPICAL SECTIONS ARE REPLACED BY THOSE SHOWN ON THIS SHEET.
2. PLACE MIN. 4" TOPSOIL FROM ON SITE SOURCE IN PARK WAYS, SLOPES, AND DISTURBED AREAS. SODDING WILL BE USED ON ALL PARKWAYS. HYDROMULCH SEED WITH BERMUDA GRASS (PER SPECIFICATIONS) WILL BE USED ON ALL SLOPES AND DISTURBED AREAS. TOPSOIL SHALL BE A SUBSIDIARY ITEM AND NO SEPARATE MEASUREMENT OR PAYMENT SHALL BE MADE.
3. ALL FILLS SHALL BE COMPACTED TO 95% MIN. PROCTOR DENSITY. FILLS SHALL BE PLACED IN 8" MAXIMUM LIFTS, AND COMPACTION SHALL BE ACCOMPLISHED BY THE USE OF A SHEEPS-FOOT ROLLER.
4. 0.50 DENOTES DISTANCE IN DECIMAL FEET ABOVE VERTICAL CONTROL POINT.



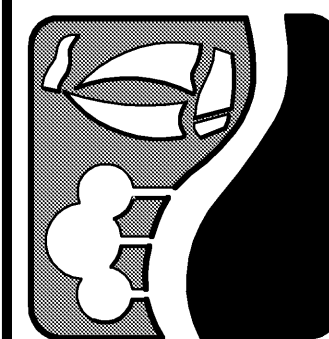
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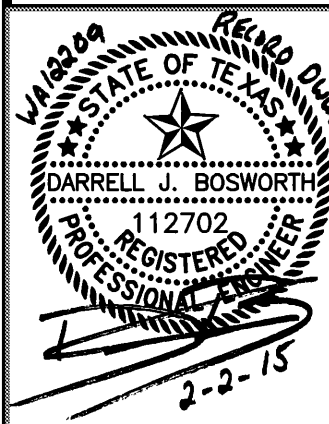
SEE SHEET A201
FOR PAVEMENT NOTES

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**ROCKWALL
TECHNOLOGY
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**TYPICAL
SECTIONS
SPRINGER ROAD**



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WA# 12209
**SHEET NO.
P002**



1. DOWEL BARS PLACED INTO EXISTING PAVEMENT SHALL BE DRILLED INTO PAVEMENT HORIZONTALLY BY USE OF A MECHANICAL ROD. DRILLING BY HAND IS NOT ACCEPTABLE. PUSHING DOWEL BARS INTO EXISTING PAVEMENT IS NOT ACCEPTABLE. FUSE DOWEL BARS IN EXISTING PAVING WITH EPOXY GROUT.
2. POLYETHYLENE FOAM BACKER ROD DOES NOT SIT ON BOTTOM OF SAW-CUT JOINT. PLACE AT DEPTH INDICATED IN DETAIL.
3. IF SEALANT PROTRUDES ABOVE THE SURFACE OF THE PAVEMENT, IT MUST BE REMOVED AND REPLACED.
4. SUBMIT MANUFACTURER'S LITERATURE FOR SEALANT. DOCUMENTING PRODUCT COMPLIES WITH ASTM SPECIFICATIONS AND PROVIDING MANUFACTURER'S RECOMMENDATIONS FOR APPLICATION. FOLLOW MANUFACTURER'S RECOMMENDATIONS ON USE OF THE PRODUCT.
5. THE CONSTRUCTION JOINT IS TO BE USED BETWEEN SEPARATE POURS OF PROPOSED PAVEMENT. NOTE THAT IT REQUIRES THE REINFORCEMENT TO BE EXTENDED THROUGH THE FORM TO TIE TO THE NEXT POUR. THE CONSTRUCTION JOINT IS THE UNION BETWEEN EXISTING CONCRETE PAVEMENT (STREET OR DRIVEWAY) AND PROPOSED PAVEMENT, UNLESS AN EXPANSION JOINT IS CALLED FOR.
6. JOINT SEALANTS SHALL BE INSTALLED SOON AFTER JOINTS ARE SAWED AND/OR COMPLETED. THE JOINTS SHALL BE SEALED BEFORE A RAIN EVENT OCCURS AFTER SAWING OR COMPLETING JOINT.



N. T. S.

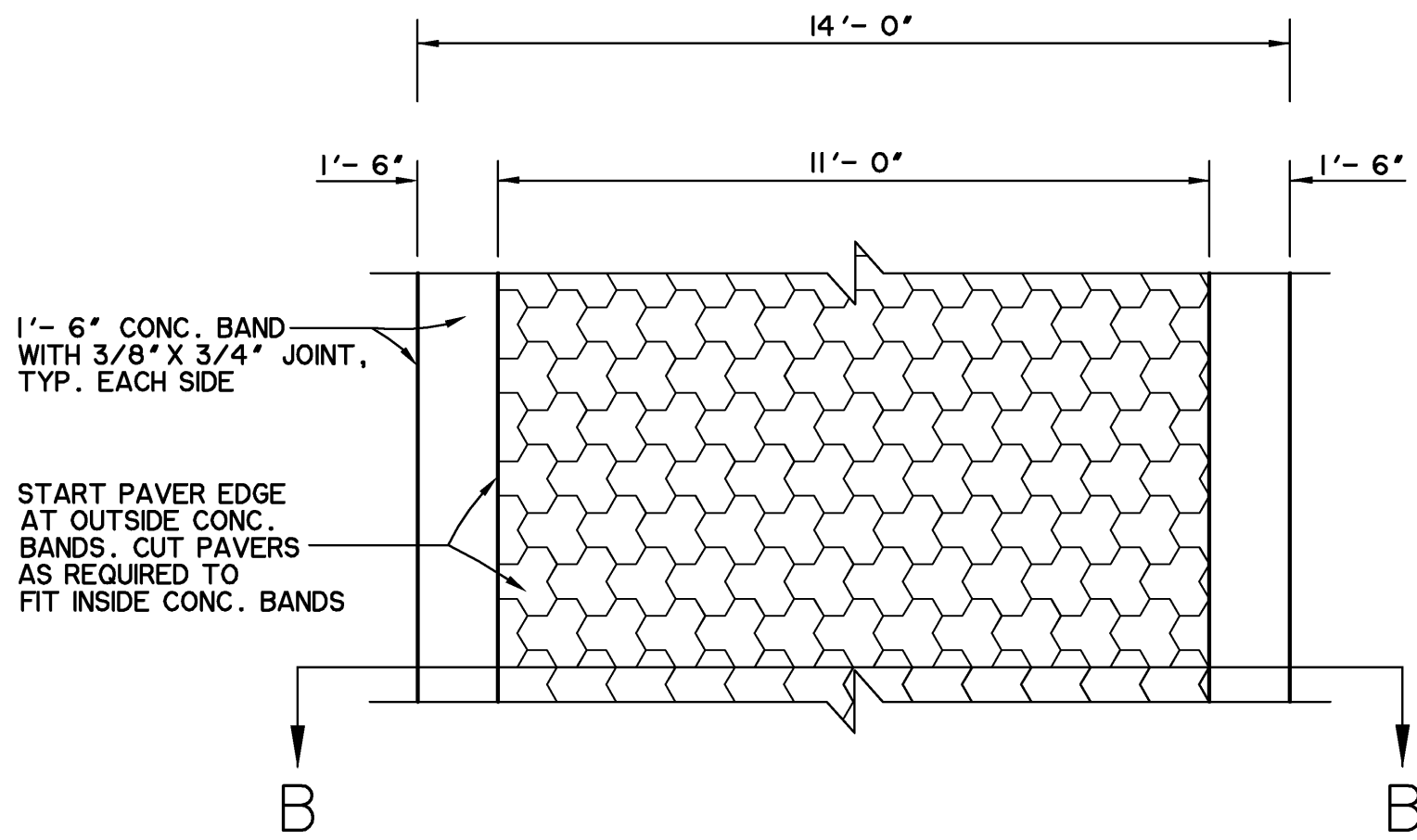


**SPECIAL NOTE
THIS PROJECT**



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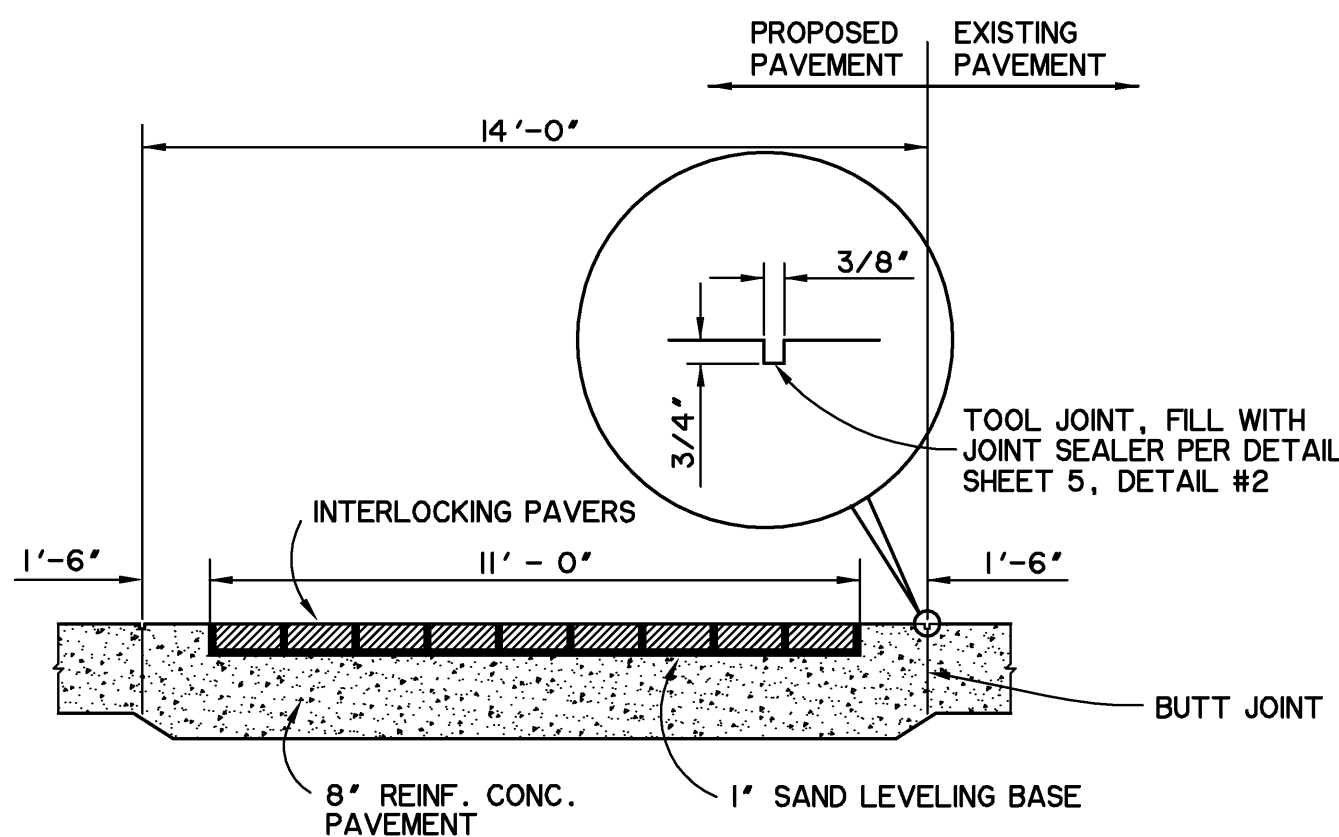
SHEET NO.
P003



DIRECTION OF PAVERS

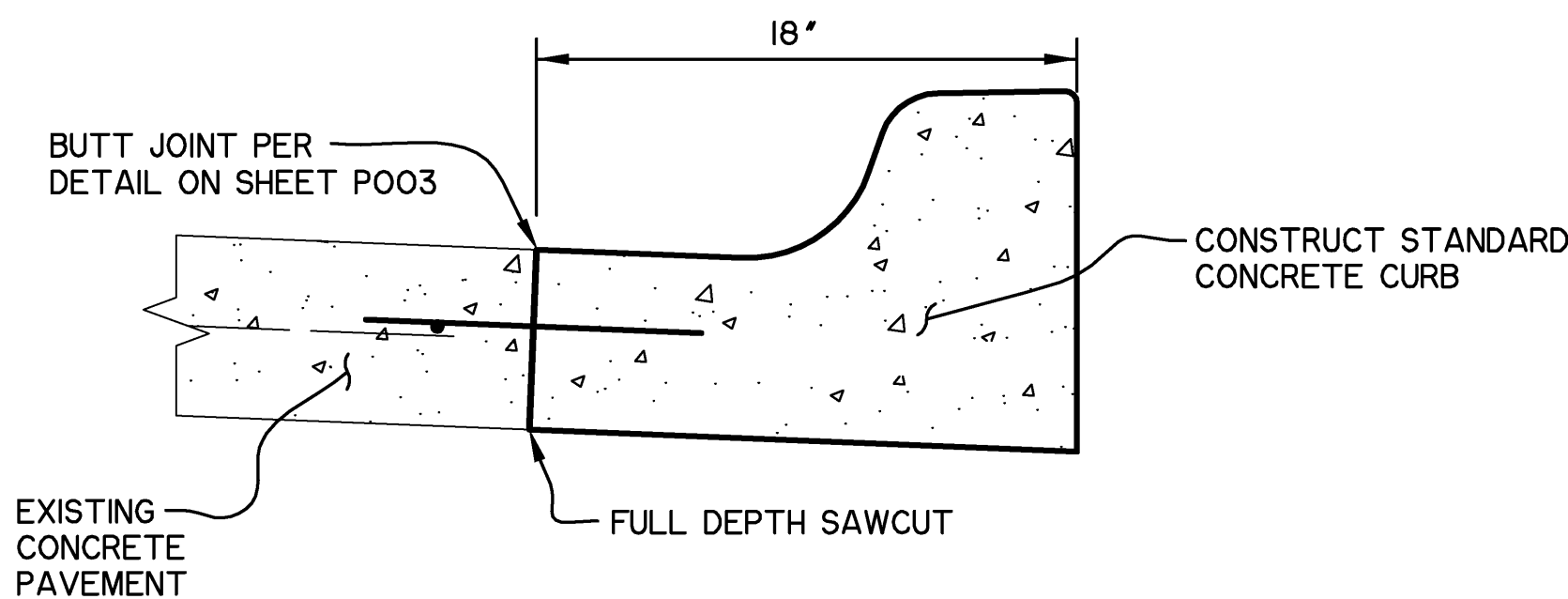
DISCOVERY BOULEVARD
SCALE: NTS

NOTE:
1. CONTRACTOR SHALL COORDINATE COLOR AND TYPE OF PAVER WITH THE OWNER AND ENGINEER. SUBMITTAL TO THE OWNER AND ENGINEER IS REQUIRED PRIOR TO ORDERING THE MATERIAL



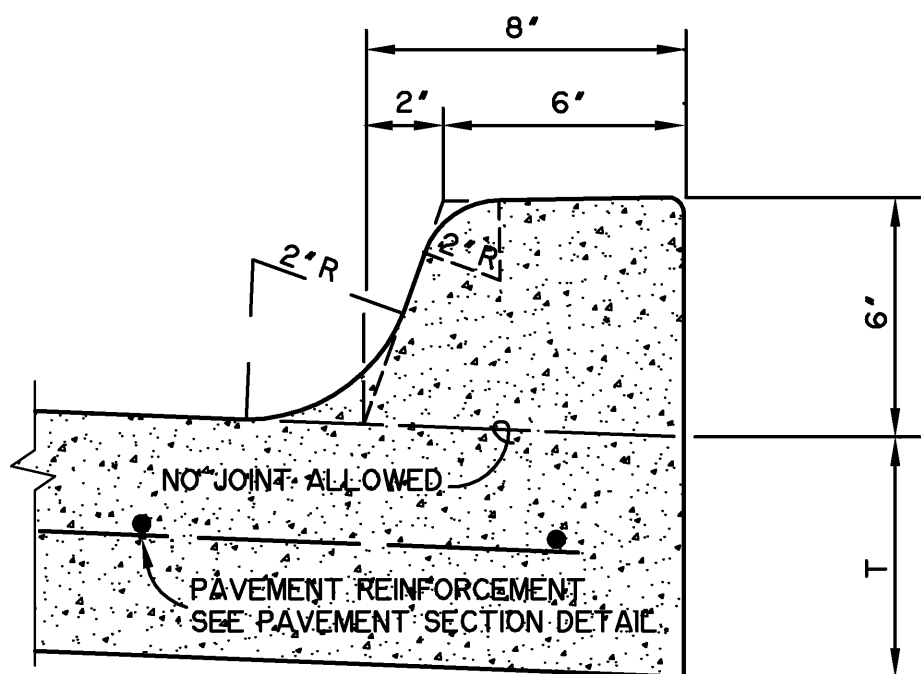
PAVER SECTION B-B

DISCOVERY BOULEVARD
SCALE: NTS



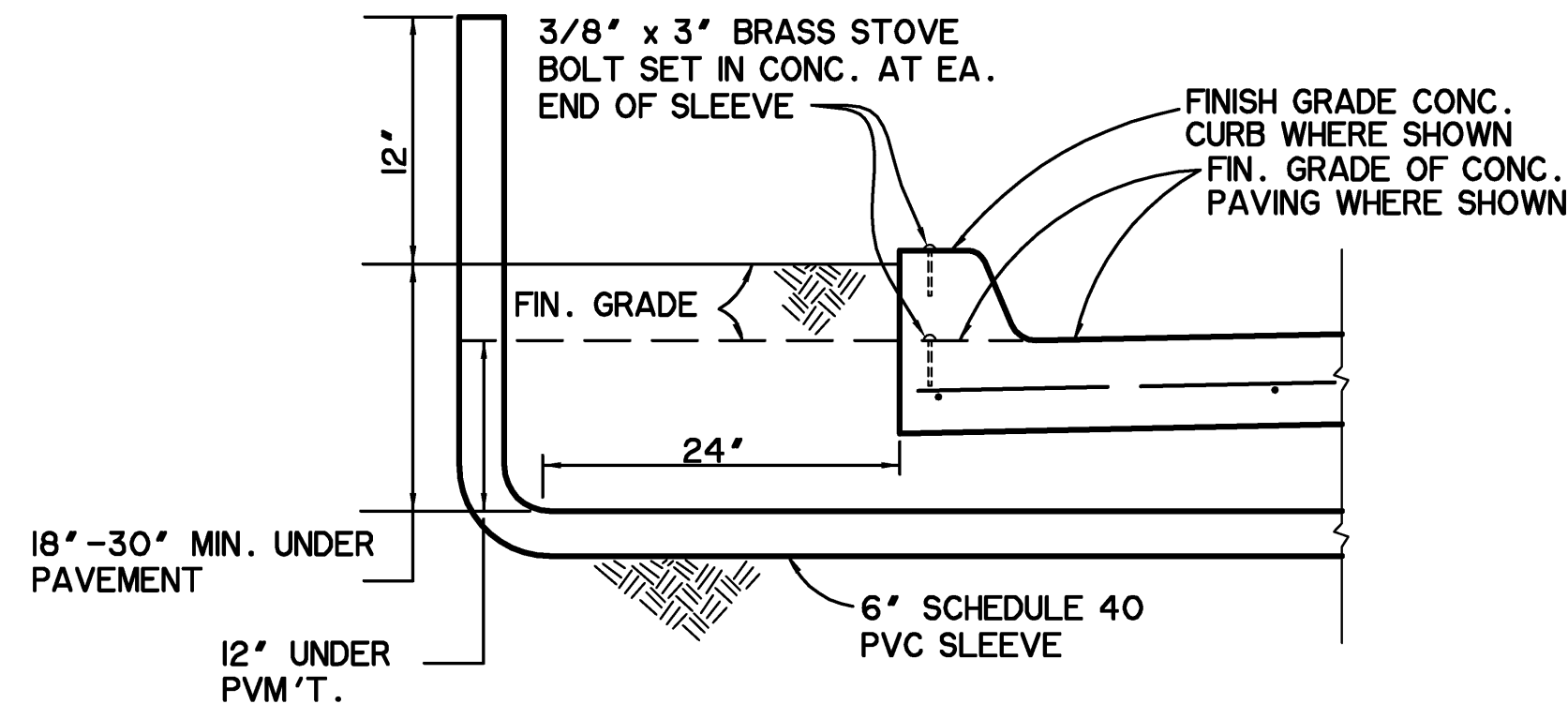
TYPICAL SECTION - STANDARD CONCRETE CURB AND GUTTER

N.T.S.



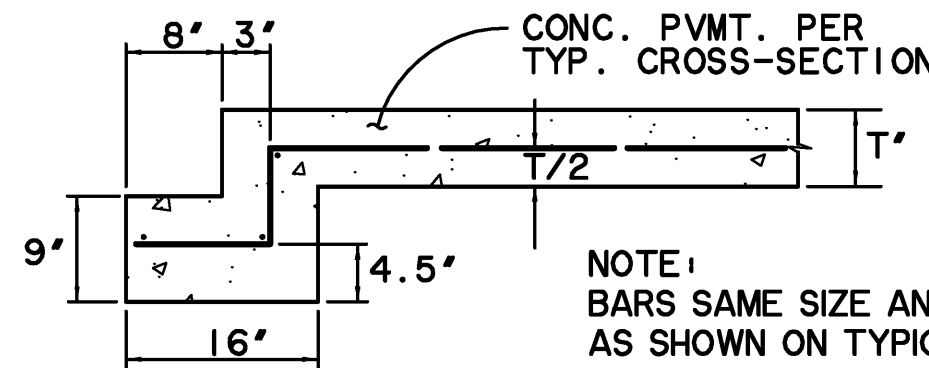
MONOLITHIC CURB

N.T.S.



CONDUIT SLEEVE UNDER PAVEMENT

N.T.S.



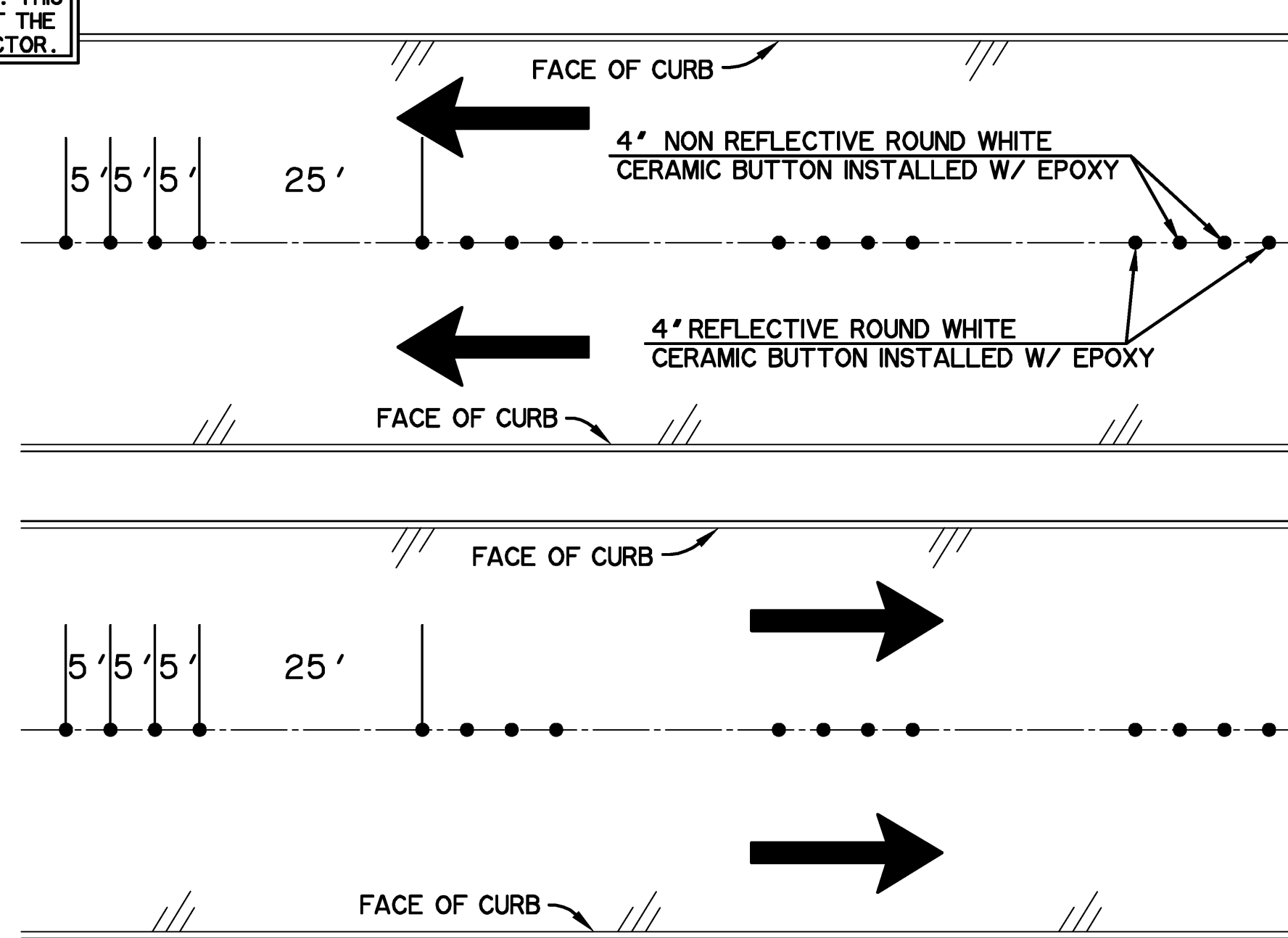
NOTE:
BARS SAME SIZE AND SPACING AS SHOWN ON TYPICAL SECTION. PAVEMENT BARS TO BE BENT DOWN INTO HEADER AND PAVEMENT TO BE MONOLITHIC.

STREET HEADER

N.T.S.

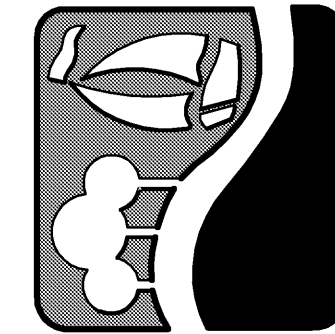
**RECORD
DRAWING
02/02/2015**

TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.



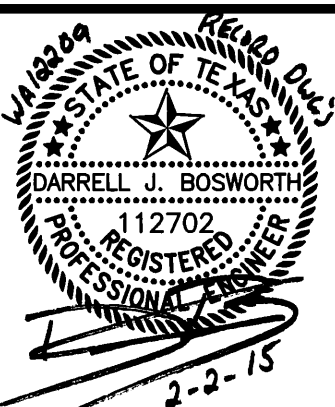
BUTTON DETAIL FOR DISCOVERY BLVD

SCALE: NTS



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**PAVING
DETAILS**



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DATE 10-23-2013
WA# 12209

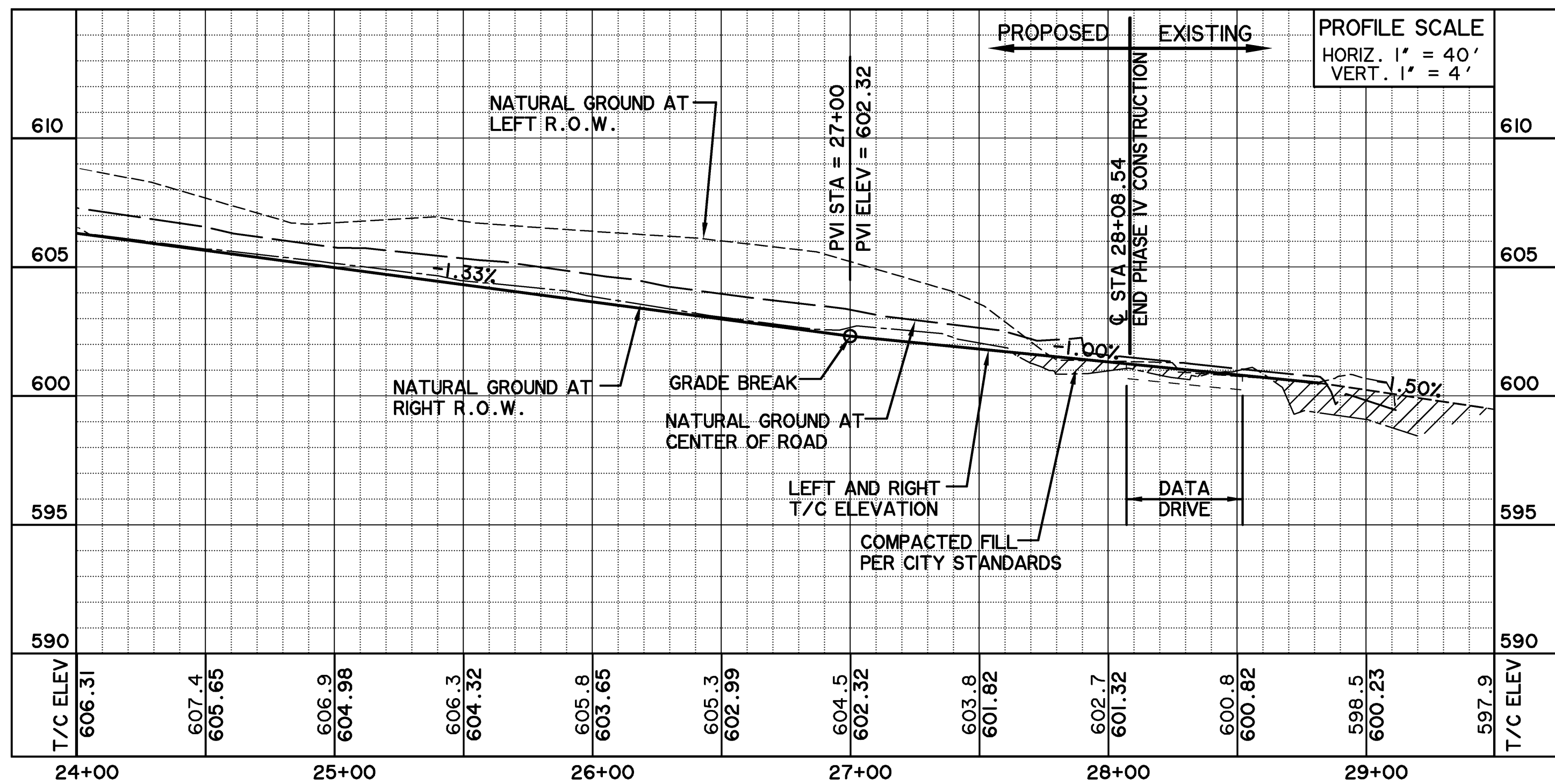
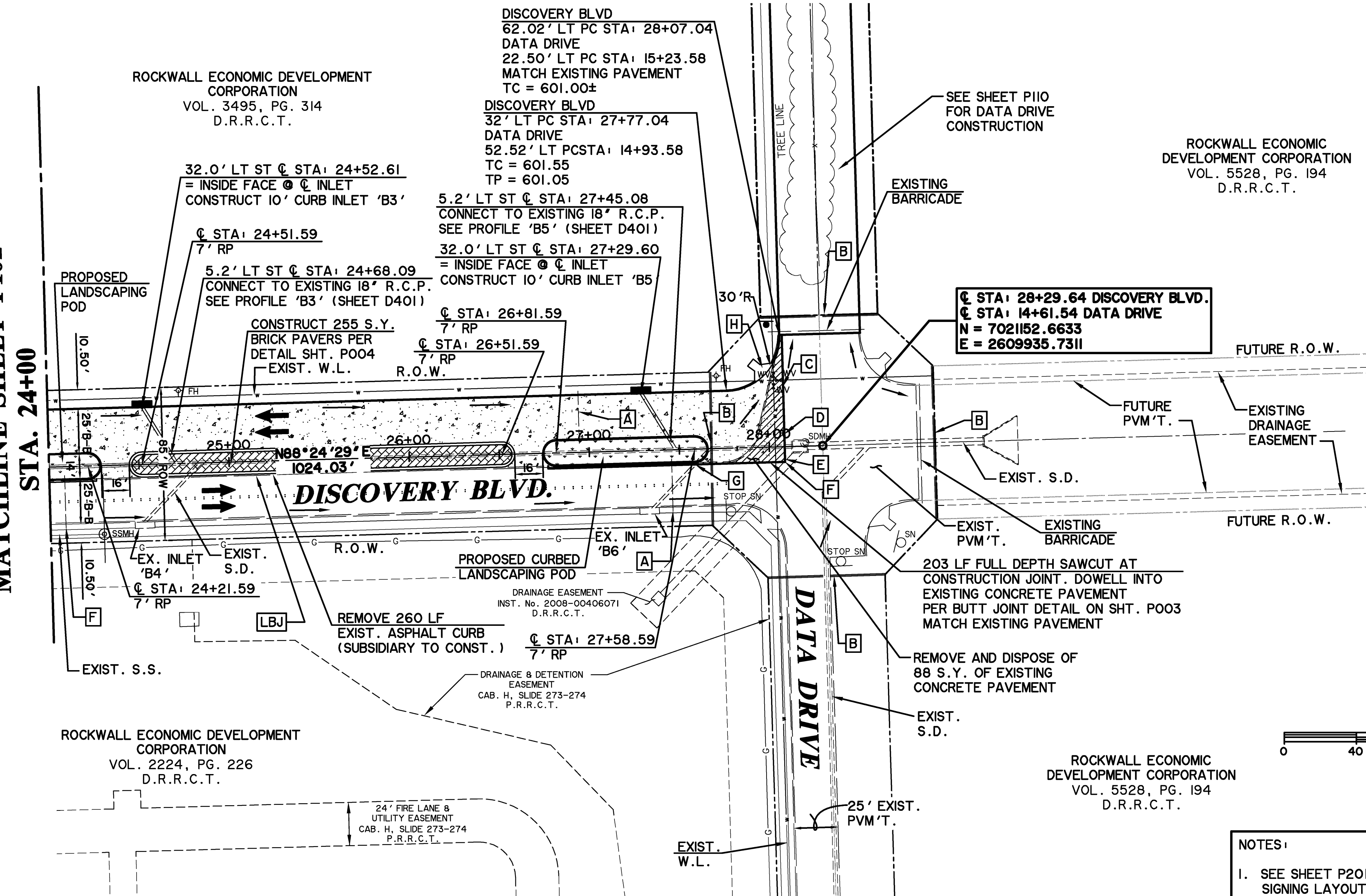
**SHEET NO.
P004**

PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
www.WierAssociates.com
Texas Firm Registration No. F-2776

TIME 15:45 FILE: P101-Pav-Discovery-12209.dwg

MATCHLINE SHEET P102

STA. 24+00



- A EXISTING IRRIGATION CONDUIT (SEE NOTE 2)
- B EXISTING 2-6" GRAY SCHEDULE 40 P.V.C. CONDUITS (FIELD VERIFY)
- C 32.00' LT @ STA 28+08.55 DISCOVERY BOULEVARD PI CURBS EX TP = 601.10
- D 7.00' LT @ STA 28+08.54 DISCOVERY BOULEVARD EX TP = 601.35
- E 7.00' RT @ STA 28+08.53 DISCOVERY BOULEVARD EX TP = 601.40
- F 9.94' RT @ STA 28+08.53 DISCOVERY BOULEVARD EX TP = 601.37
- G 9.61' RT @ STA 27+58.57 DISCOVERY BOULEVARD EX TP = 601.96
- H CONSTRUCT BARRIER FREE RAMP FOR FUTURE 5' WIDE SIDEWALK SEE CITY OF ROCKWALL DETAILS FOR CONSTRUCTION OF RAMPS
- LBJ DOWELL INTO EXIST. CONC. P.V.M.T. PER LONGITUDINAL BUTT JOINT DETAIL ON SHT. P003 MATCH EXIST. PAVEMENT

CAUTION !!
EXISTING UTILITIES ARE INDICATED ON THE PLANS FROM AVAILABLE INFORMATION. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE LOCATION OF ALL UTILITIES, TO NOTIFY ALL UTILITY COMPANIES OF THE CONTRACTORS OPERATIONS, TO PROTECT ALL UTILITIES FROM DAMAGE, TO REPAIR ALL UTILITIES DAMAGED DUE TO THE CONTRACTORS OPERATIONS, AND TO NOTIFY THE ENGINEER PROMPTLY OF ALL CONFLICTS OF THE WORK WITH EXISTING UTILITIES.

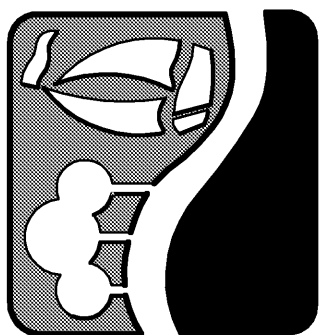
- NOTES:**
- SEE SHEET P201 FOR STRIPING AND SIGNING LAYOUT FOR DISCOVERY BLVD
 - FIELD VERIFY AND FLAG ALL IRRIGATION AND PVC CONDUITS AND CROSSINGS NOTIFY ENGINEER OF ANY DISCREPANCIES.
 - CONTRACTOR TO LOCATE ALL EXISTING FIRE HYDRANTS, VALVES, AND WATER METERS AND ADJUST TO PROPOSED GRADE.
 - DISCOVERY BLVD. DESIGN SPEED = 40 MPH.
 - SEE SHEET T101 FOR TRAFFIC CONTROL PLAN.

* ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OF DESIGN. *

- BENCH MARKS**
- BM A AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.
- BM B AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.
- BM C - AN '□' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

**RECORD
DRAWING
02/02/2015**

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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

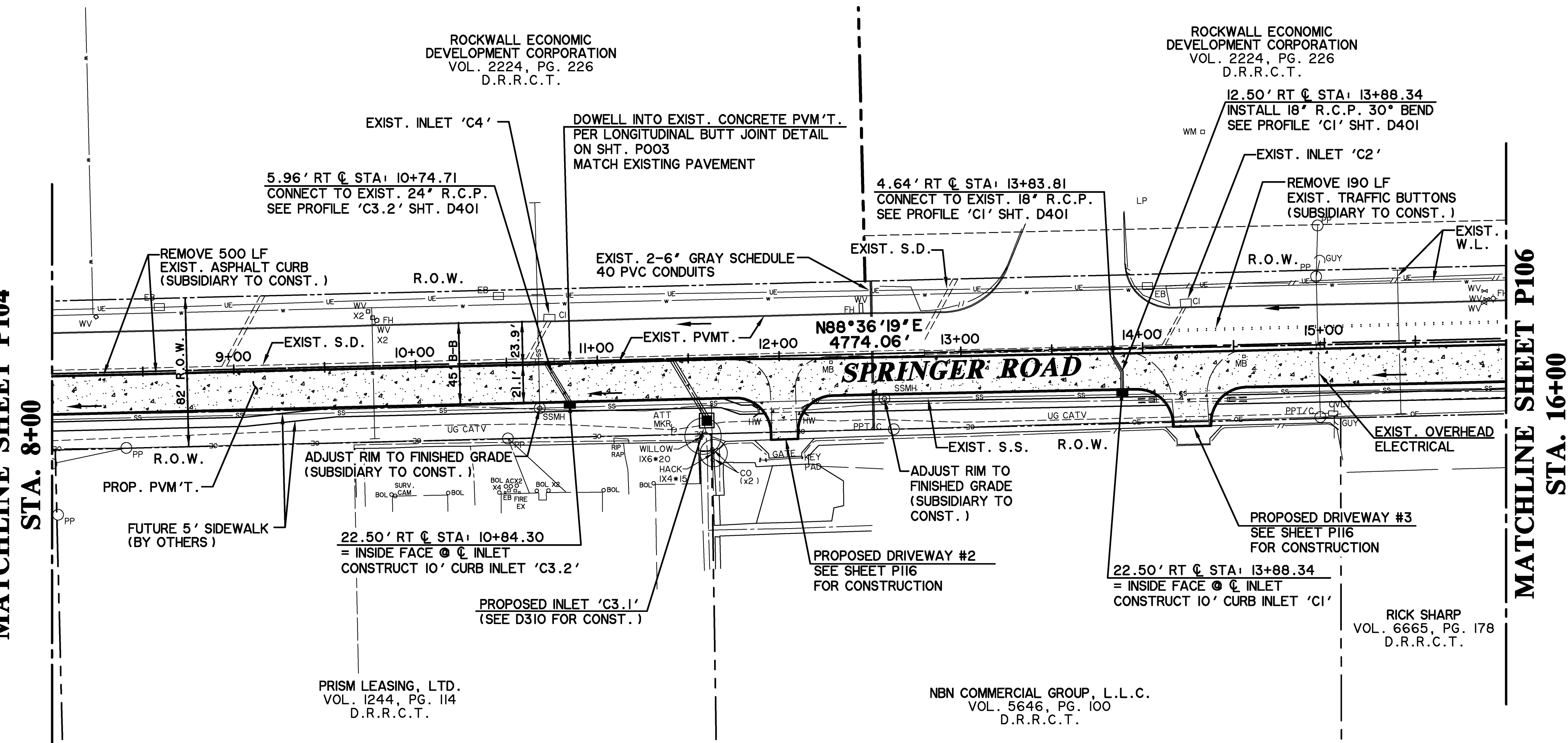
**DISCOVERY BLVD.
PAVING PLAN AND PROFILE
STA 24+00 TO STA 28+29.59**



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LAST SHEET EDIT
DATE 12-26-2013
WA# 12209
**SHEET NO.
P103**

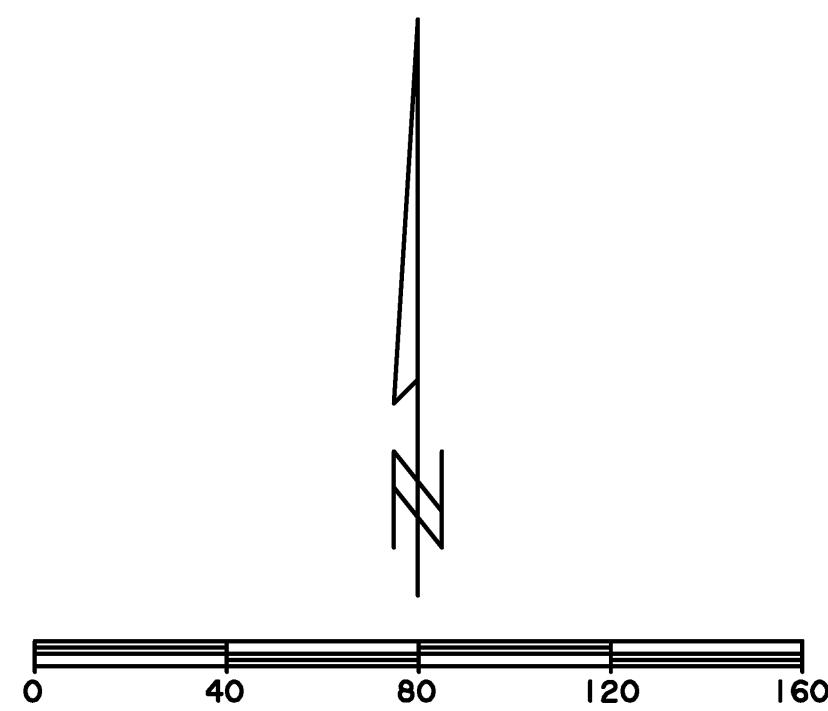
PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

MATCHLINE SHEET P104
STA. 8+00



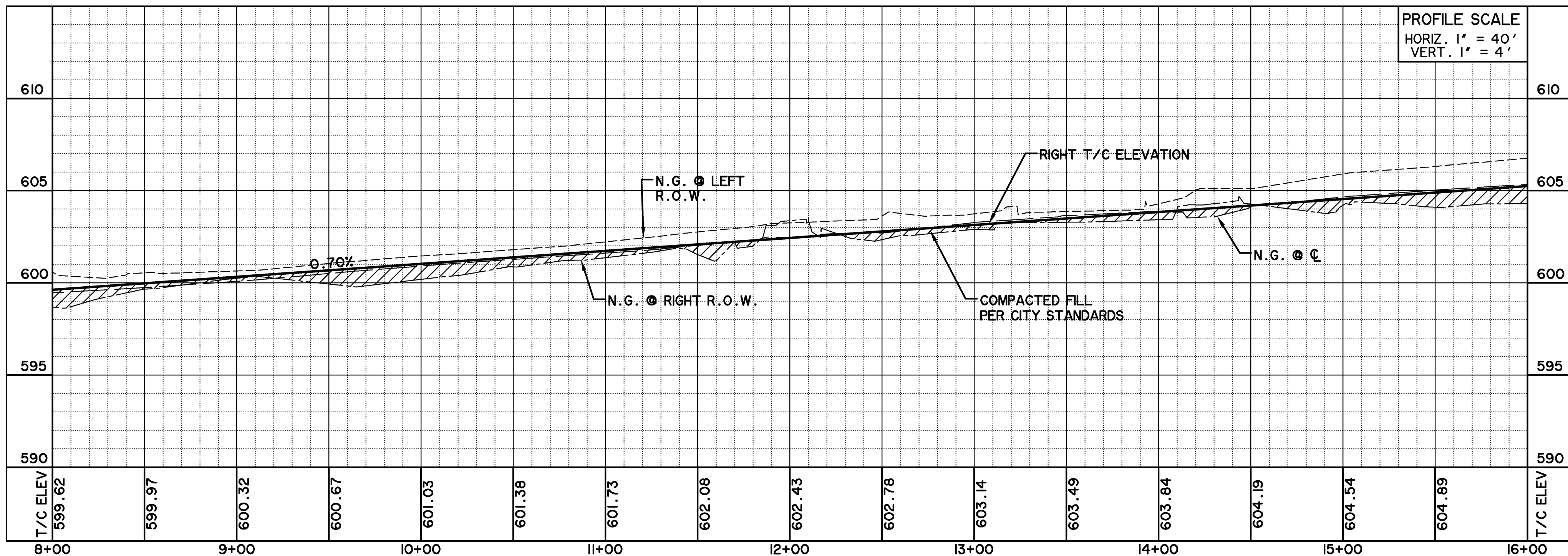
MATCHLINE SHEET P106
STA. 16+00

- NOTE:
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 - 2) STREET DIMENSIONS ARE FROM BACK OF CURB TO EDGE OF EXIST. CONCRETE (BC-EC) UNLESS OTHERWISE NOTED.
 - 3) SEE TYPICAL SECTIONS ON SHEET P002 FOR GRADING ON SOUTH SIDE OF SPRINGER ROAD.
 - 4) FIELD VERIFY AND FLAG ALL IRRIGATION AND PVC CONDUITS AND CROSSINGS. NOTIFY ENGINEER OF ANY DISCREPANCIES.
 - 5) DESIGN SPEED = 35 MPH.
 - 6) SEE SHEET T101 FOR TRAFFIC CONTROL PLAN.



CAUTION !!
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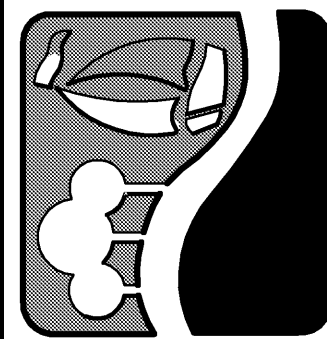


* BENCH MARKS *

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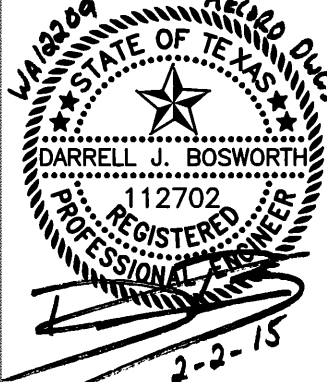
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02/02/2015**

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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**SPRINGER RD
PAVING PLAN AND PROFILE
STA 8+00 TO STA 16+00**

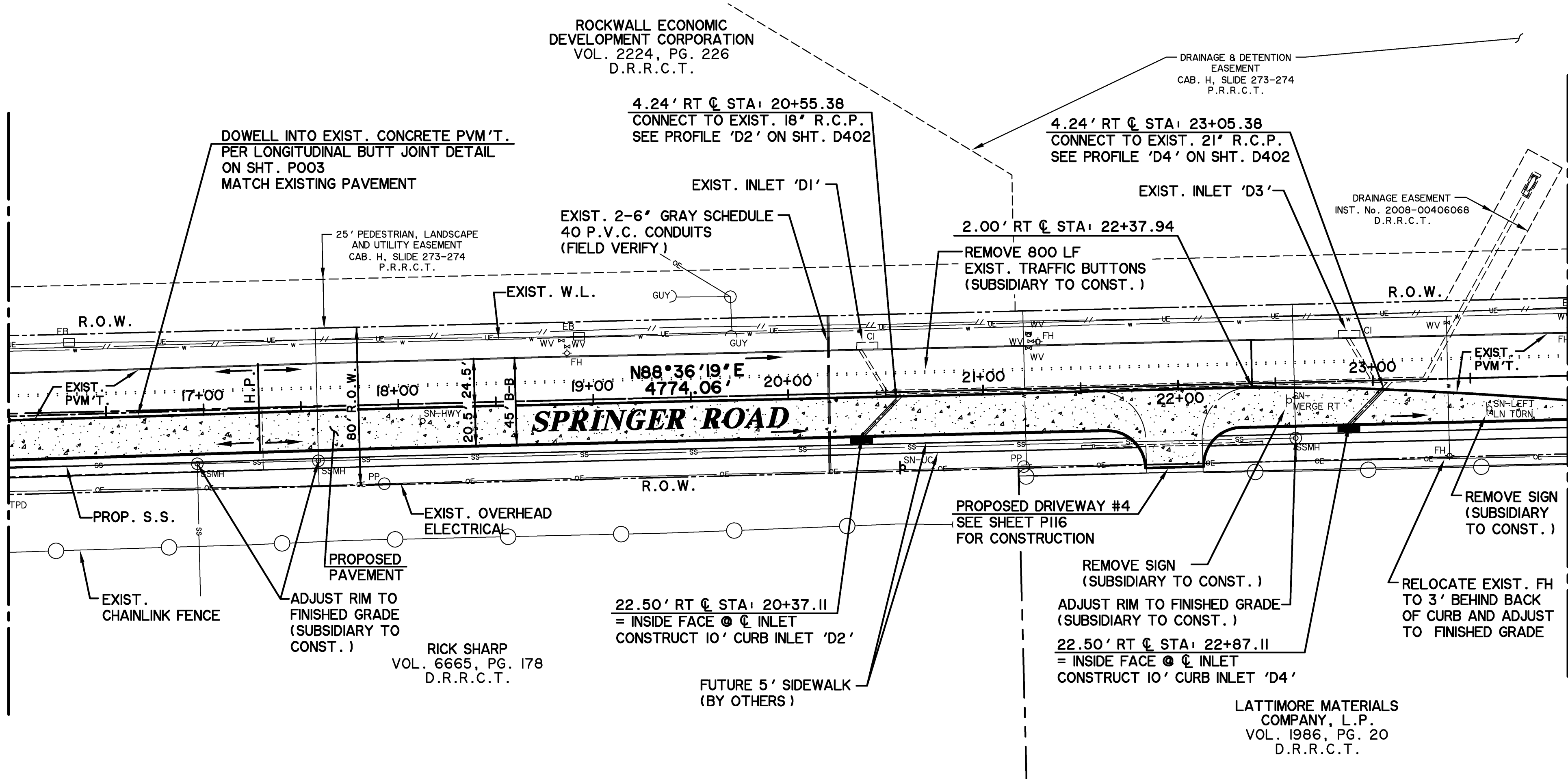


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LAST SHEET EDIT
DATE: 06-2013
WA# 12209
**SHEET NO.
P105**

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701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
www.WierAssociates.com
Texas Firm Registration No. F-2776

MATCHLINE SHEET P105

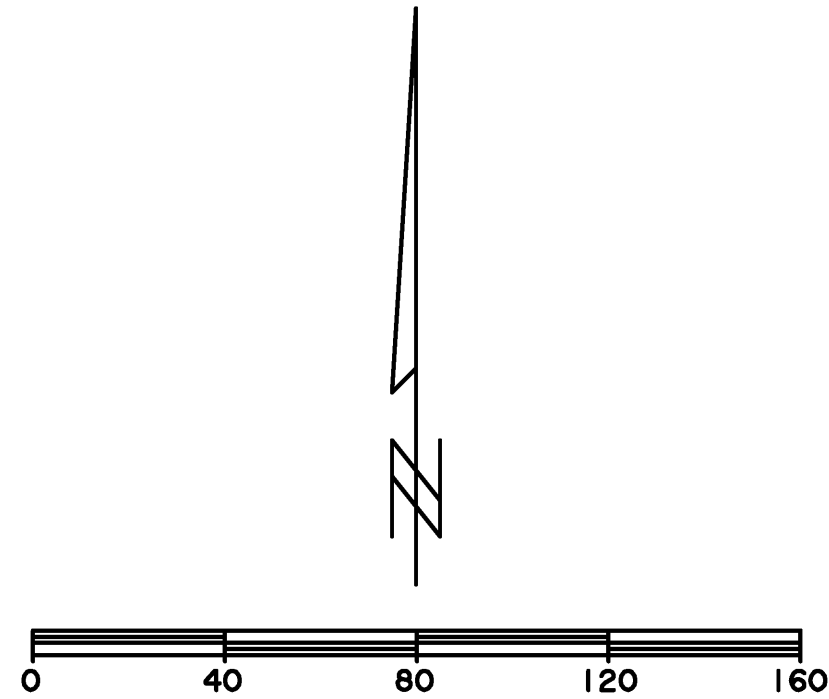
STA. 16+00



MATCHLINE SHEET P107

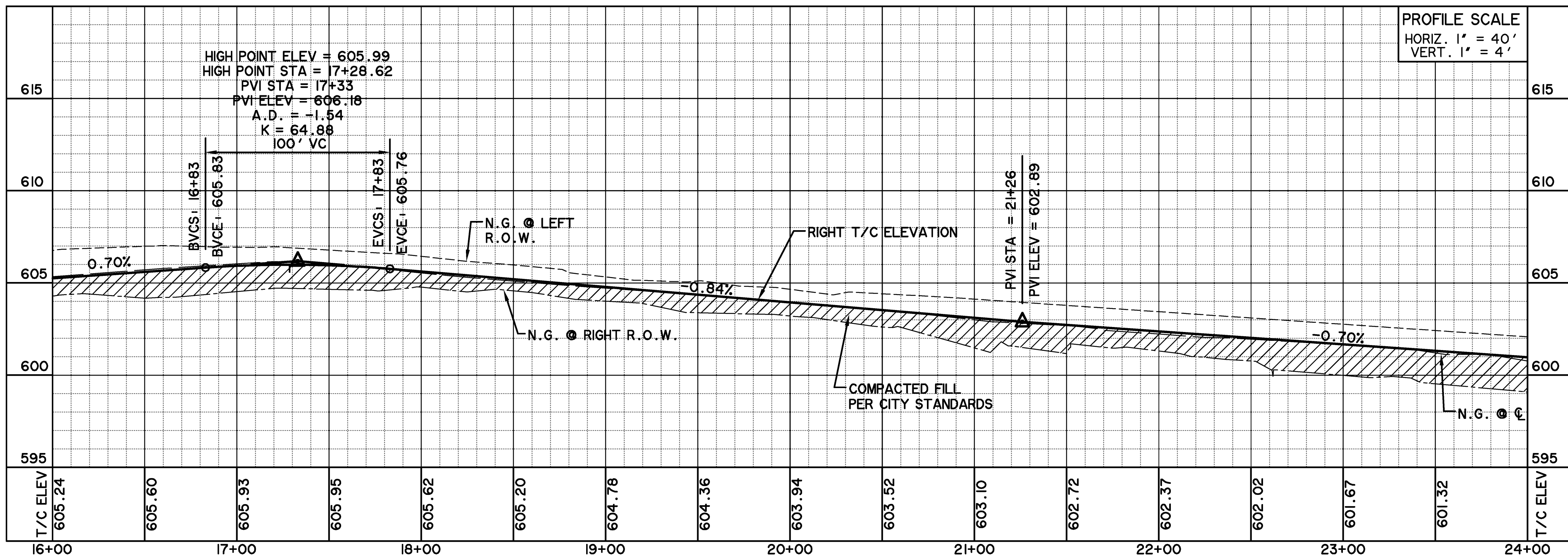
STA. 24+00

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 - 4) FIELD VERIFY AND FLAG ALL IRRIGATION AND PVC CONDUITS AND CROSSINGS. NOTIFY ENGINEER OF ANY DISCREPANCIES.
 - 5) DESIGN SPEED = 35 MPH.
 - 6) SEE SHEET T101 FOR TRAFFIC CONTROL PLAN.



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• BENCH MARKS •

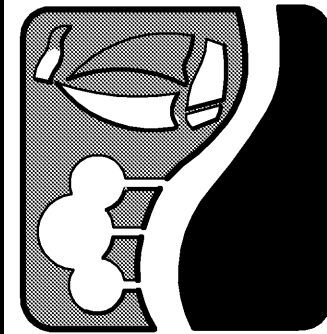
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BM C - AN "X" CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

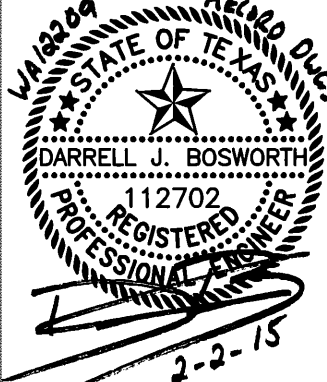
**RECORD
DRAWING
02/02/2015**

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**ROCKWALL
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PARK
PHASE IV**

**SPRINGER RD
PAVING PLAN AND PROFILE
STA 16+00 TO STA 24+00**



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DATE: 10-7-2013
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**SHEET NO.
P106**

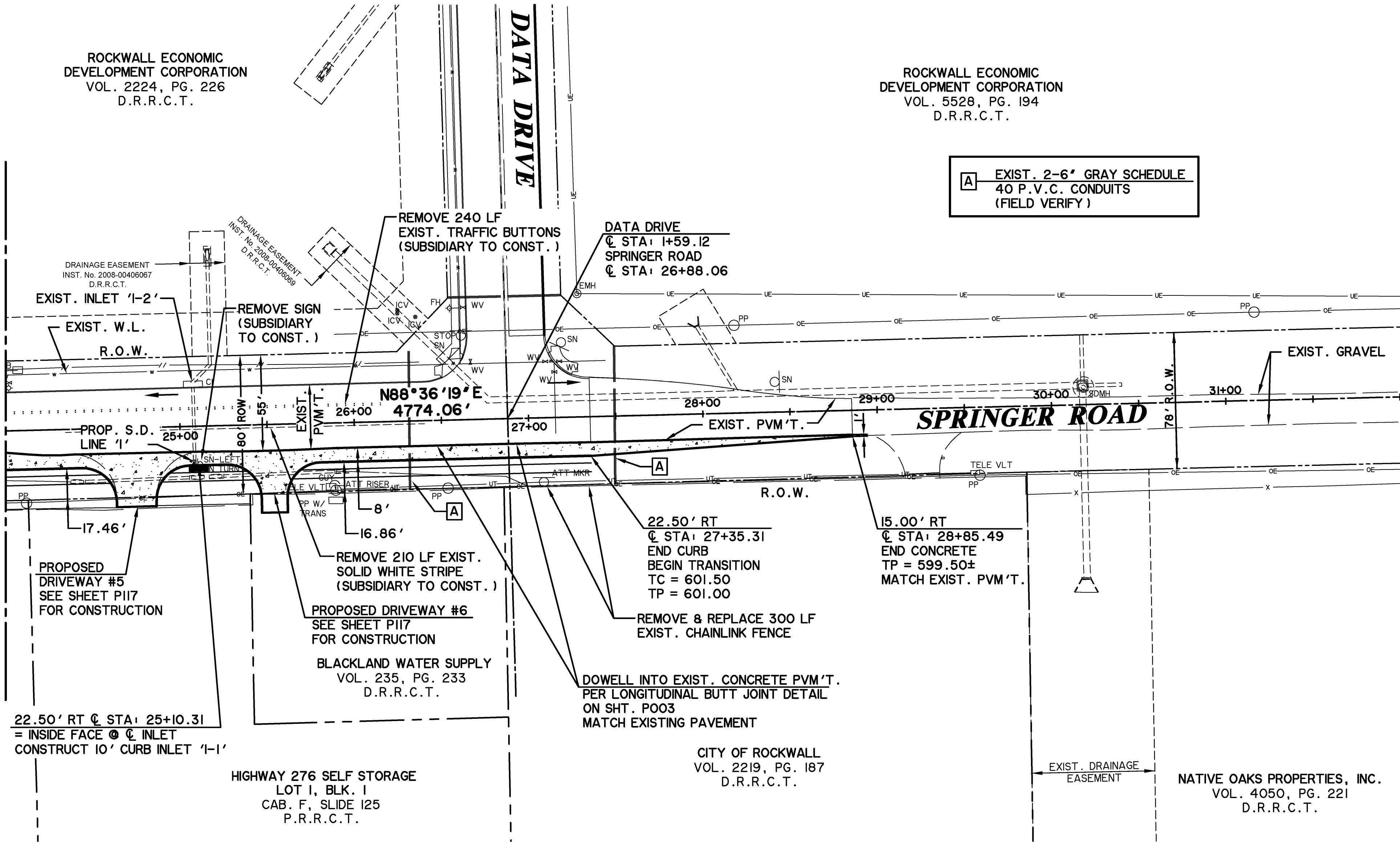
PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
www.WierAssociates.com
Texas Firm Registration No. F-2776

MATCHLINE SHEET P106

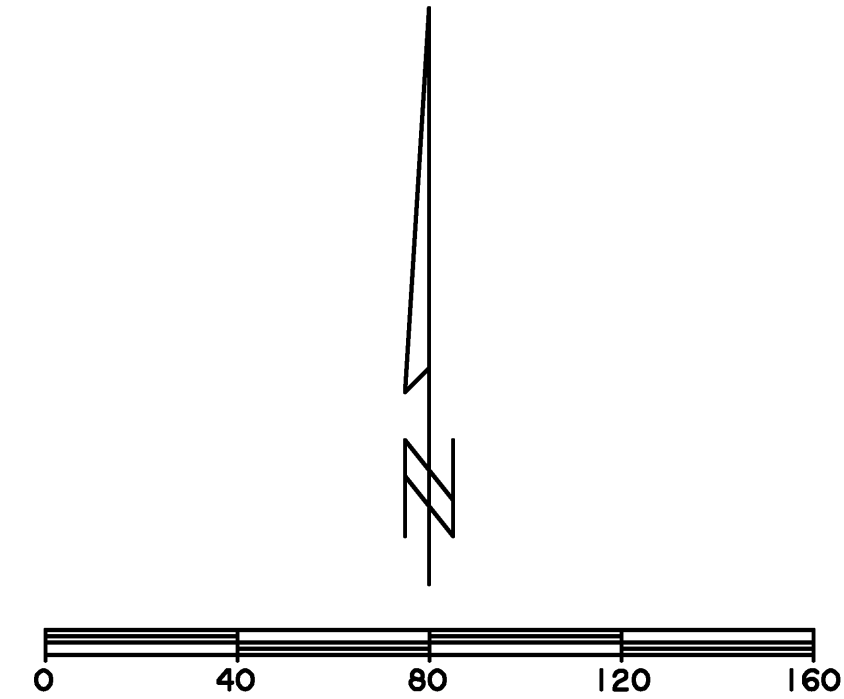
STA. 24+00

ROCKWALL ECONOMIC
DEVELOPMENT CORPORATION
VOL. 2224, PG. 226
D.R.R.C.T.

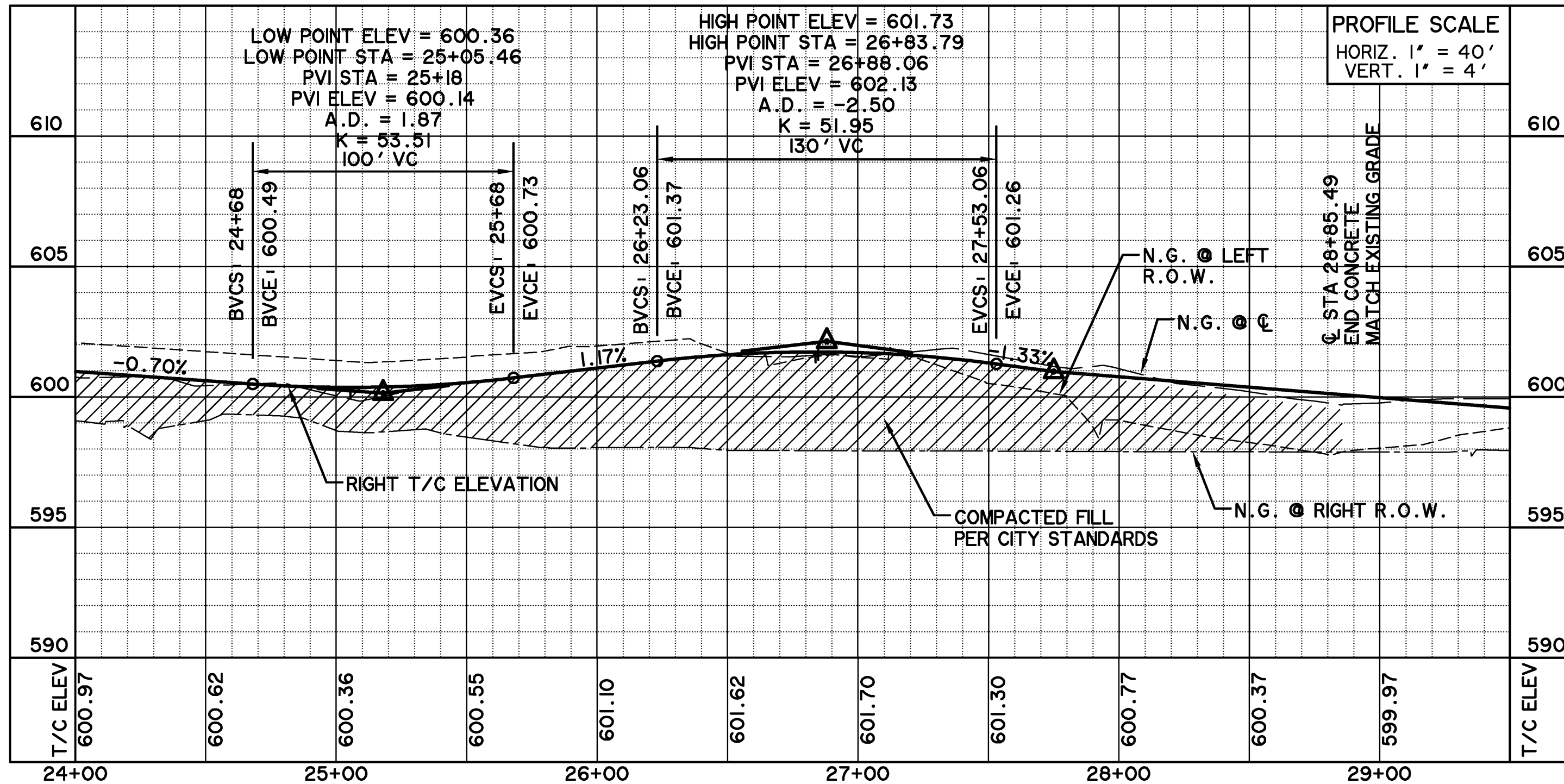
ROCKWALL ECONOMIC
DEVELOPMENT CORPORATION
VOL. 5528, PG. 194
D.R.R.C.T.



- NOTE:
- 1) CONTRACTOR TO LOCATE ALL EXISTING FIRE HYDRANTS, VALVES, AND WATER METERS AND ADJUST TO PROPOSED GRADE.
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 - 4) FIELD VERIFY AND FLAG ALL IRRIGATION AND PVC CONDUITS AND CROSSINGS. NOTIFY ENGINEER OF ANY DISCREPANCIES.
 - 5) DESIGN SPEED = 35 MPH.
 - 6) SEE SHEET T101 FOR TRAFFIC CONTROL PLAN.



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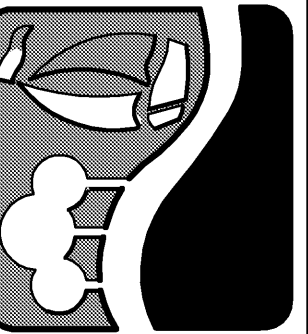
* ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OF DESIGN. *

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- BM C - AN "X" CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

**RECORD
DRAWING
02/02/2015**

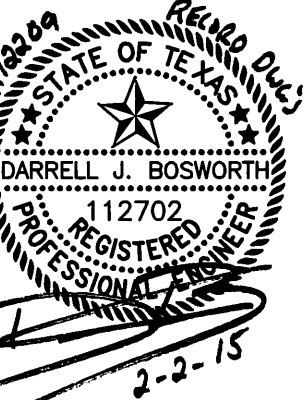
TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

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701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
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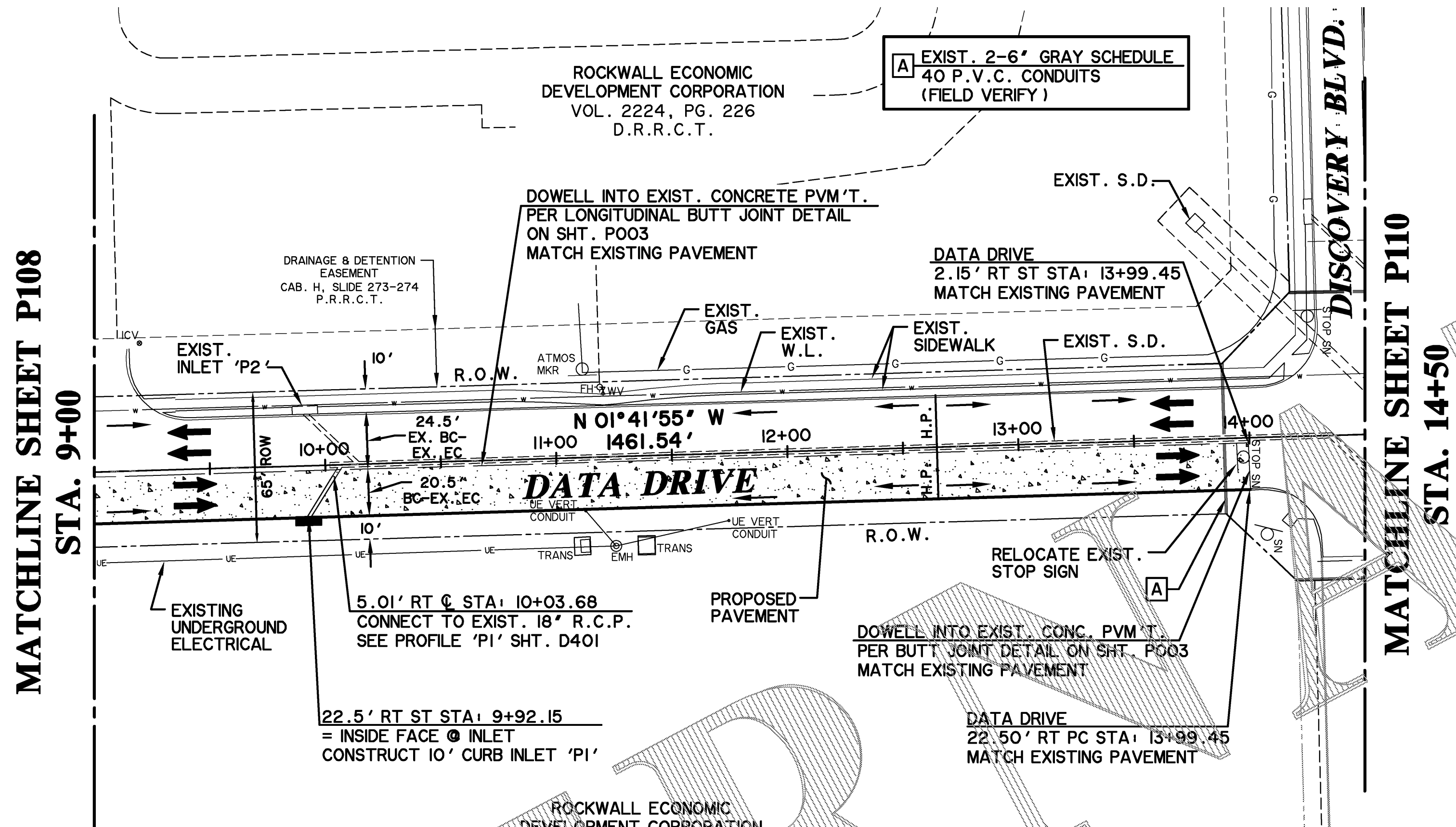


**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**SPRINGER RD
PAVING PLAN AND PROFILE
STA 24+00 TO STA 32+00**

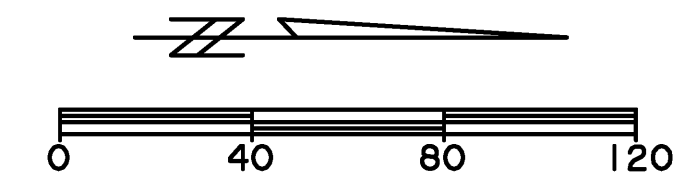


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LAST SHEET EDIT
DATE: 02-07-2015
WA# 12209
**SHEET NO.
P107**



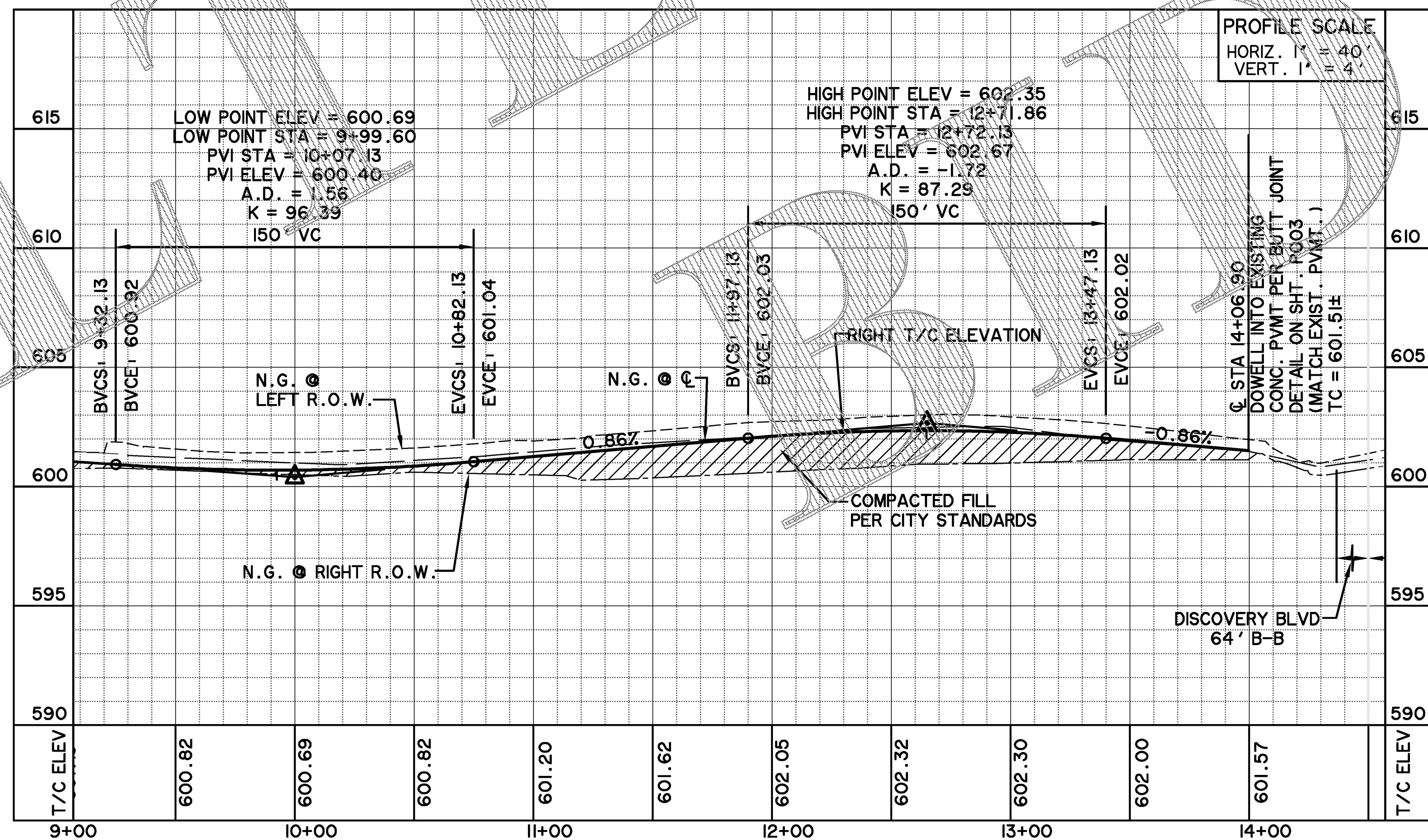
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- 5) DESIGN SPEED = 35 MPH.
- 6) SEE SHEET T101 FOR TRAFFIC CONTROL PLAN.



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* BENCH MARKS *

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LOCATED AT THE SOUTH RIGHT-OF-WAY LINE
OF SPRINGER ROAD $\pm 2470'$ EAST OF THE
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549. 598.80 FT.

BM B AN "X" CUT IN THE BACK OF CURB
LOCATED AT THE NORTH RIGHT-OF-WAY
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BOULEVARD AND F.M. 549. 599.82 FT.

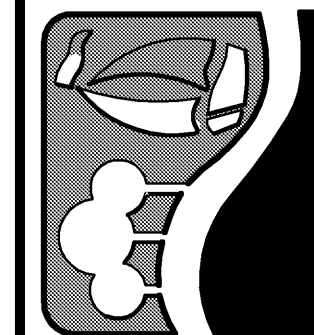
BM C - AN "□" CUT IN DISCOVERY
BOULEVARD IN A MEDIAN NOSE ±60' WEST OF
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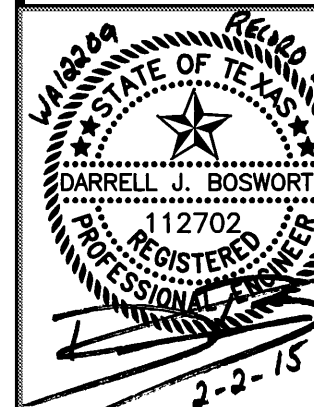
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WIA WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817) 467-7700
Texas Firm Registration No. F-2776 www.wierassociates.com



ROCKWALL TECHNOLOGY PARK PHASE IV

**DATA DRIVE
PAVING PLAN AND PROFILE
STA 9+00 TO STA 14+50**



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LAST SHEET EDIT
DATE 10-17-2013
WA# 12209

SHEET NO.
P109

STA. 14+50

DISCOVERY BLVD.

DATA DRIVE
22.50' LT PC STA: 15+23.58
MATCH EXISTING PAVEMENT

—DOWELL INTO EXIST. CONC. PVM'T.
PER BUTT JOINT DETAIL ON SHT. PO03
MATCH EXISTING PAVEMENT

22.5' LT ST STA: 17+30.00
= INSIDE FACE @ INLET

CONSTRUCT DIVERSION SWALE
BETWEEN STREET STATIONS
15+20 AND 20+50
(SEE DETAIL THIS SHEET.)
DIVERSION SWALE TO
BE REMOVED UPON
DEVELOPMENT OF ADJACENT
PROPERTY

ROCKWALL ECONOMIC
DEVELOPMENT CORPORATION
VOL. 3495, PG. 314
D.R.R.C.T.

EXISTING UTILITIES ARE INDICATED ON THE PLANS FROM AVAILABLE INFORMATION. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE LOCATION OF ALL UTILITIES, TO NOTIFY ALL UTILITY COMPANIES OF THE CONTRACTORS OPERATIONS, TO PROTECT ALL UTILITIES FROM DAMAGE, TO REPAIR ALL UTILITIES DAMAGED DUE TO THE CONTRACTORS OPERATIONS, AND TO NOTIFY THE ENGINEER PROMPTLY OF ALL CONFLICTS OF THE WORK WITH EXISTING UTILITIES.

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- 2) STREET DIMENSIONS ARE FROM BACK OF CURB TO EDGE OF EXIST. CONCRETE (BC-EC) UNLESS OTHERWISE NOTED.
- 3) SEE TYPICAL SECTIONS ON SHEET PO02 FOR GRADING ON SOUTH SIDE OF SPRINGER ROAD.
- 4) FIELD VERIFY AND FLAG ALL IRRIGATION AND PVC CONDUITS AND CROSSINGS. NOTIFY ENGINEER OF ANY DISCREPANCIES.
- 5) DATA DRIVE DESIGN SPEED = 35 MPH.

DATA DRIVE
22.50' RT PC STA: 15+23.50
MATCH EXISTING PAVEMENT

DATA DRIVE
 STA: 14+61.54
 DISCOVER BLVD
 STA: 28+29.64

22.5' RT ST STA: 17+30.00
= INSIDE FACE @ INLET
CONSTRUCT 10' CURB INLET 'M2'

**✓ EXIST. TREES WITHIN R.O.W.
TO BE REMOVED & DISPOSED
(SUBSIDIARY TO CONSTRUCTION)**

ROCKWALL ECONOMIC
DEVELOPMENT CORPORATION
VOL. 5528, PG. 194
D.R.R.C.T.

PROP. S.D. LINE

└ PROP. S.D. LINE 'M'

S.D. LINE 'M'



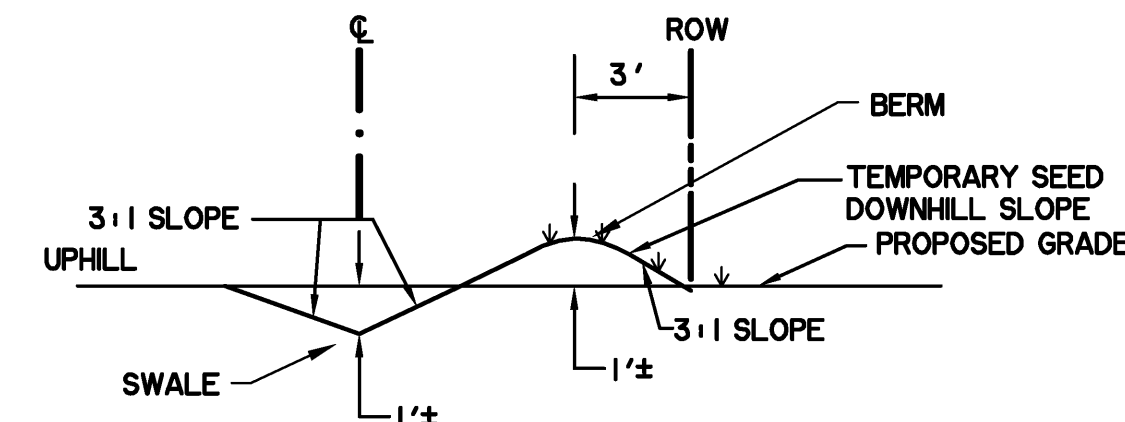
— PROPOSED
PAVEMENT

PROP. CULVERT L

— EXIST FENCE

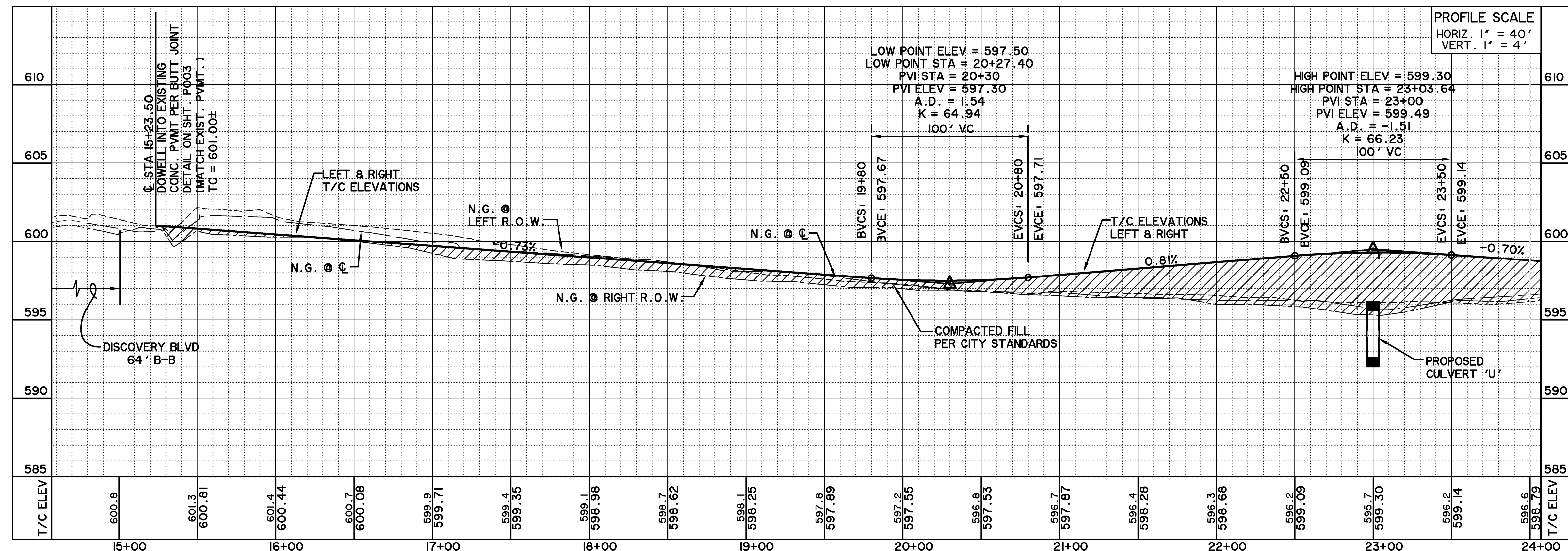
MATCHLINE SHEET P111

STA: 24+00



TEMPORARY SWALE / BERM DETAIL

N.T.S.



PROFILE SCALE
HORIZ. 1" = 40'
VERT. 1" = 4'

* BENCH MARKS *

BM A AN 'X' CUT IN THE BACK OF CURB
LOCATED AT THE SOUTH RIGHT-OF-WAY LINE
OF SPRINGER ROAD $\pm 2470'$ EAST OF THE
INTERSECTION OF SPRINGER ROAD AND F.M.
549. 598.80 FT.

BM B AN ☒ CUT IN THE BACK OF CURB
LOCATED AT THE NORTH RIGHT-OF-WAY
LINE OF DISCOVERY BOULEVARD ±80' EAST
OF THE INTERSECTION OF DISCOVERY
BOULEVARD AND F.M. 549. 599.82 FT.

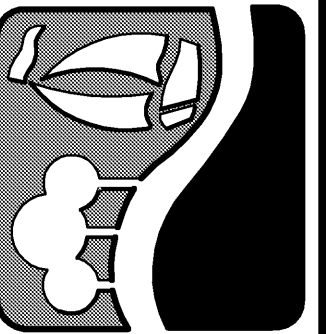
BM C - AN '□' CUT IN DISCOVERY
BOULEVARD IN A MEDIAN NOSE ±60' WEST OF
THE INTERSECTION OF DISCOVERY BOULEVARD
AND F.M. 549. 598.20 FT.

' ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN
REMAINS WITH THE DESIGN ENGINEER. THE CITY
OF ROCKWALL, IN REVIEWING AND RELEASING
PLANS FOR CONSTRUCTION, ASSUMES NO
RESPONSIBILITY FOR ADEQUACY OF DESIGN.'

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DRAWING
02/02/2015**

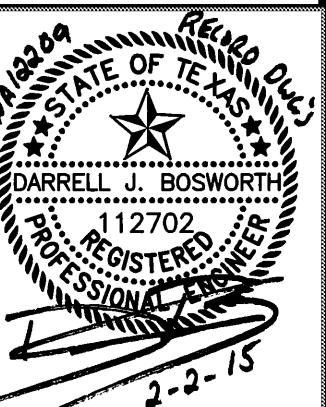
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ENGINEERS SURVEYORS LAND PLANNERS
 701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817) 467-7700
 Texas Firm Registration No. F-2776 www.WierAssociates.com



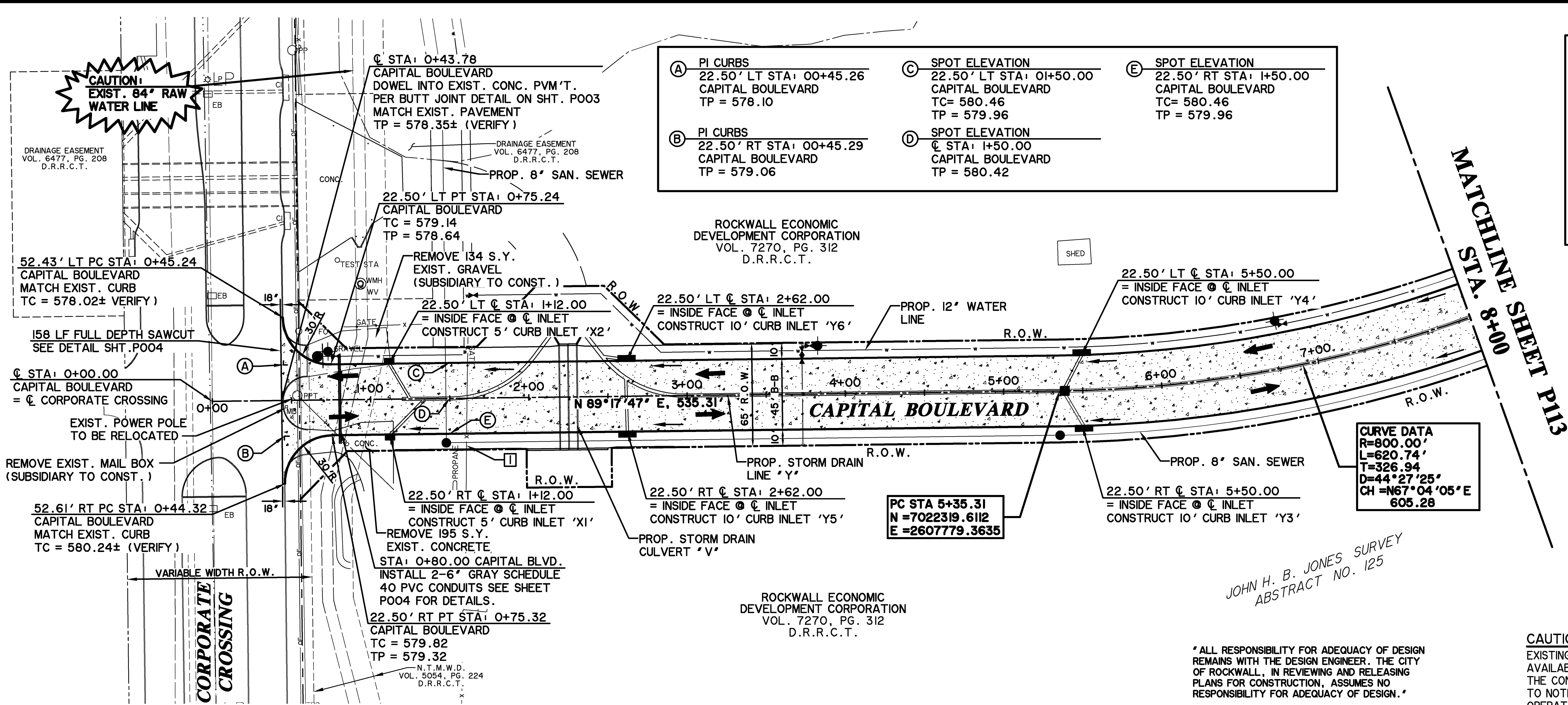
ROCKWALL TECHNOLOGY PARK PHASE IV

**DATA DRIVE
PAVING PLAN AND PROFILE
STA 14+50 TO STA 25+00**



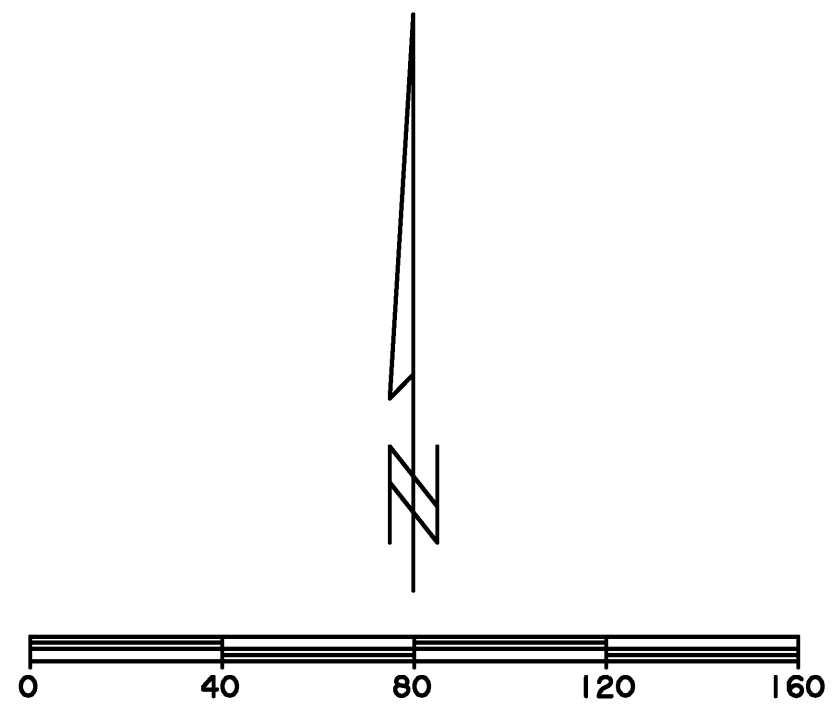
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LAST SHEET EDIT
DATE 10-17-2013
WA# 12209

SHEET NO.
P110



- NOTE:
- CAPITAL BOULEVARD DIMENSIONS ARE FROM BACK OF CURB TO BACK OF CURB (BC-BC) FROM STA 0+00 TO STA 12+41.74 AND BACK OF CURB TO EDGE OF CONCRETE (BC-EC) FROM 12+41.74 TO END.
 - DESIGN SPEED = 35 MPH
 - SEE TXDOT DETAIL TCP (1-4)-12 ON SHEET T201 FOR TRAFFIC CONTROL ON CORPORATE CROSSING DURING CONSTRUCTION. (USE TCP (1-4a))

1 REMOVE 100 L.F. OF EXIST. BARBED WIRE FENCE SUBSIDIARY TO CONSTRUCTION



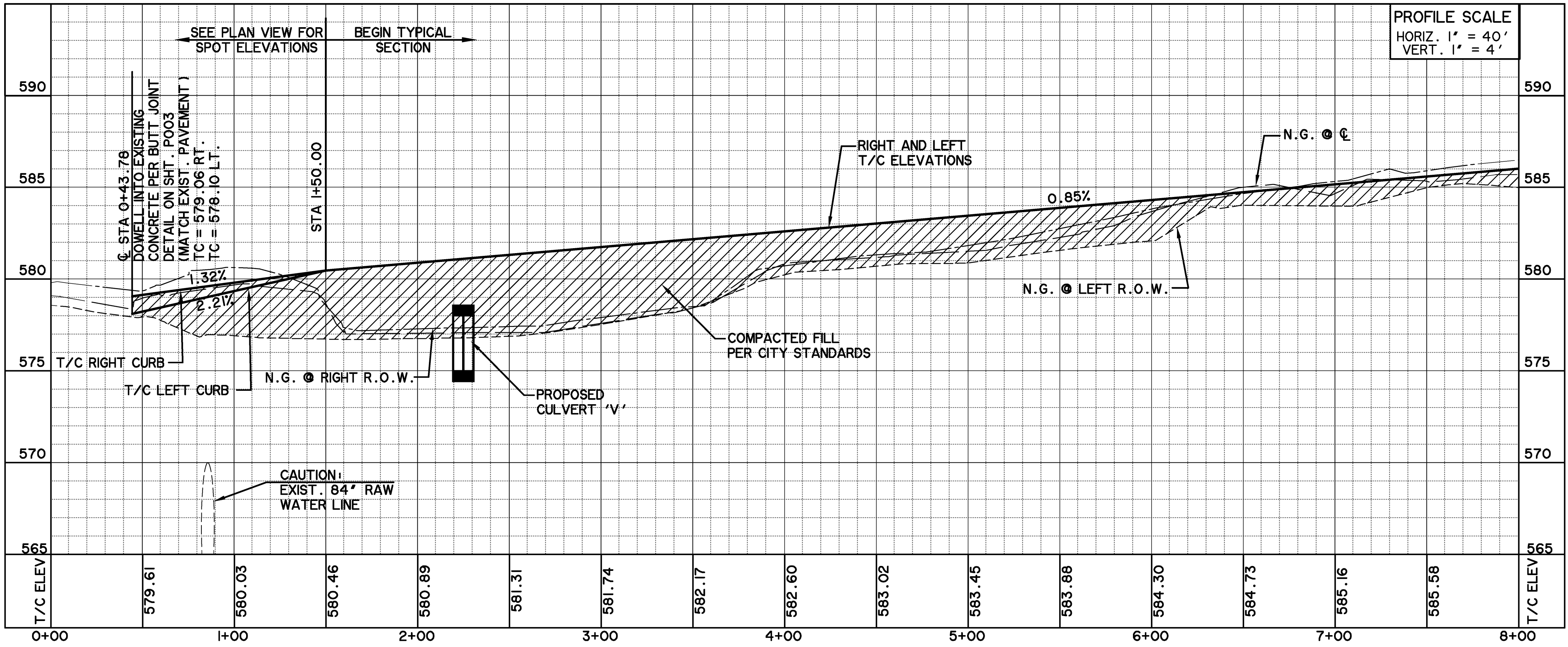
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L=620.74'
T=326.94'
D=44°27'25"
CH=N67°04'05"E
605.28

PC STA 5+35.31
N=7022319.6112
E=2607779.3635

JOHN H. B. JONES SURVEY
ABSTRACT NO. 125

* ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OF DESIGN. *

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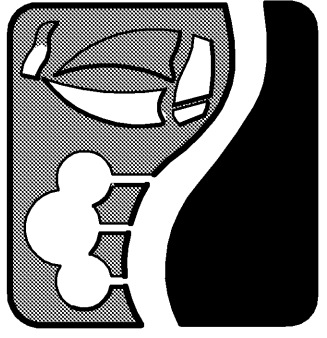


- * BENCH MARKS *
- BM A - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.
- BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.
- BM C - AN 'X' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

**RECORD
DRAWING
02/02/2015**

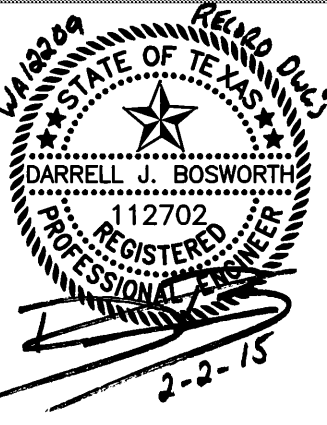
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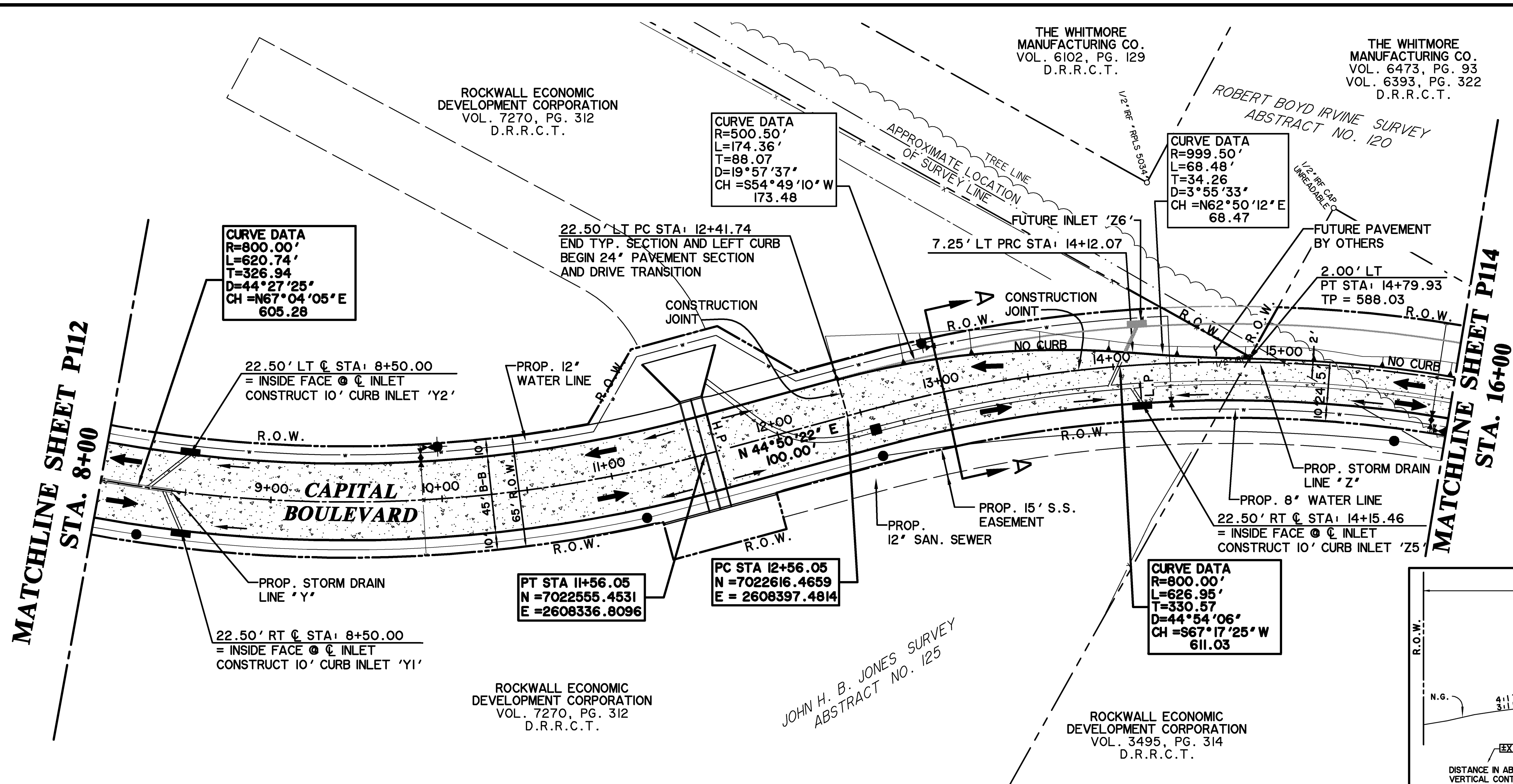


**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**CAPITAL BOULEVARD
PAVING PLAN AND PROFILE
STA 0+00 TO STA 8+00**



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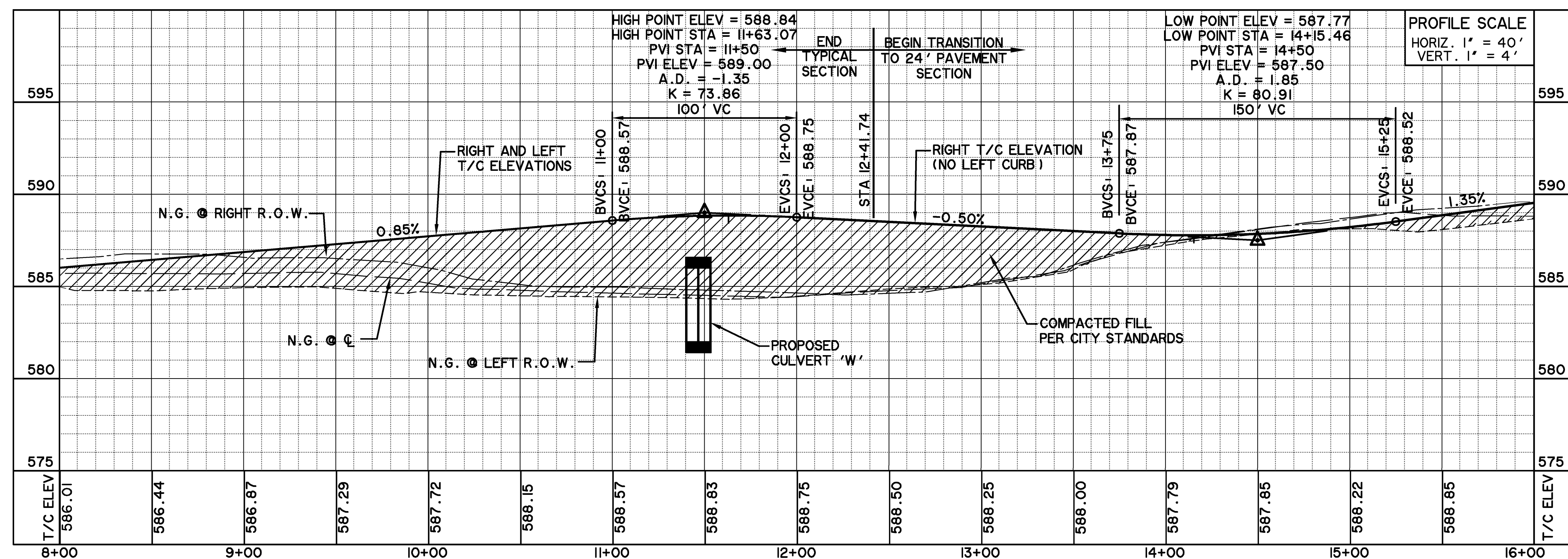
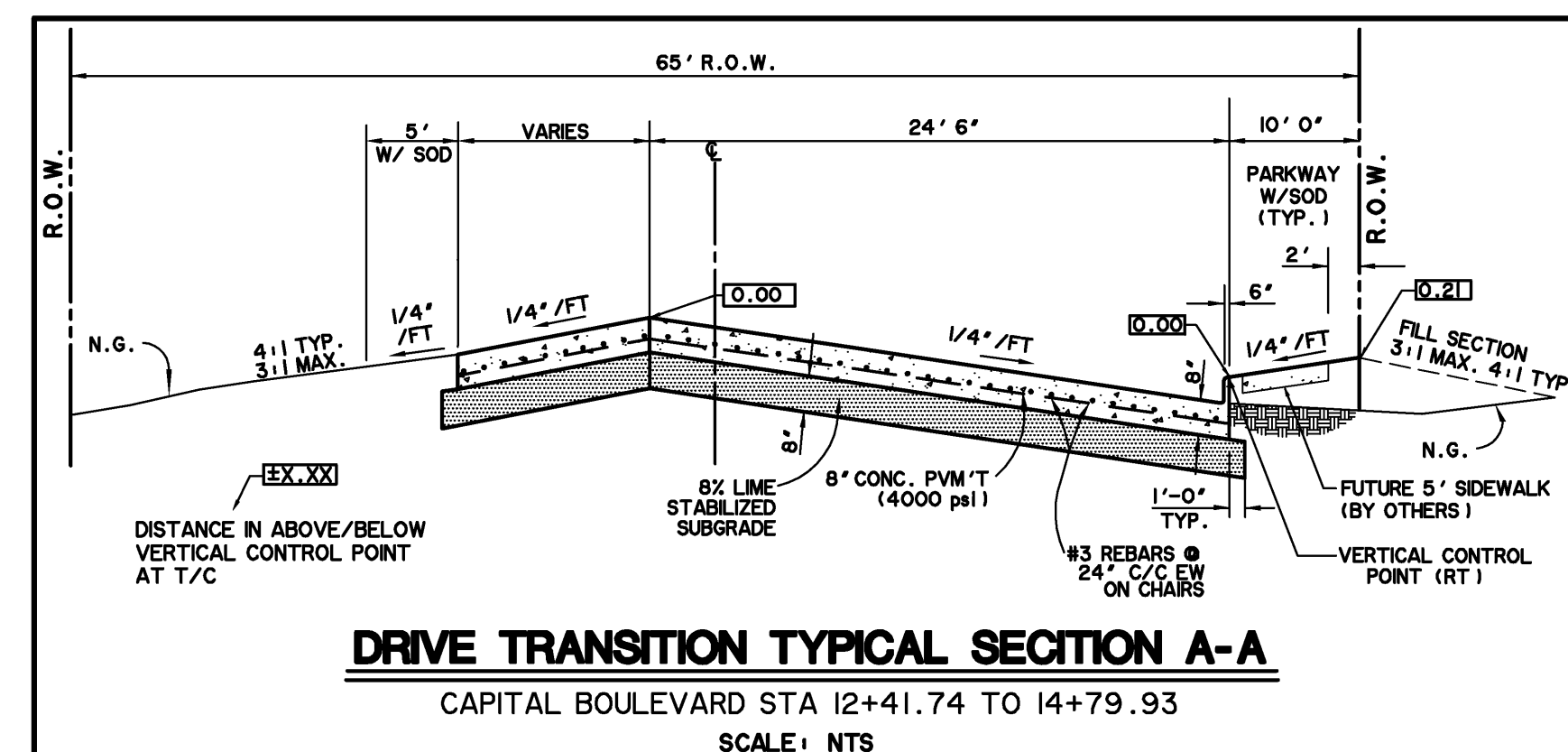


NOTE:

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- DESIGN SPEED = 35 MPH

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* BENCH MARKS *

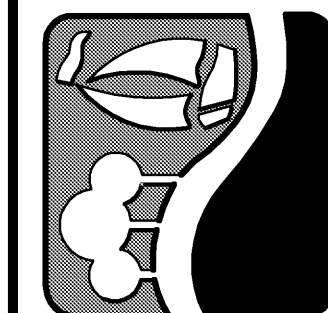
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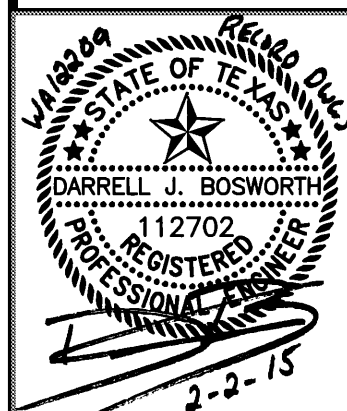
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DRAWING
02/02/2015**

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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

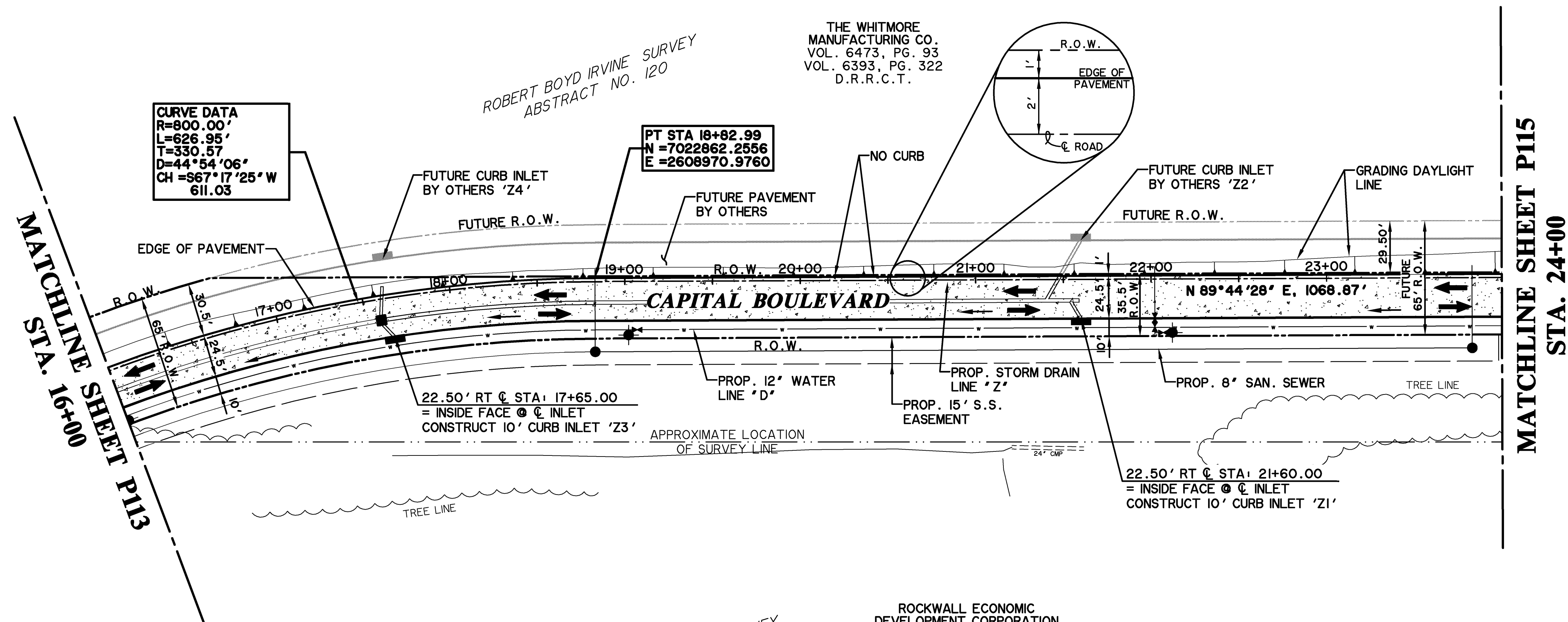
**CAPITAL BOULEVARD
PAVING PLAN AND PROFILE
STA 8+00 TO STA 16+00**



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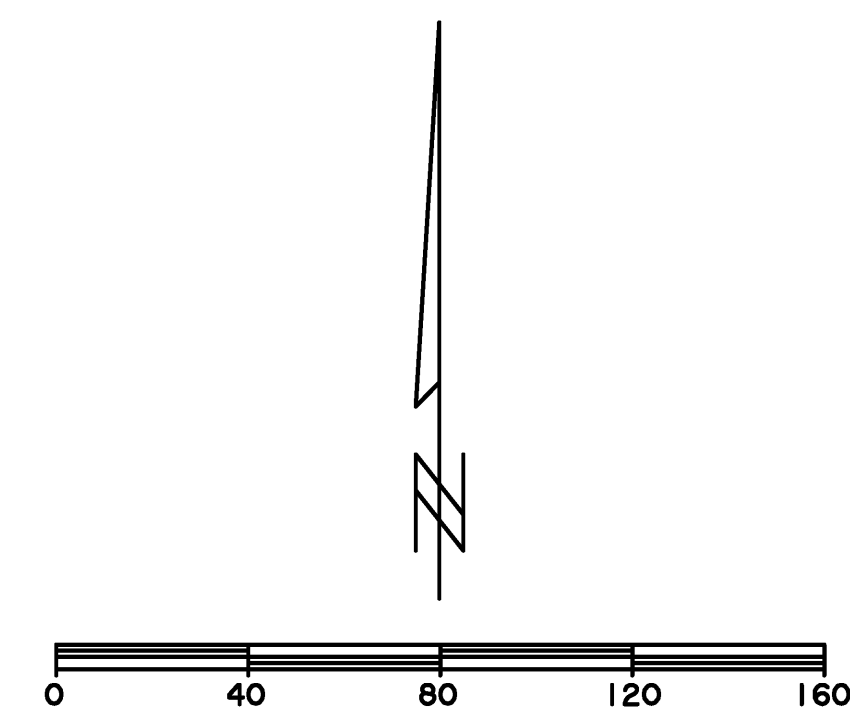
**SHEET NO.
P113**

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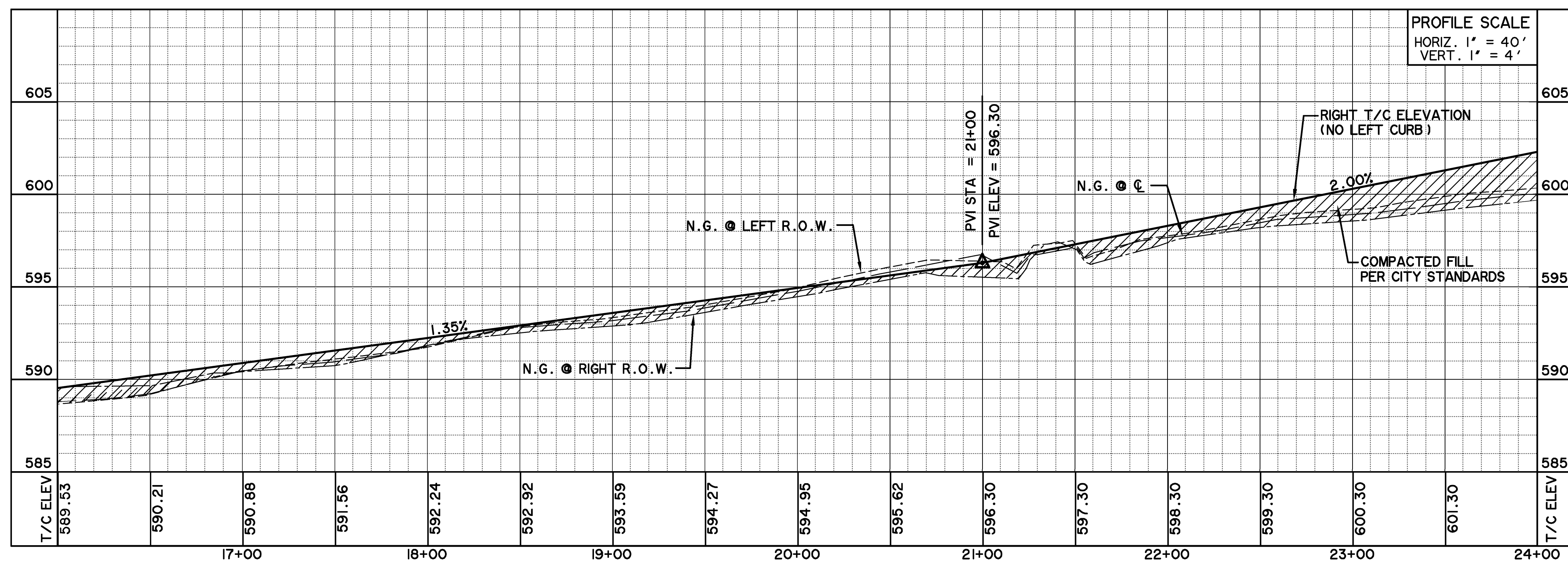
NOTE:

- 1) STREET DIMENSIONS ARE FROM BACK OF CURB TO EDGE OF CONCRETE (BC-EC).
- 2) DESIGN SPEED = 35 MPH



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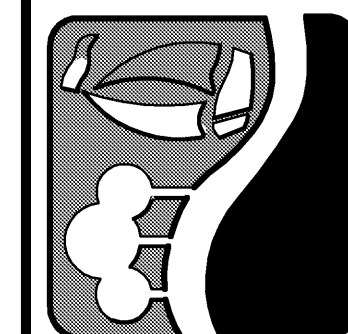
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549. 598.80 FT

BM B AN "X" CUT IN THE BACK OF CURB
LOCATED AT THE NORTH RIGHT-OF-WAY
LINE OF DISCOVERY BOULEVARD ±580' EAST
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BM C - AN "□" CUT IN DISCOVERY
BOULEVARD IN A MEDIAN NOSE ±60' WEST OF
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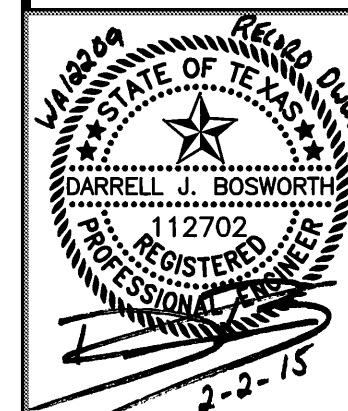
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02/02/2015**

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ROCKWALL TECHNOLOGY PARK PHASE IV

**CAPITAL BOULEVARD
PAVING PLAN AND PROFILE
STA 16+00 TO STA 24+00**



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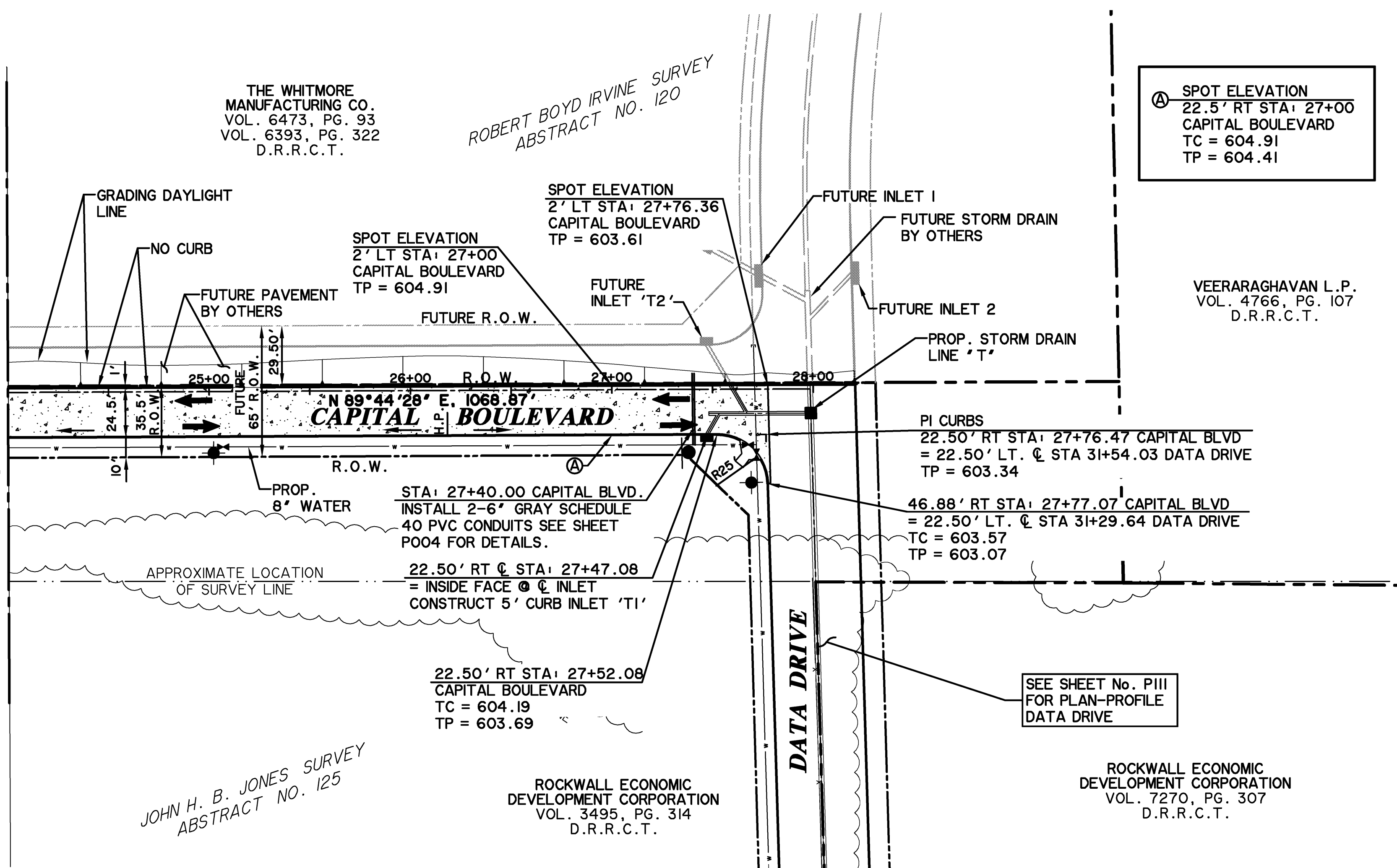
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P114

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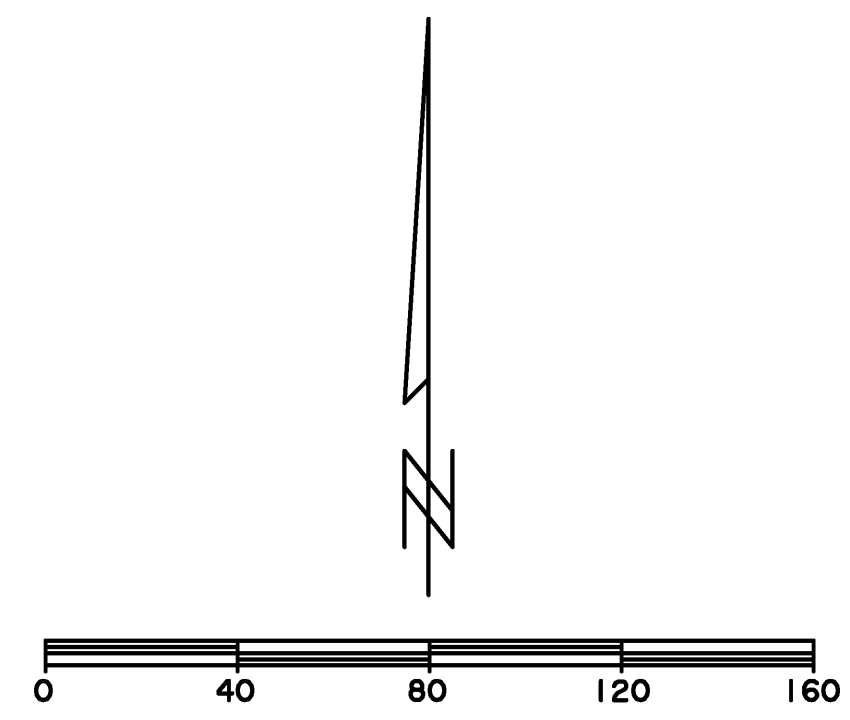
STA. 24+00



NOTE:

1) STREET DIMENSIONS ARE FROM BACK OF CURB TO EDGE OF CONCRETE (BC-EC).

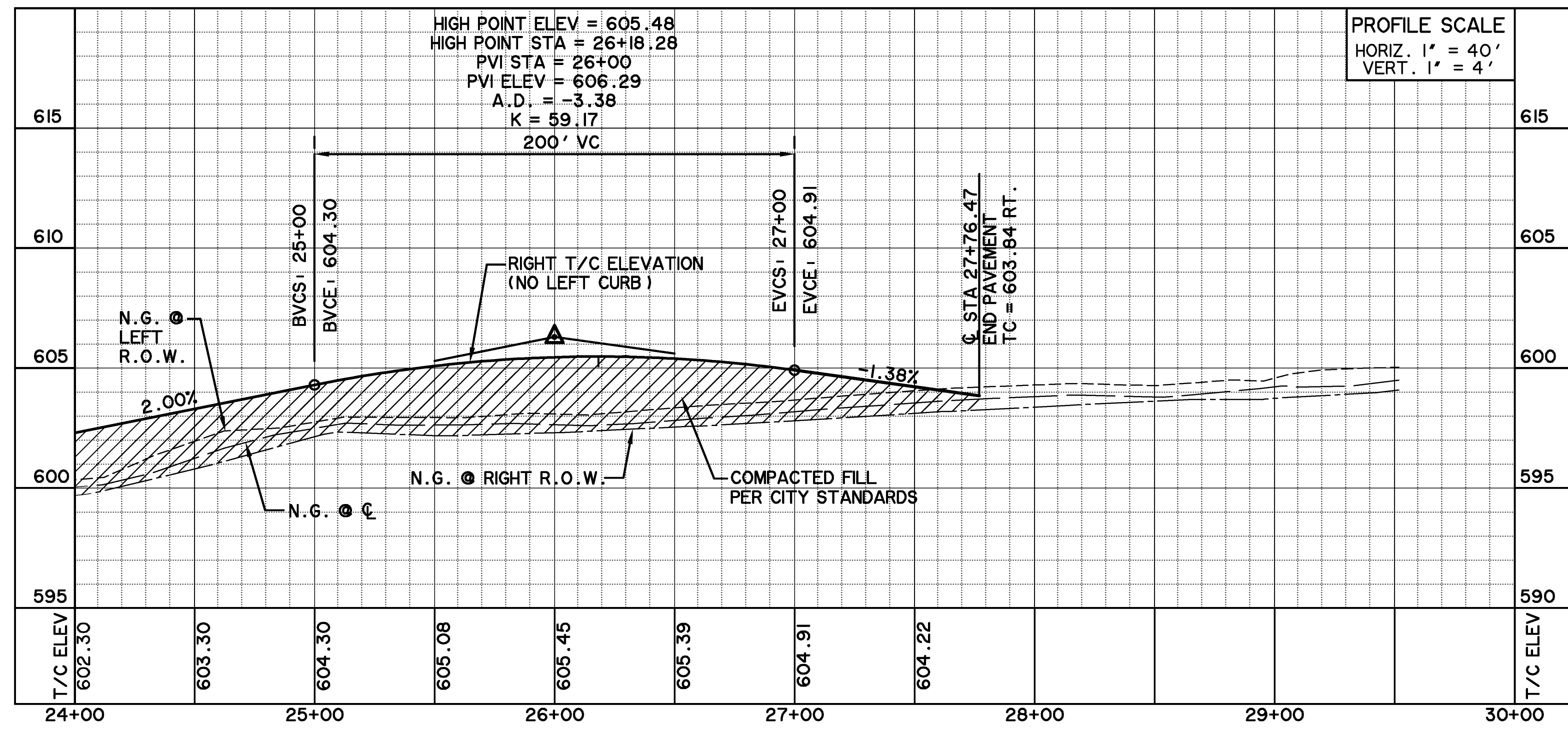
2) DESIGN SPEED = 35 MPH



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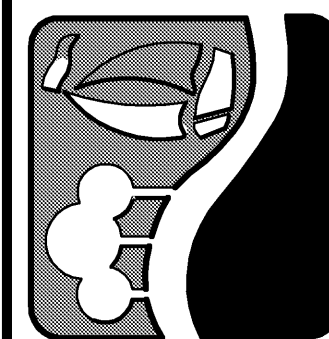
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**RECORD
DRAWING
02/02/2015**

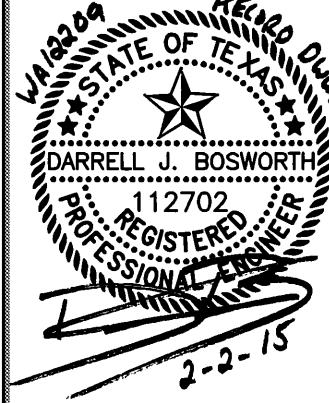
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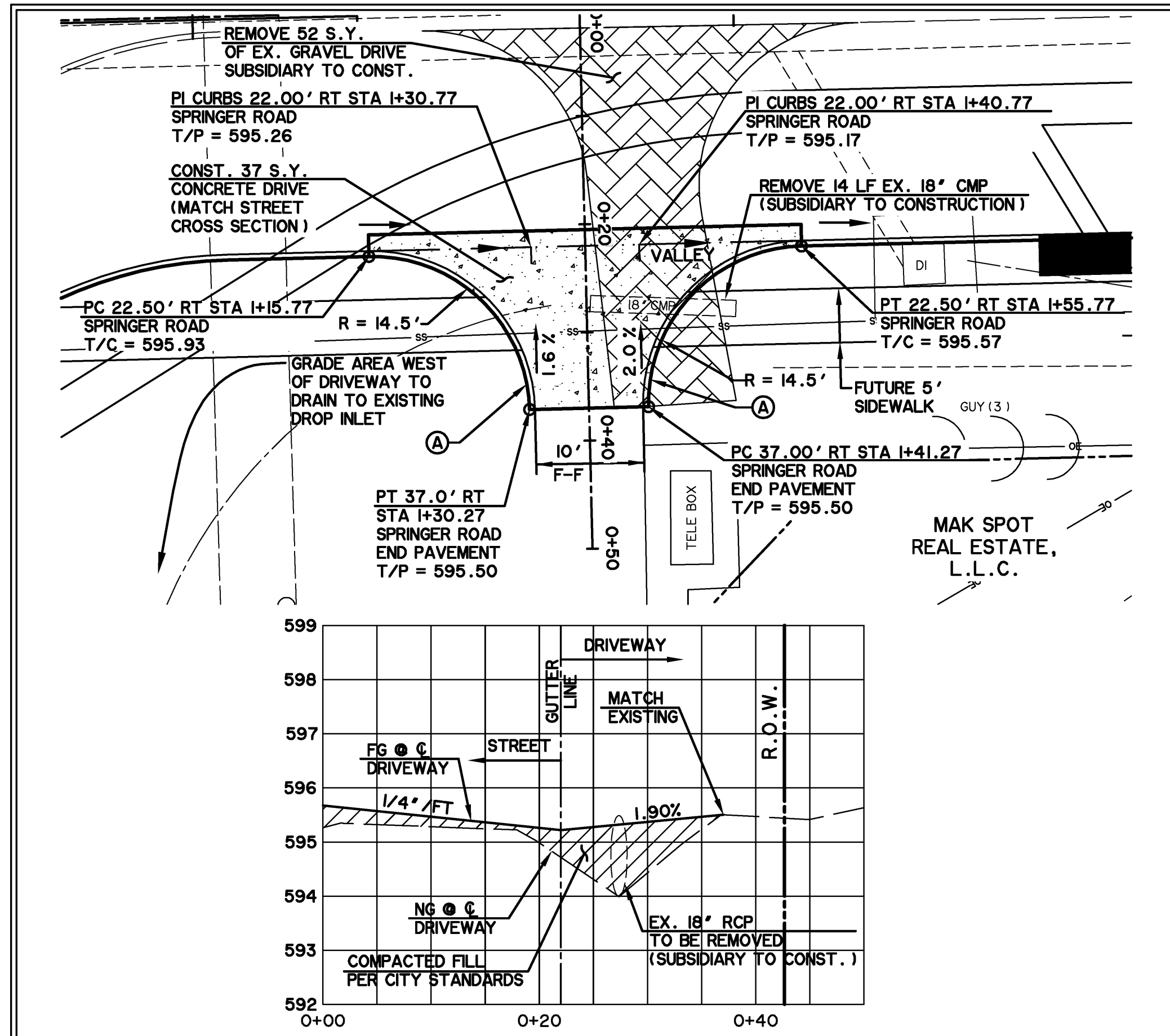
**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**CAPITAL BOULEVARD
PAVING PLAN AND PROFILE
STA 24+00 TO END**

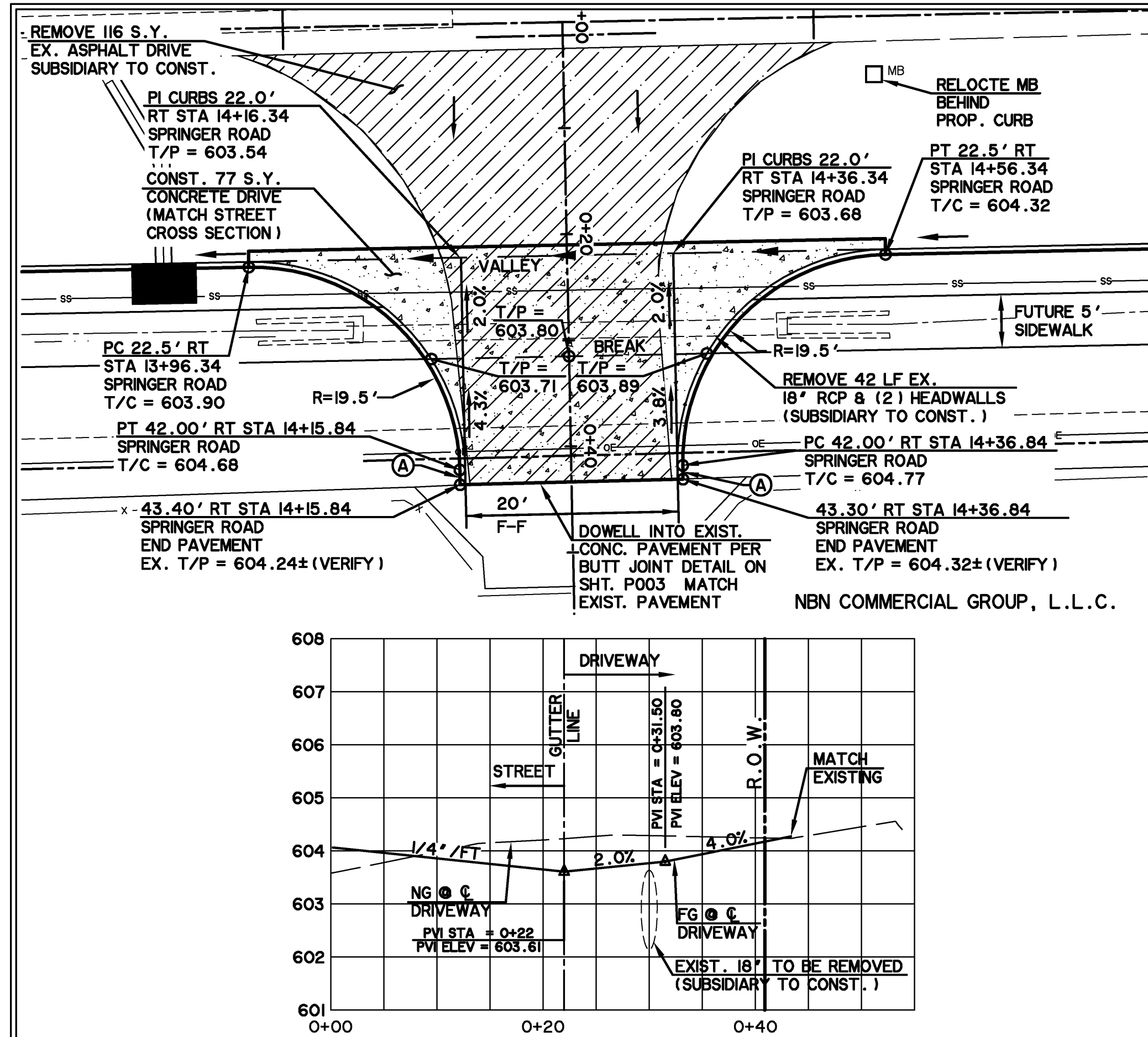


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**SHEET NO.
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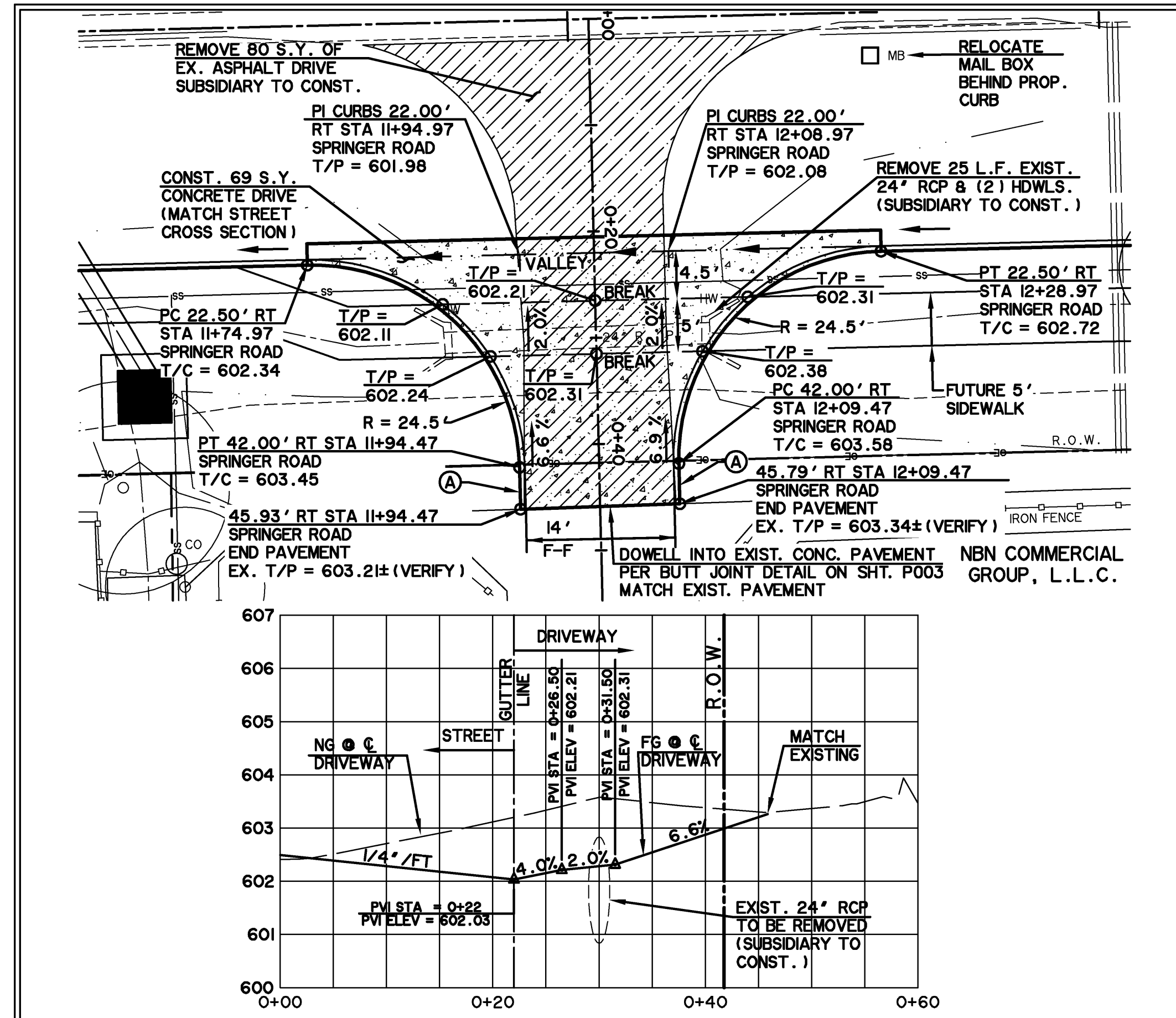
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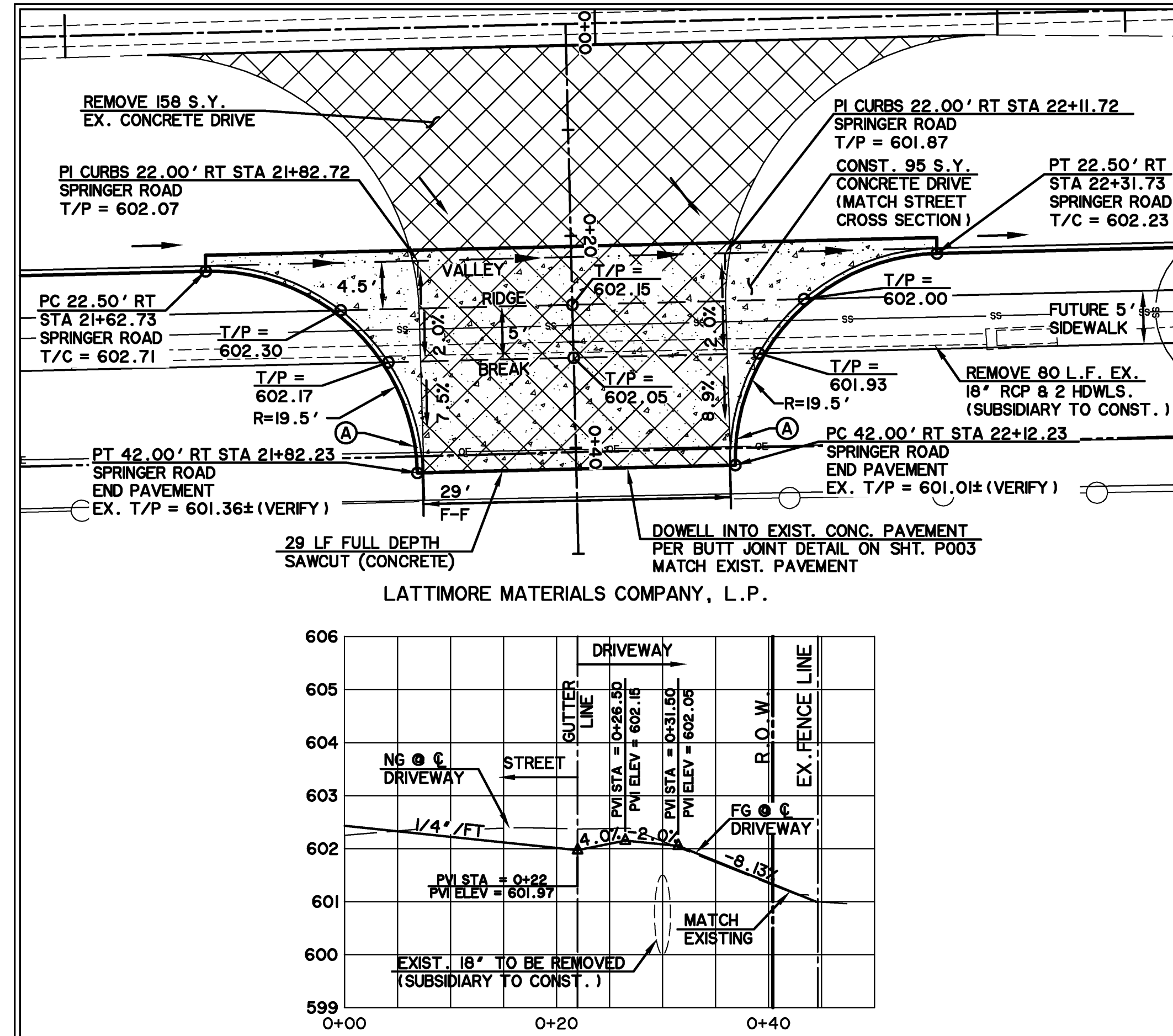
DRIVEWAY 3



DRIVEWAY 2



DRIVEWAY 4



CAUTION !!

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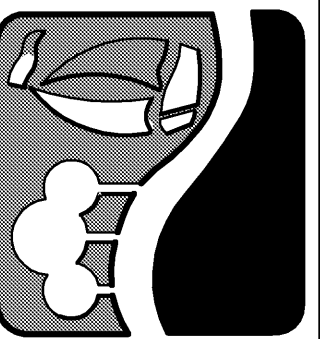
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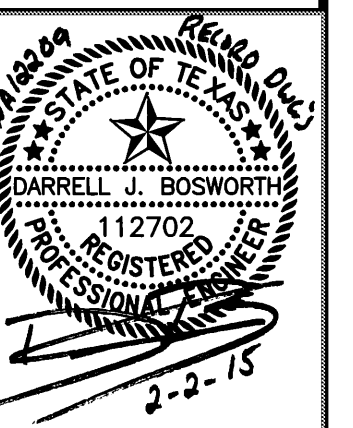
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

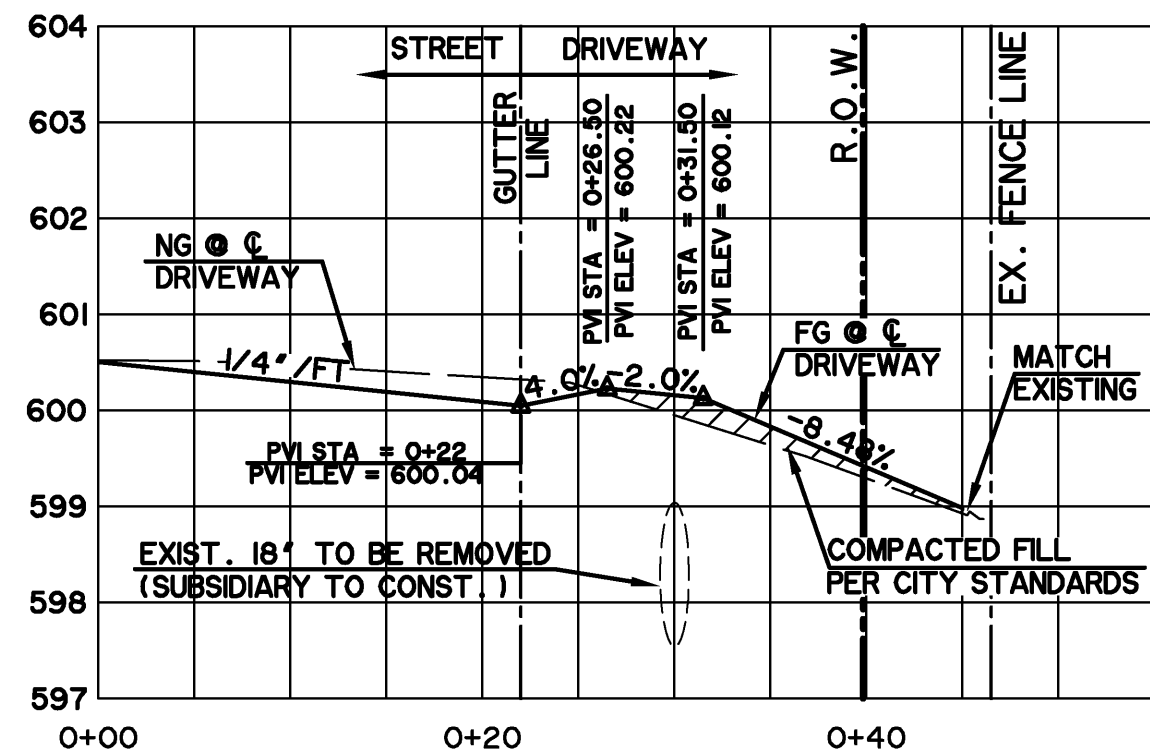
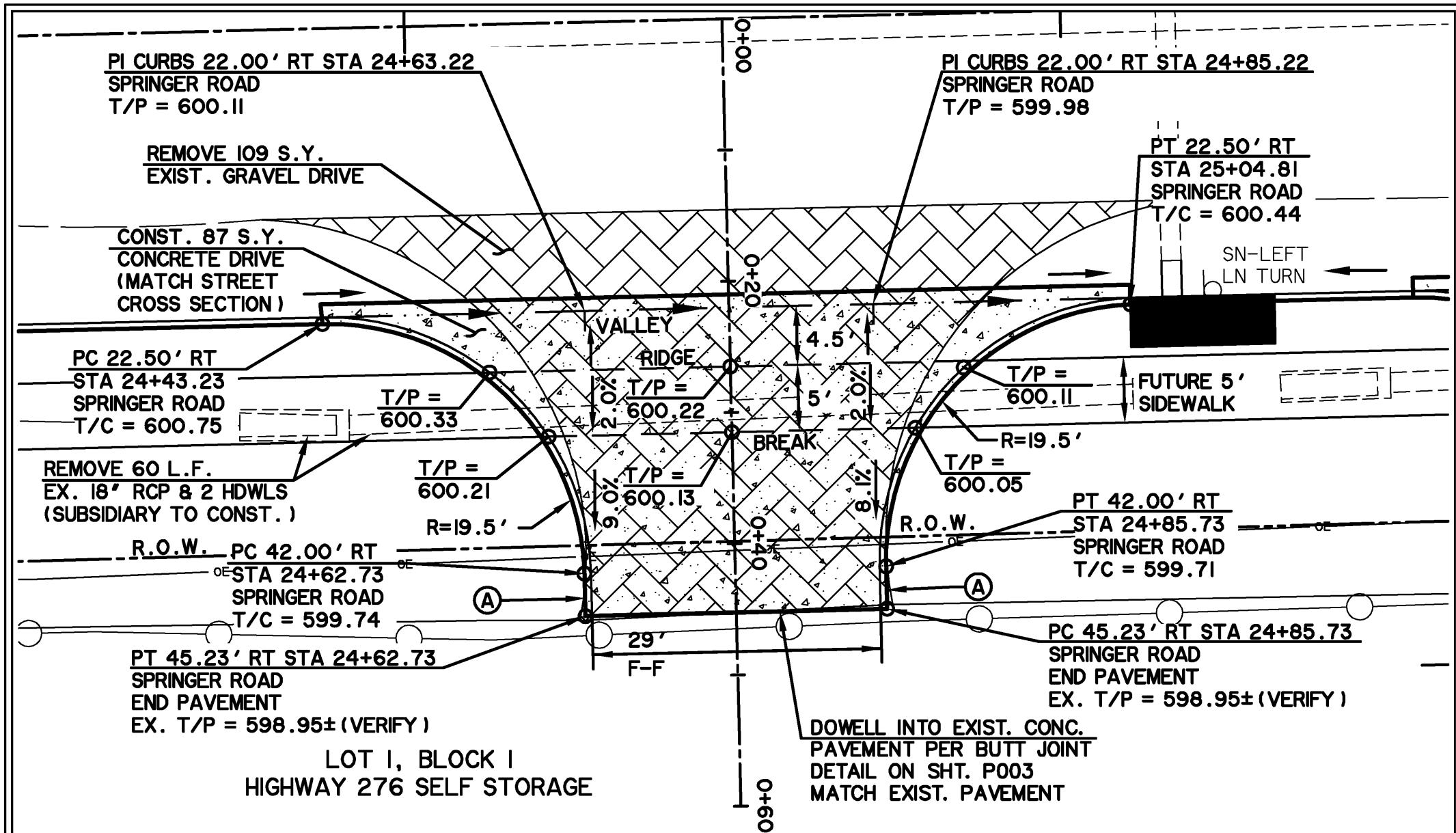
**DRIVEWAYS
SOUTH OF
SPRINGER ROAD**



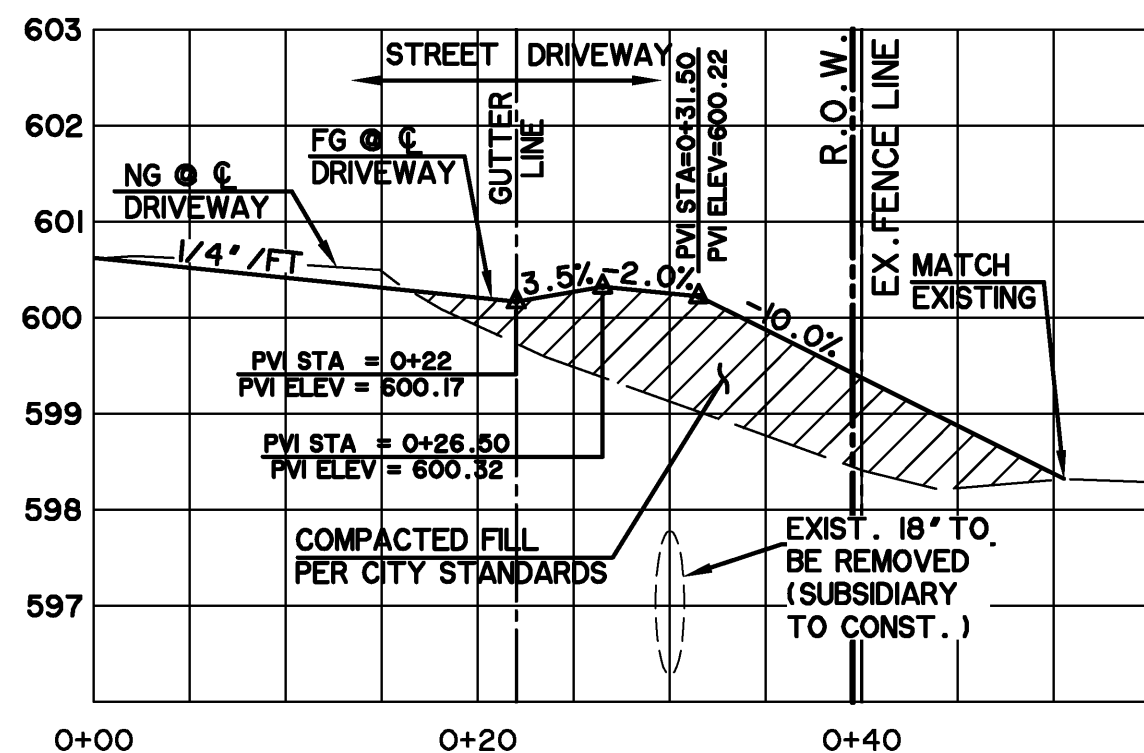
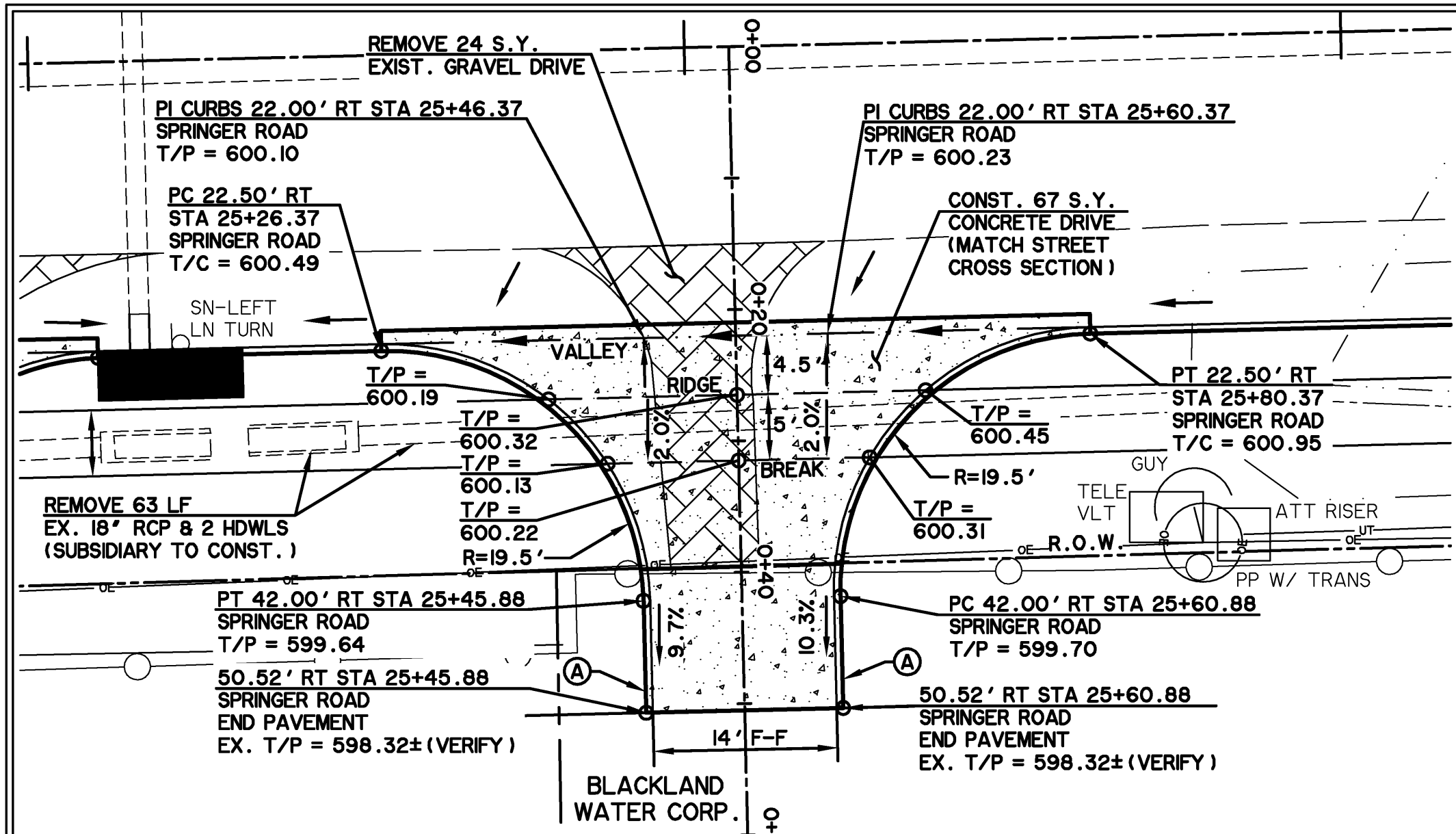
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**SHEET NO.
P116**

DRIVEWAY 5



DRIVEWAY 6



CAUTION !!

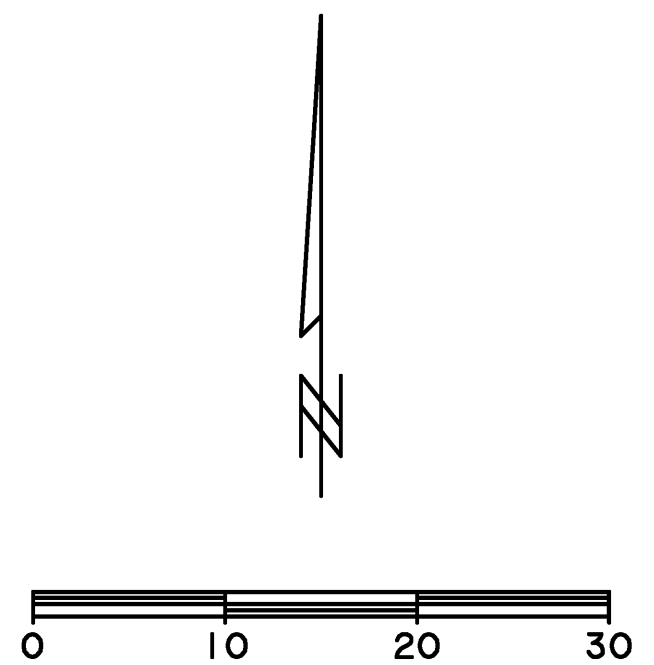
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BOULEVARD IN A MEDIAN NOSE ±60' WEST OF
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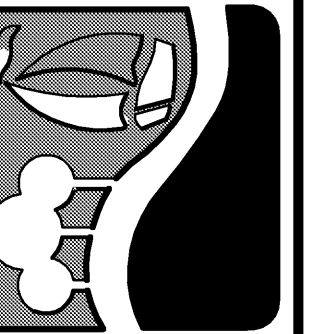


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DRAWING
02/02/2015**

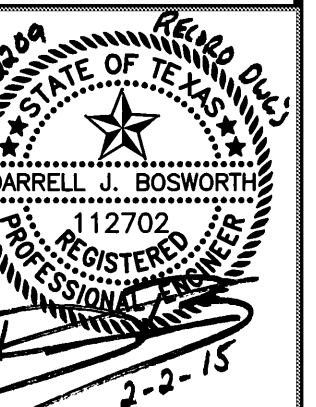
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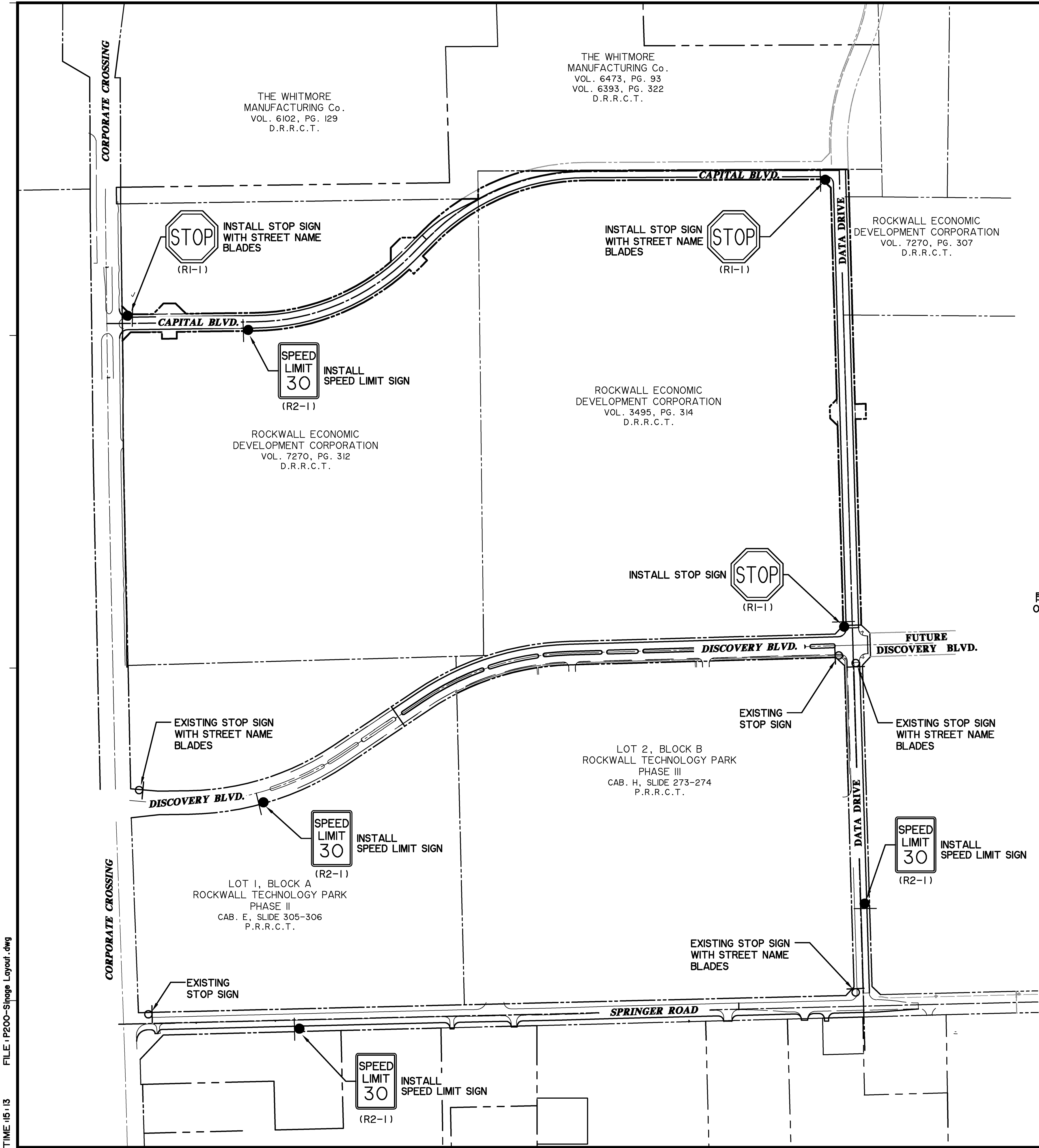
**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**DRIVEWAYS
SOUTH OF
SPRINGER ROAD**



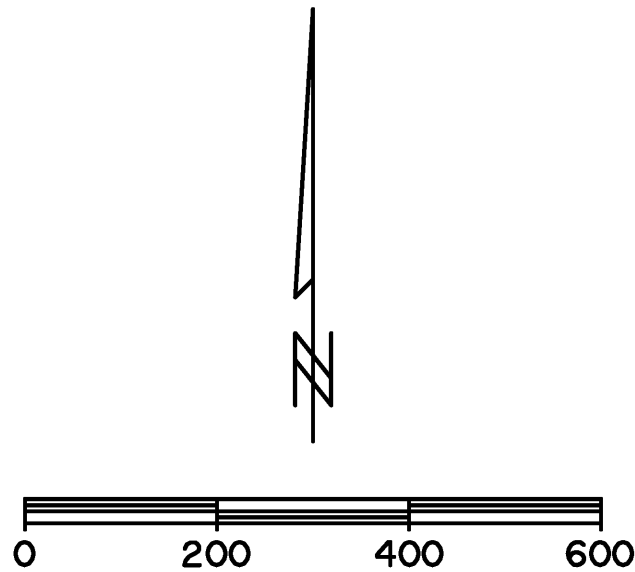
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WA# 12209

**SHEET NO.
P117**



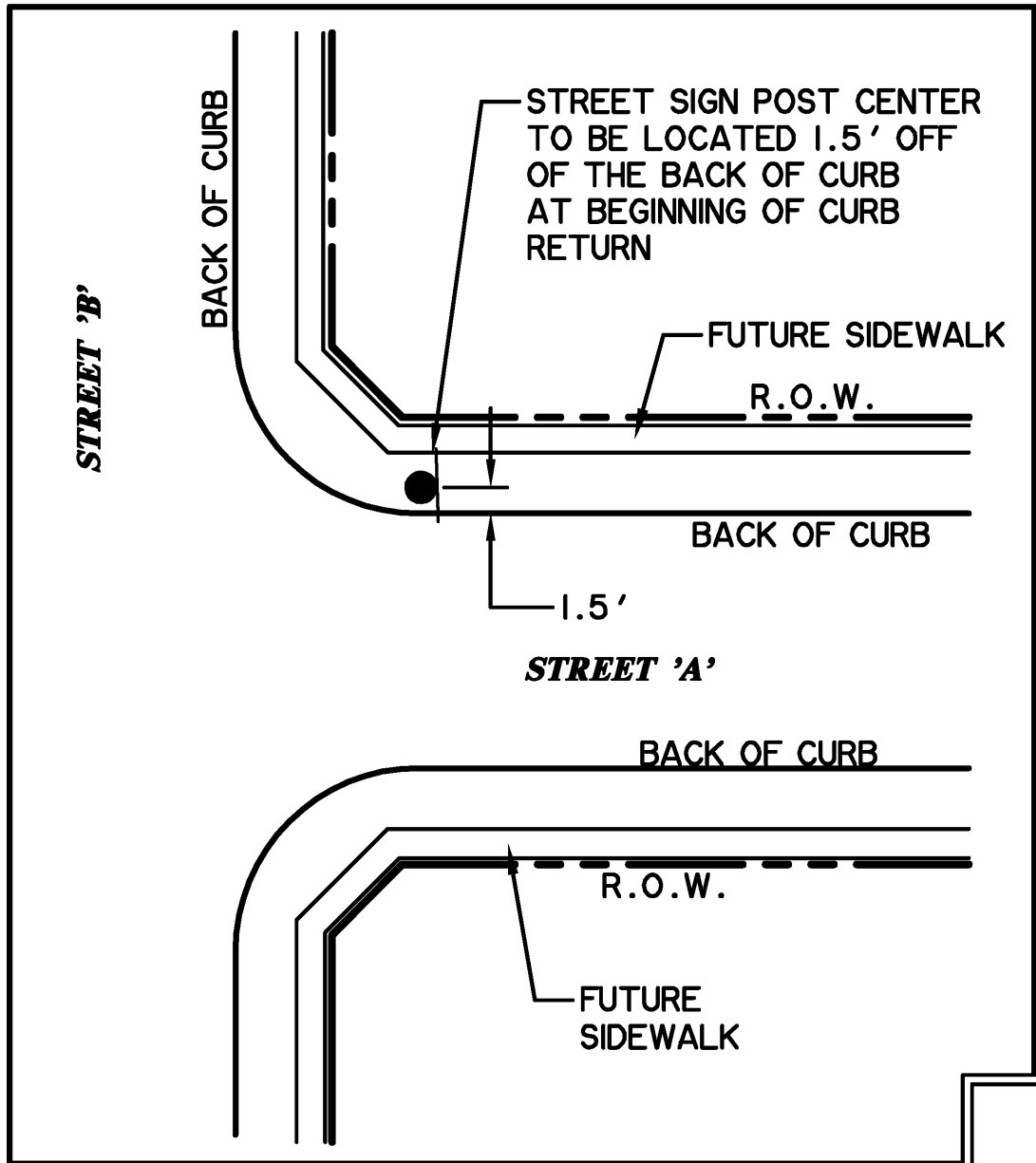
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* BENCH MARKS *	
BM A - AN "X" CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549.	598.80 FT.
BM B - AN "X" CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	599.82 FT.
BM C - AN "□" CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	598.20 FT.

- NOTES**
1. BLOCK NUMBERS ARE REQUIRED ON ALL STREET NAME BLADES.
 2. ALL SIGNAGE INSTALLED SHALL COMPLY WITH THE CURRENT TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES.
 3. STREET NAME BLADES SHALL BE NINE INCHES TALL EXTRUDED ALUMINUM. THE BLADES SHALL BE 0.080 INCHES THICK.
 4. HIGH INTENSITY RETROREFLECTIVE SHEETING FOR STREET, REGULATORY AND WARNING SIGNS SHALL BE DIAMOND GRADE PRISMATIC TYPE III HIGH INTENSITY.
 5. THE LETTERING FOR STREET NAME BLADES SHALL BE HIROAD B-FONT HIGHWAY GOTHIC-8" UPPERCASE 6" TALL LETTERS. LETTERS OF ABBREVIATED STREET DESIGNATIONS SHALL BE 3" TALL AND ALL UPPERCASE (I.E., LN, PKWY, CT, ETC.) BLOCK NUMBERS SHALL BE 3" TALL.
 6. THE STREET BACKGROUND SHALL BE GREEN AND THE LEGEND SHALL BE WHITE.
 7. STREET BLADE MUST INCORPORATE THE CURRENT CITY OF ROCKWALL LOGO.
 8. STREET SIGN POST CENTER SHALL BE LOCATED 1.5' FROM BACK OF CURB.



TYPICAL PLACEMENT DETAIL
N.T.S.

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02/02/2015**

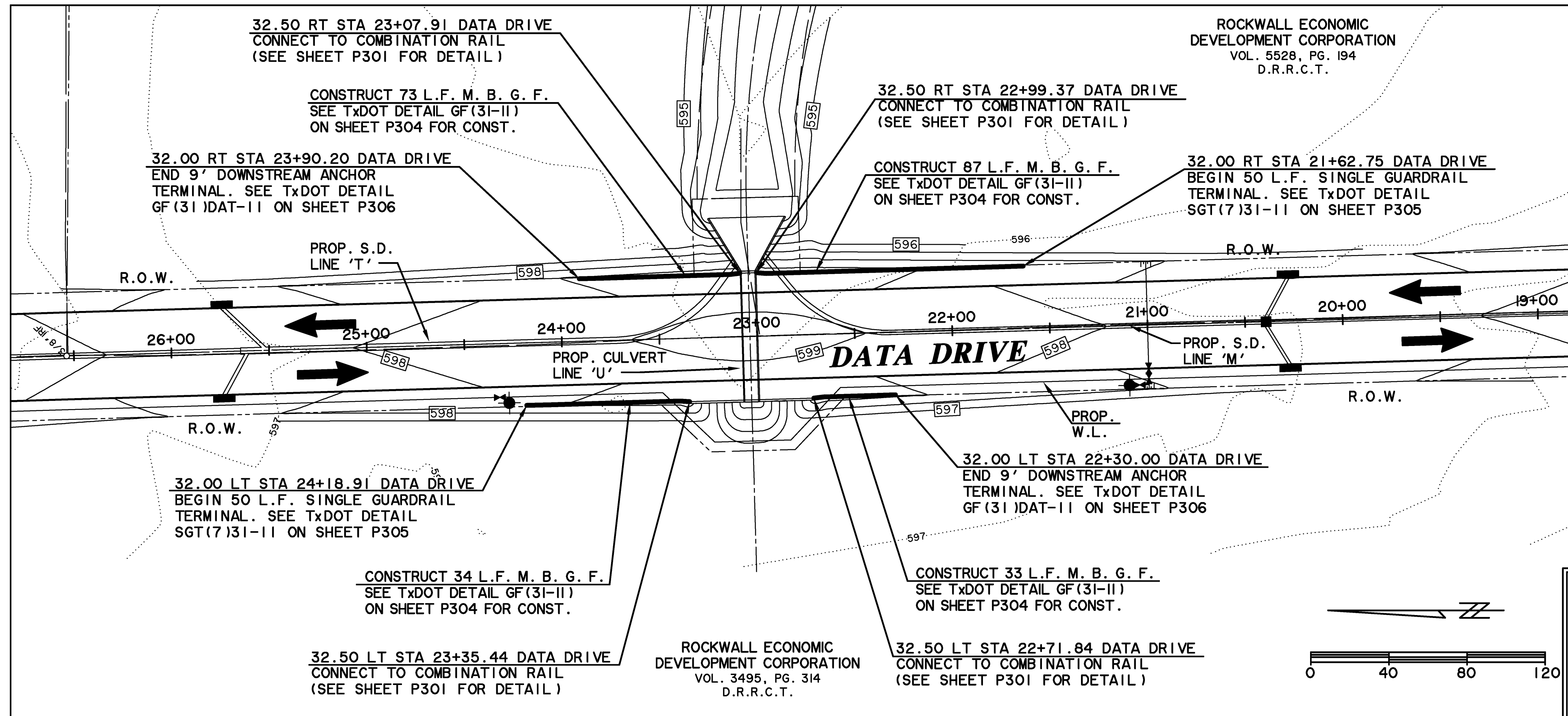
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701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV
SIGNAGE LAYOUT**

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LAST SHEET OF 2
DATE 10-21-2013
WA# 12209

**SHEET NO.
P200**



← TRAFFIC FLOW DIRECTION

* BENCH MARKS *

BM A - AN "X" CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.

BM B - AN "X" CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.

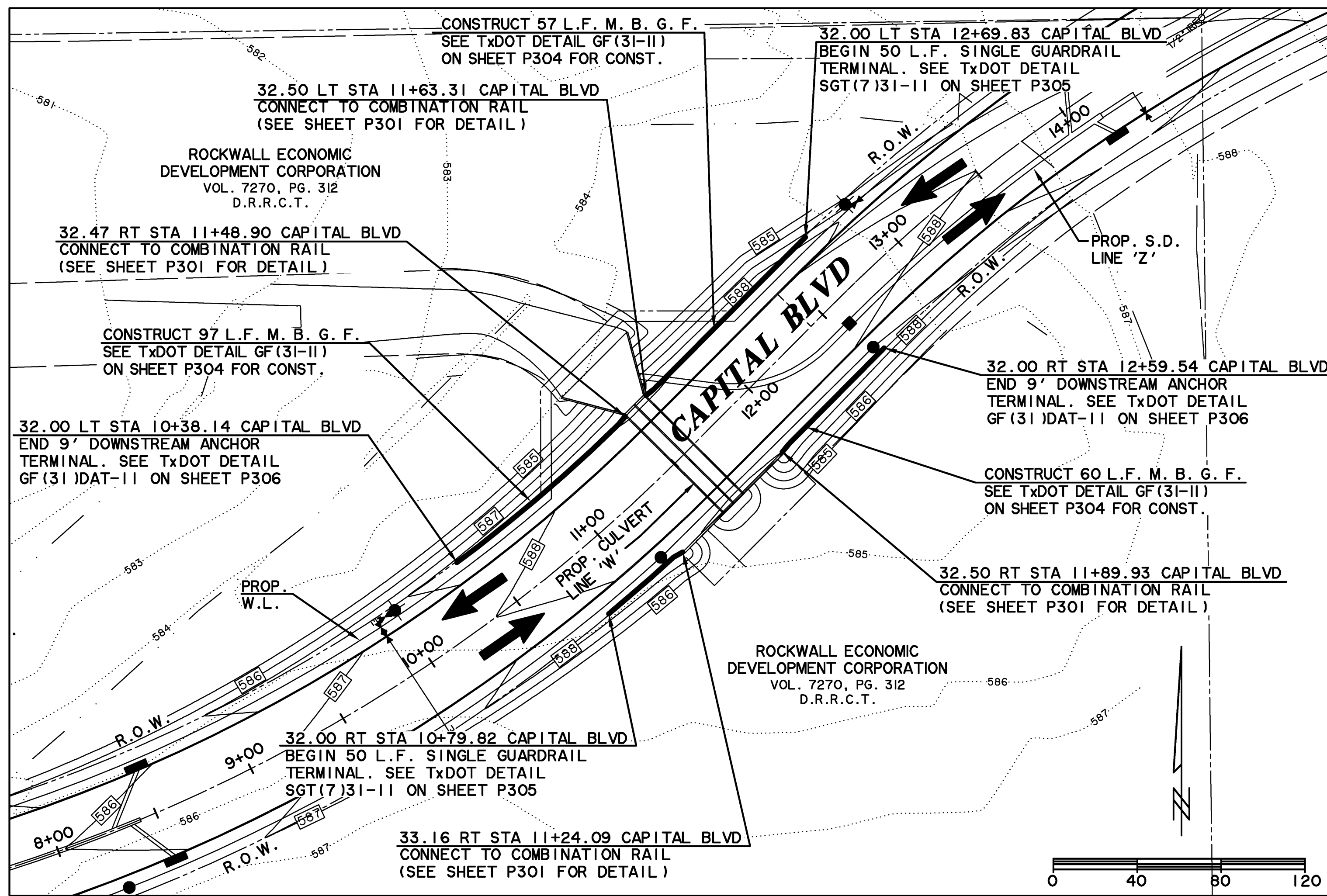
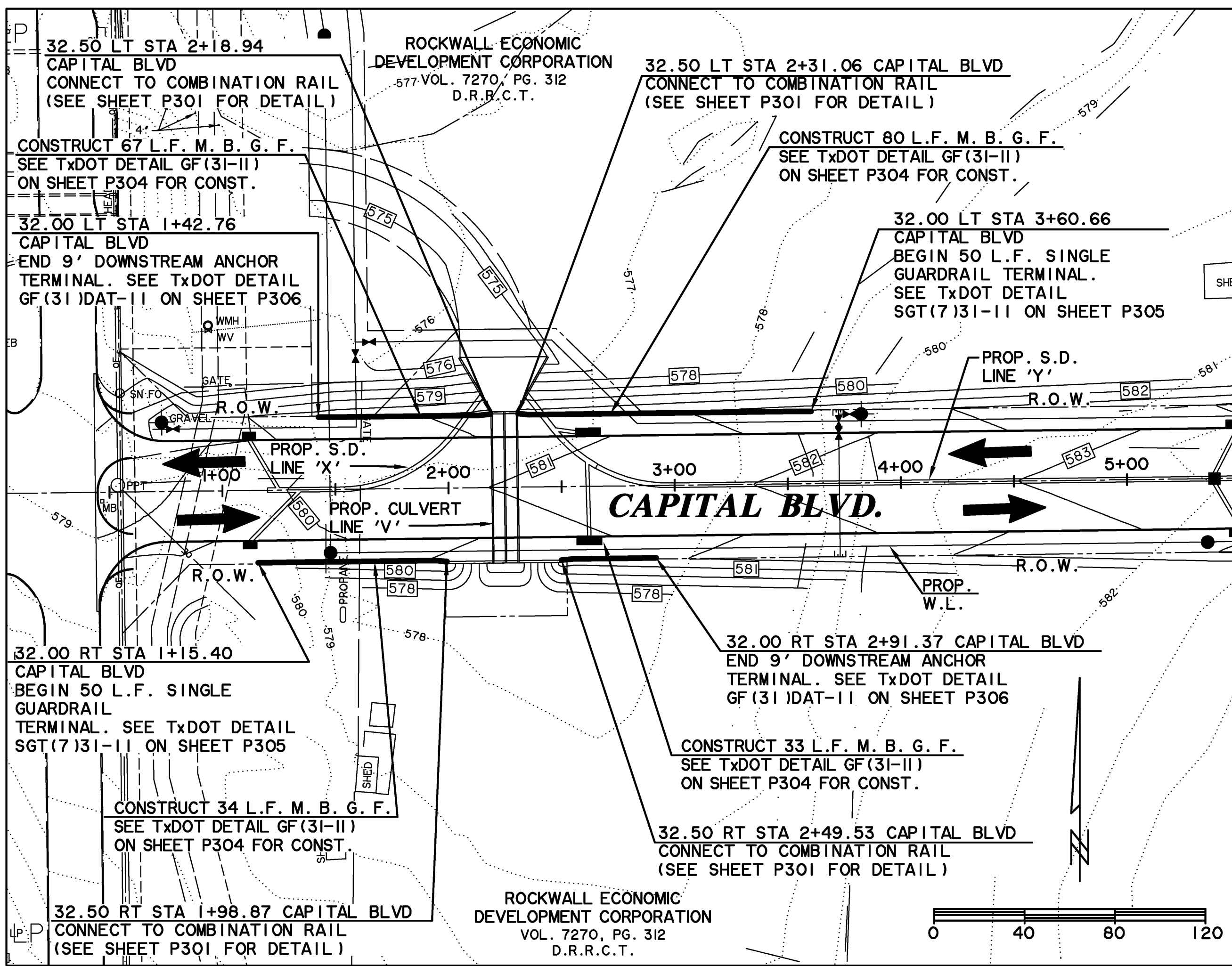
BM C - AN "X" CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

CAUTION !!

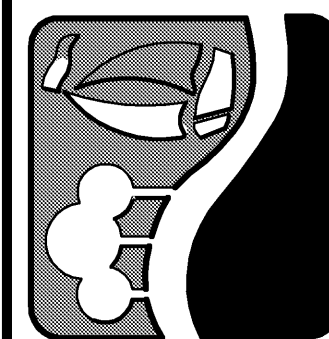
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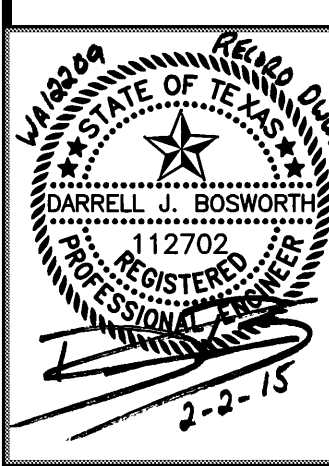


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ROCKWALL TECHNOLOGY PARK
PHASE IV

GUARDRAIL LAYOUT



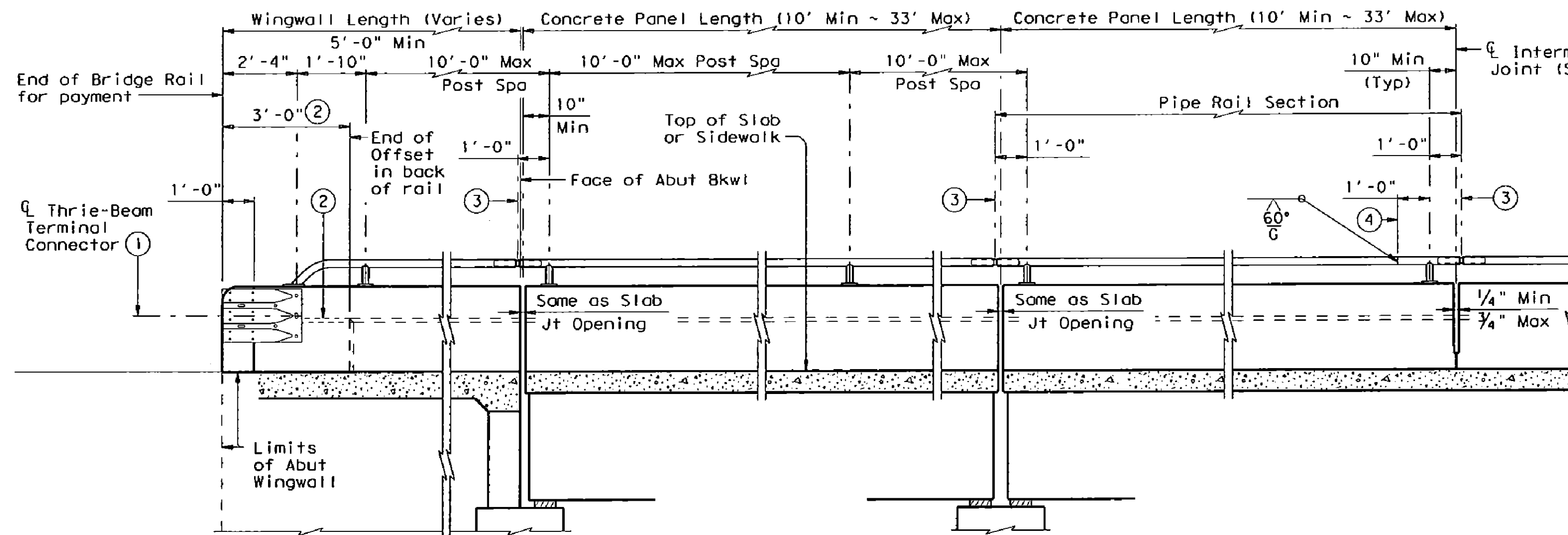
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SHEET NO.
P202

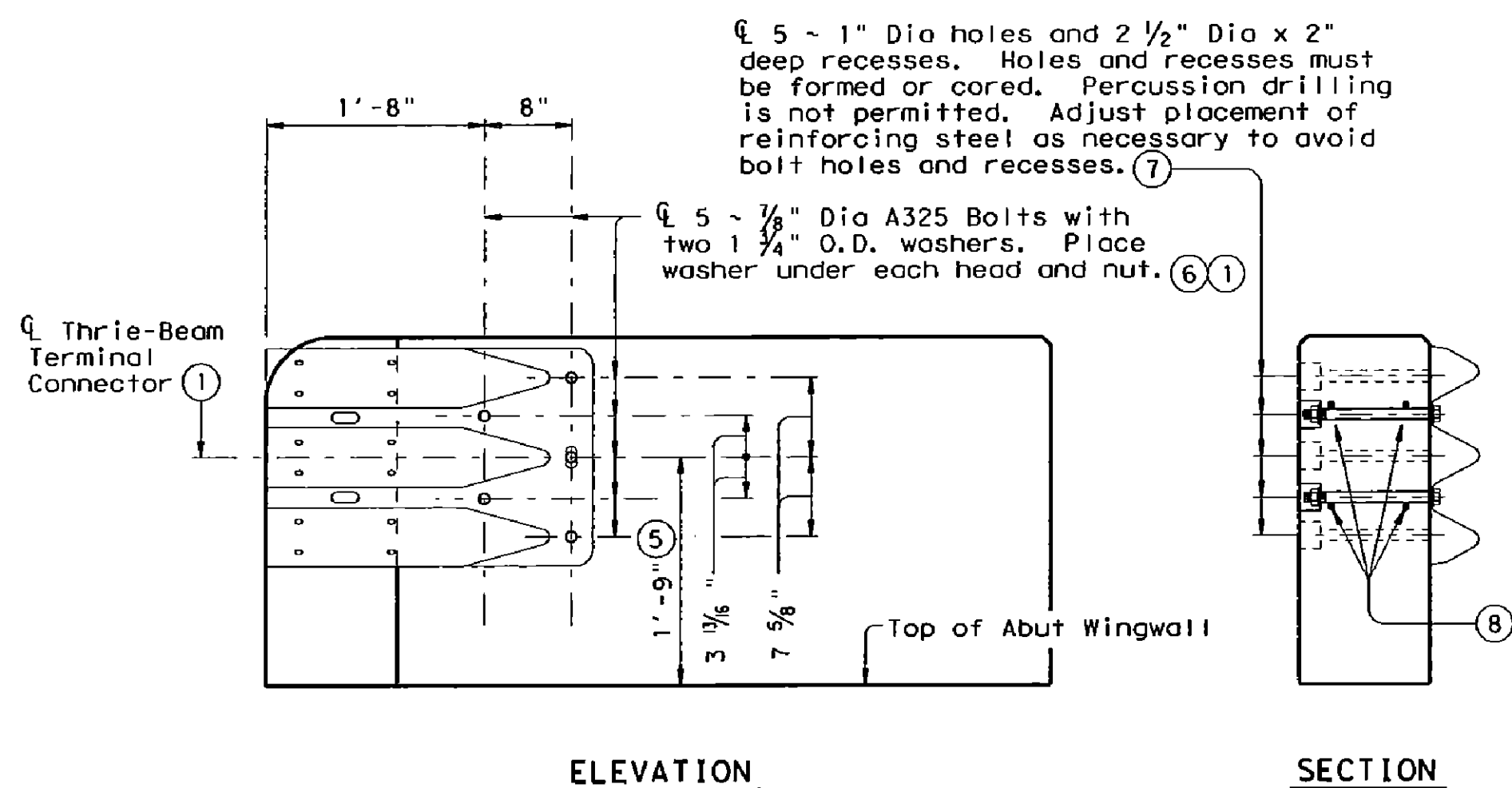
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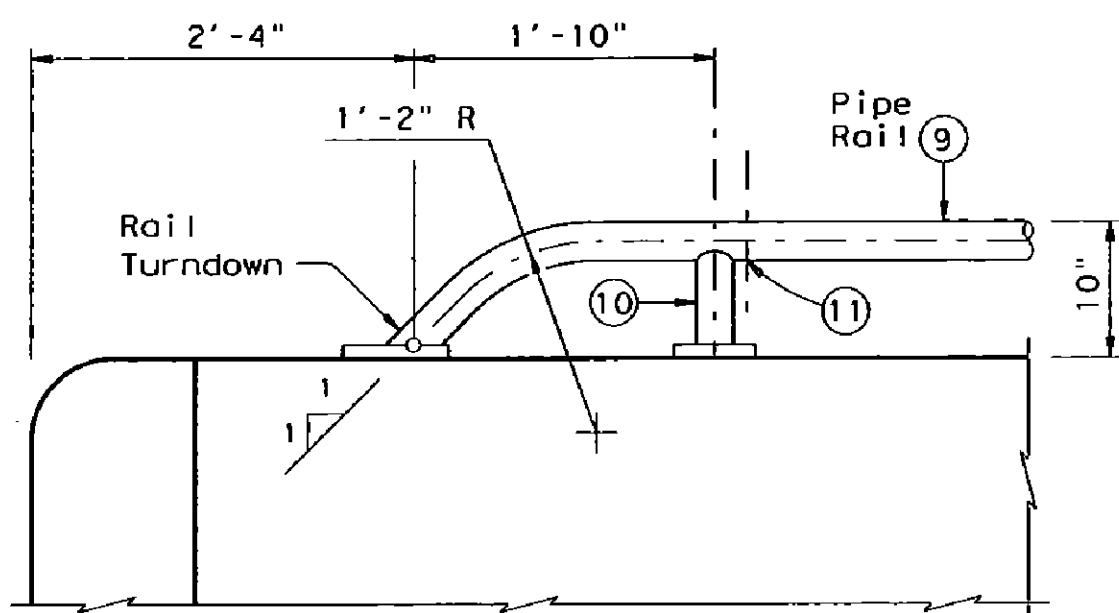
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ROADWAY ELEVATION OF RAIL

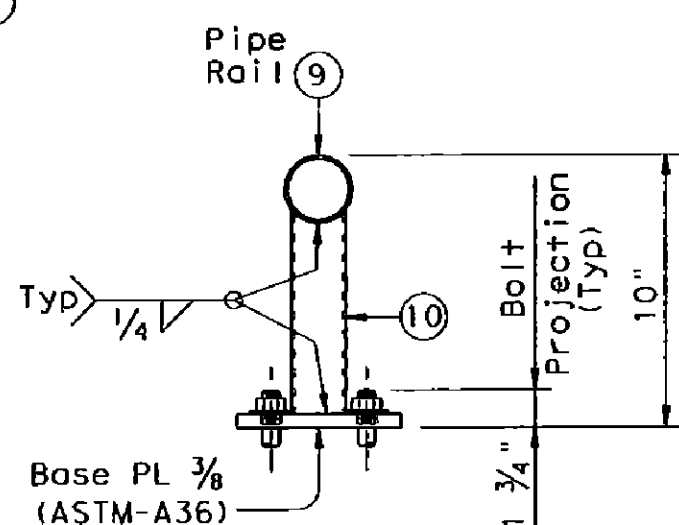


TERMINAL CONNECTION DETAILS

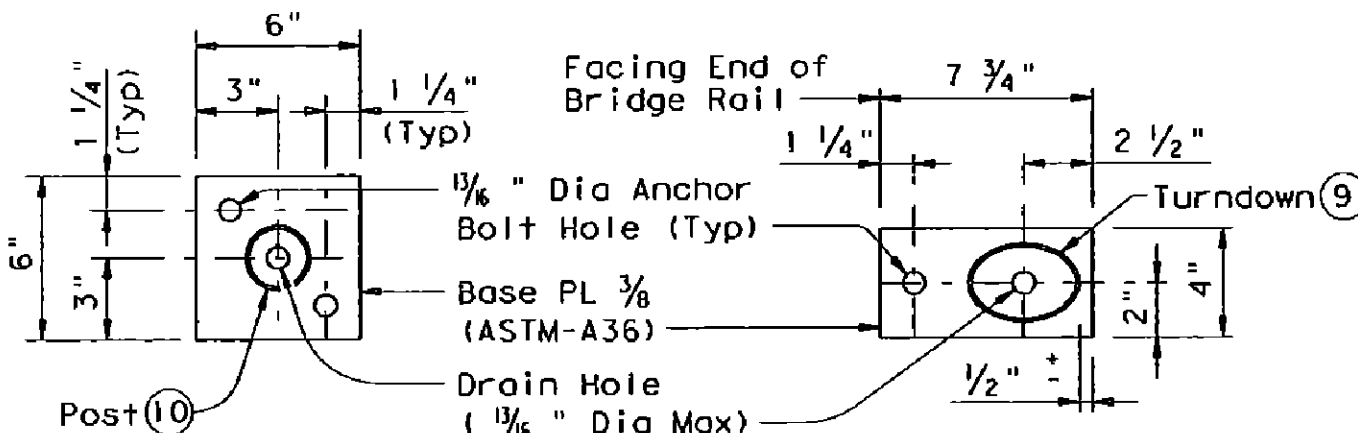


Note that at least two anchor points (as shown) are required for the Bridge Rail on the Abutment Wingwall. Longer Wingwalls may require more than two Rail anchorages.

PIPE RAIL TERMINAL DETAIL



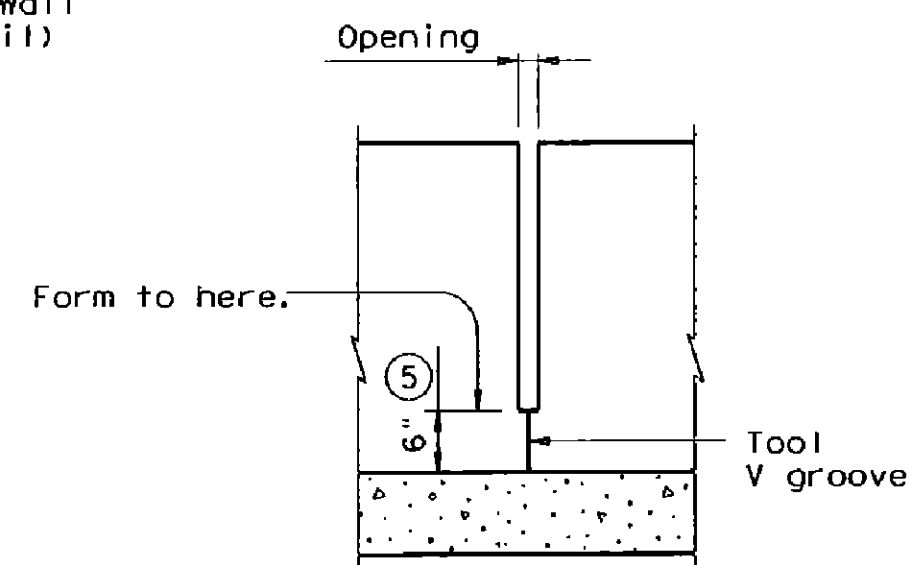
TRANSVERSE SECTION



POST BASE PLATE PLAN

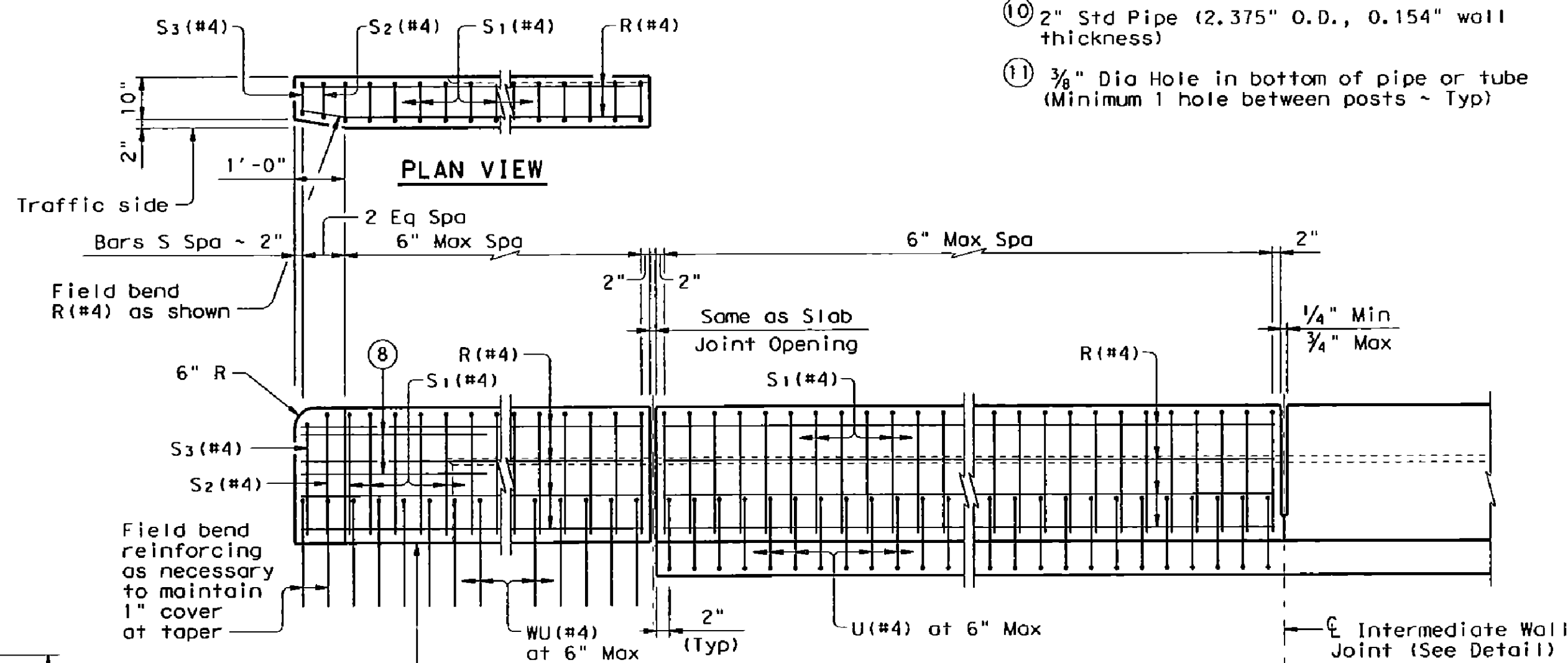
RAIL TURNDOWN BASE PLATE PLAN

PIPE RAIL DETAILS



INTERMEDIATE WALL JOINT DETAIL

Provide at all interior bents without slab expansion joints. Space equally in between at 33' Max, 10' Min. Location independent of pipe rail splices.



ELEVATION SHOWING TYPICAL REINFORCING PLACEMENT (Showing without raised sidewalk)

- Terminal Connectors and associated hardware are to be paid for under the item "Metal Beam Guard Fence". Metal Beam Guard Fence Transitions must be attached to the bridge rail and extended along the embankment unless otherwise shown in the plans.
- Back of rail offset may, with Engineer's approval be continued to the end of the railing.
- Exp Joint or Splice Joint as required.
- One shop splice per pipe rail section is permitted with minimum 85 percent penetration. The weld may be square groove, or single vee groove. Grind smooth.
- Increase 2" for structures with overlay.
- Bolts must be of sufficient length to extend 1/2" to 3/4" beyond nut.
- Bolt recesses are only required when pedestrian sidewalks are adjacent to back of rail.
- 4 additional Bars R(#4) 3'-8" in length must be placed inside Bars S(#4) and centered 2'-0" from end of rail when Terminal Connections are required. Field bend as needed.
- 2 1/2" Std Pipe (2.875" O.D., 0.203" wall thickness)
- 2" Std Pipe (2.375" O.D., 0.154" wall thickness)
- 3/8" Dia Hole in bottom of pipe or tube (Minimum 1 hole between posts - Typ)

The use of this railing is restricted to design speeds of 45 mph or less.

RECORD DRAWING
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SHEET 1 OF 3

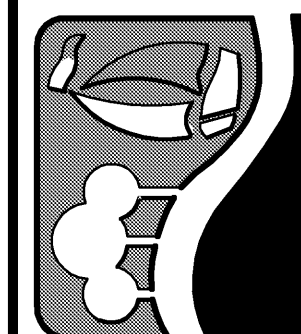
Texas Department of Transportation
Bridge Division

COMBINATION RAIL

TYPE C221

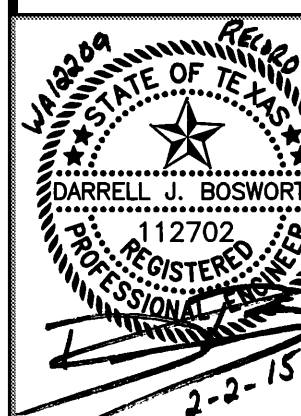
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TXDOT April 2009	DISTRICT	FEDERAL AID PROJECT	SHEET	
REVISIONS				
05-11: Wall Joint Note				
07-12: Guardrail Transition				

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ROCKWALL TECHNOLOGY PARK
PHASE IV

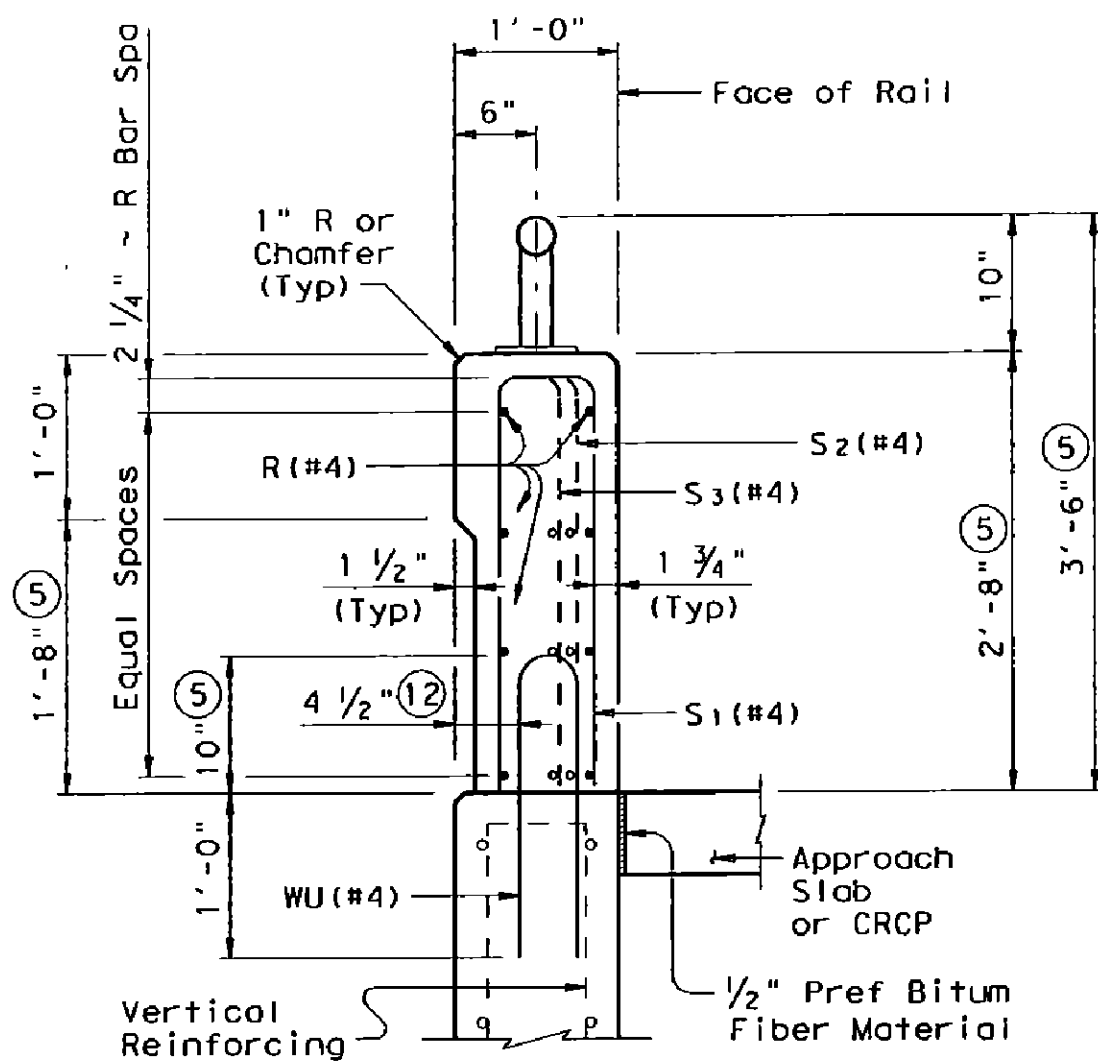
TXDOT
COMBINATION RAIL
TYPE C221 (1 OF 3)



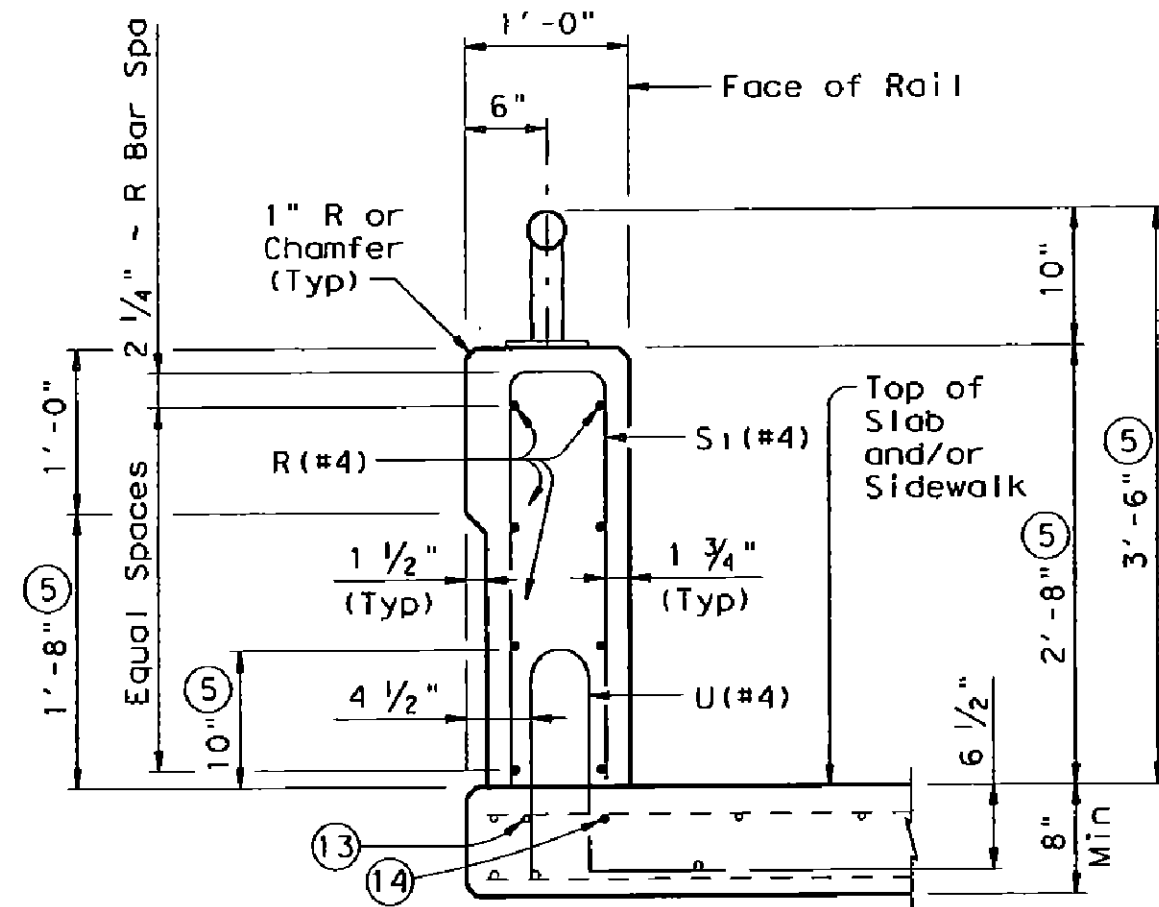
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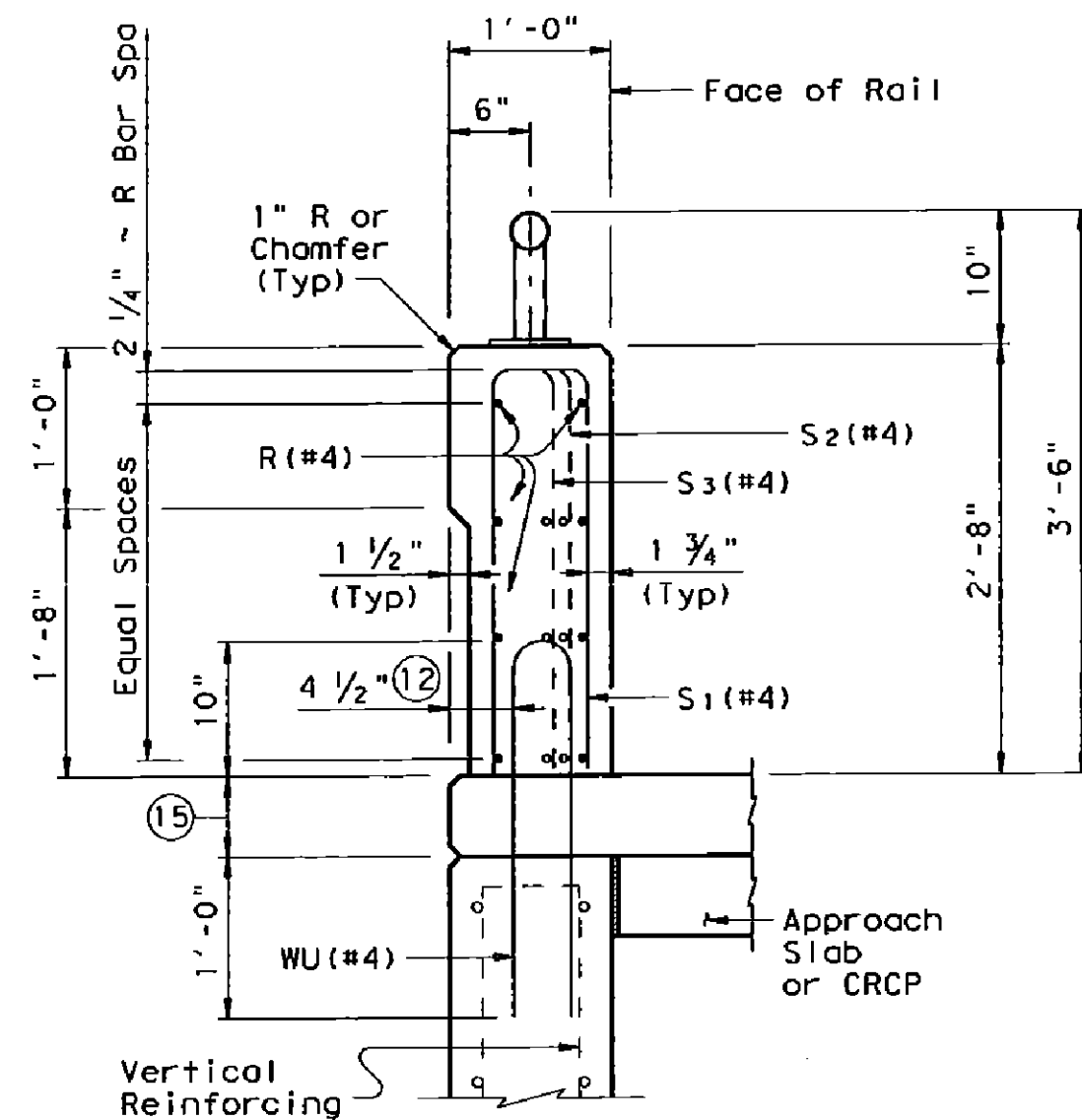


ON ABUTMENT WINGWALLS OR CIP RETAINING WALLS

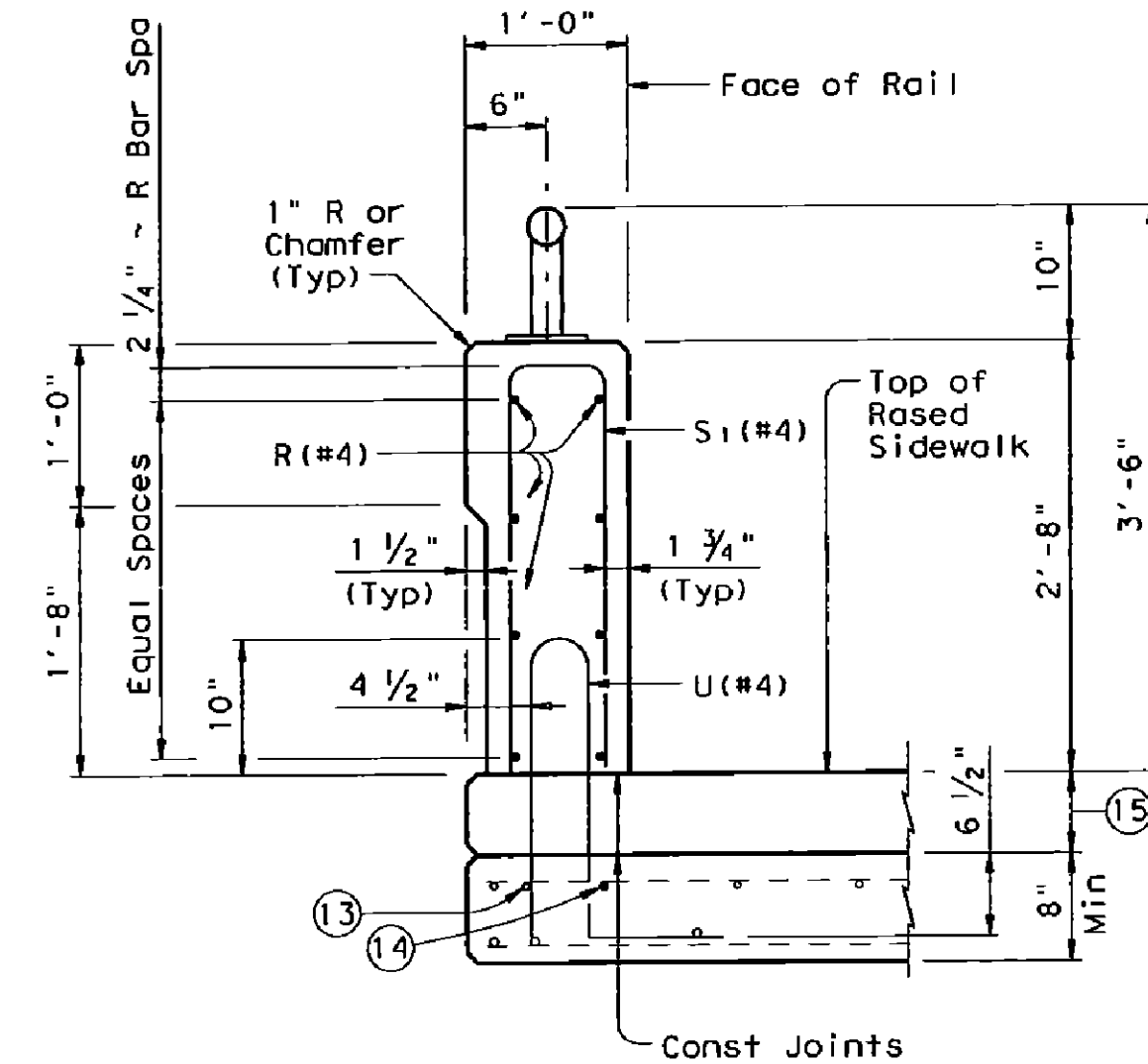


ON BRIDGE SLAB

SECTIONS THRU RAIL WITHOUT RAISED SIDEWALK

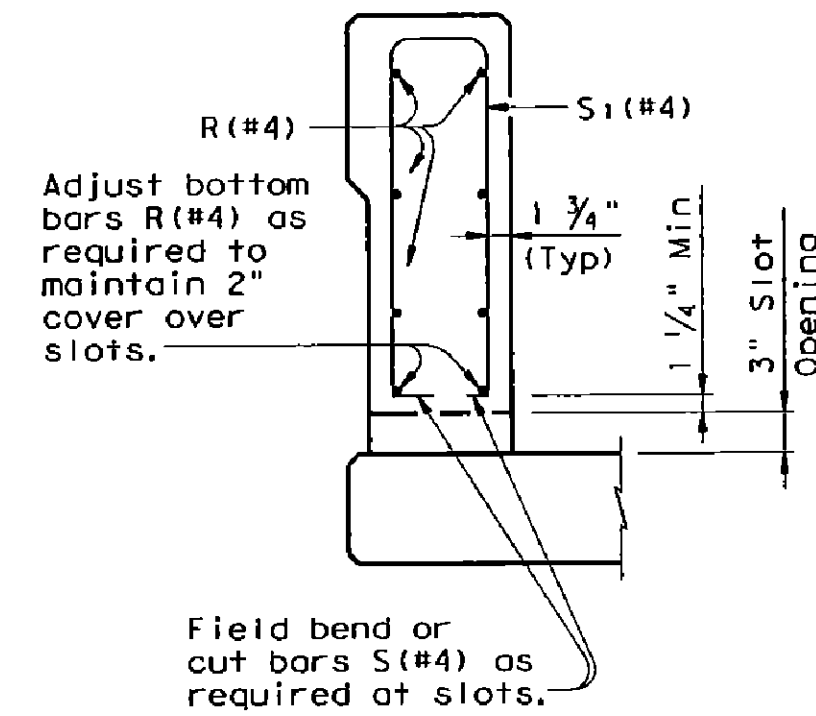


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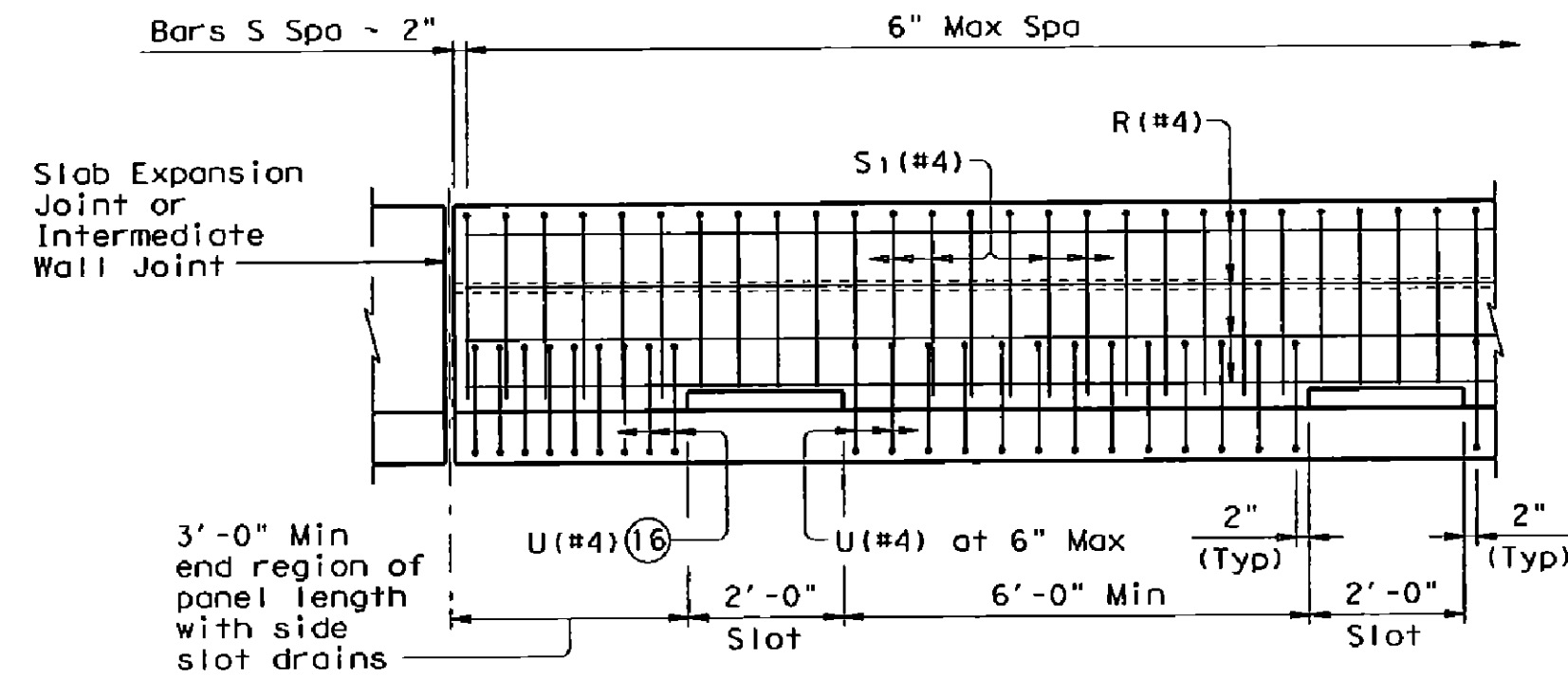


ON BRIDGE SLAB

SECTIONS THRU RAIL WITH RAISED SIDEWALK



SECTION THRU OPTIONAL SIDE SLOT DRAIN



OPTIONAL SIDE SLOT DRAIN DETAIL

Note: Side Slot Drains may be used where shown elsewhere on the plans or as directed by the Engineer. Drains should not be placed over railroad tracks, lower roadways, or sidewalks. When this rail is used as a separator between a roadway surface and a sidewalk surface, side drain slots will not be permitted.

RECORD DRAWING
02/02/2015

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SHEET 2 OF 3

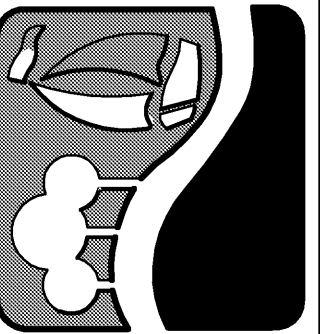
Texas Department of Transportation
Bridge Division

COMBINATION RAIL

TYPE C221

FILE: r1std018.dgn	DR: TXDOT	CR: TXDOT	DN: JTR	CK: JWH
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REVISIONS				
05-11: Wall Joint Note.				
07-12: Guardrail Transition.				

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ROCKWALL TECHNOLOGY PARK
PHASE IV

TXDOT
COMBINATION RAIL
TYPE C221 (2 OF 3)



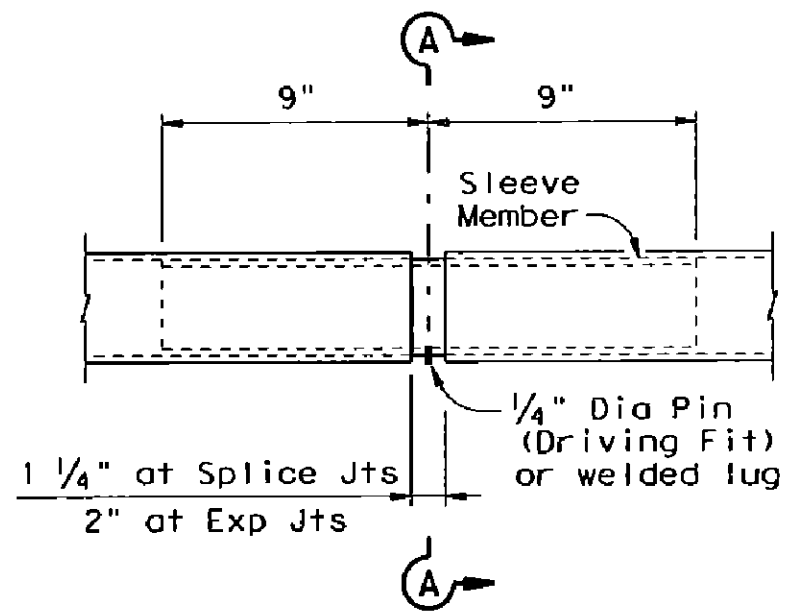
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SHEET NO.
P302

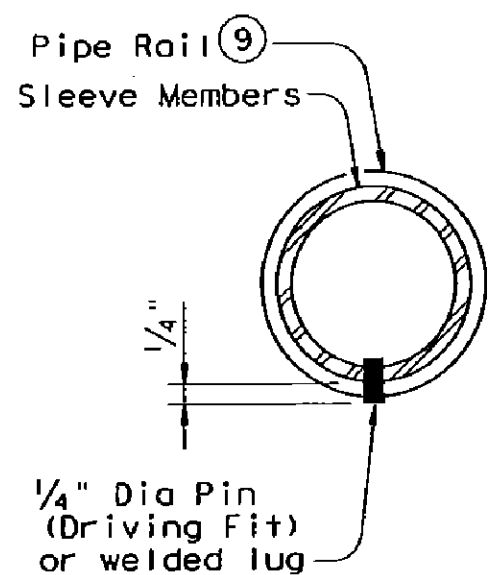
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RAIL DATA FOR HORIZONTAL CURVES		
	RADIUS TO FACE OF RAIL	MAX CHORD LENGTH
Pipe Rail	Over 2800'	29'-0"
	Over 1400' thru 2800'	14'-6"
	Over 700' thru 1400'	7'-3"
	Thru 700'	Zero
		CONSTRUCT OR FABRICATE
		Straight rail panels
		To required radius or to chords shown (17)
		To required radius (17)



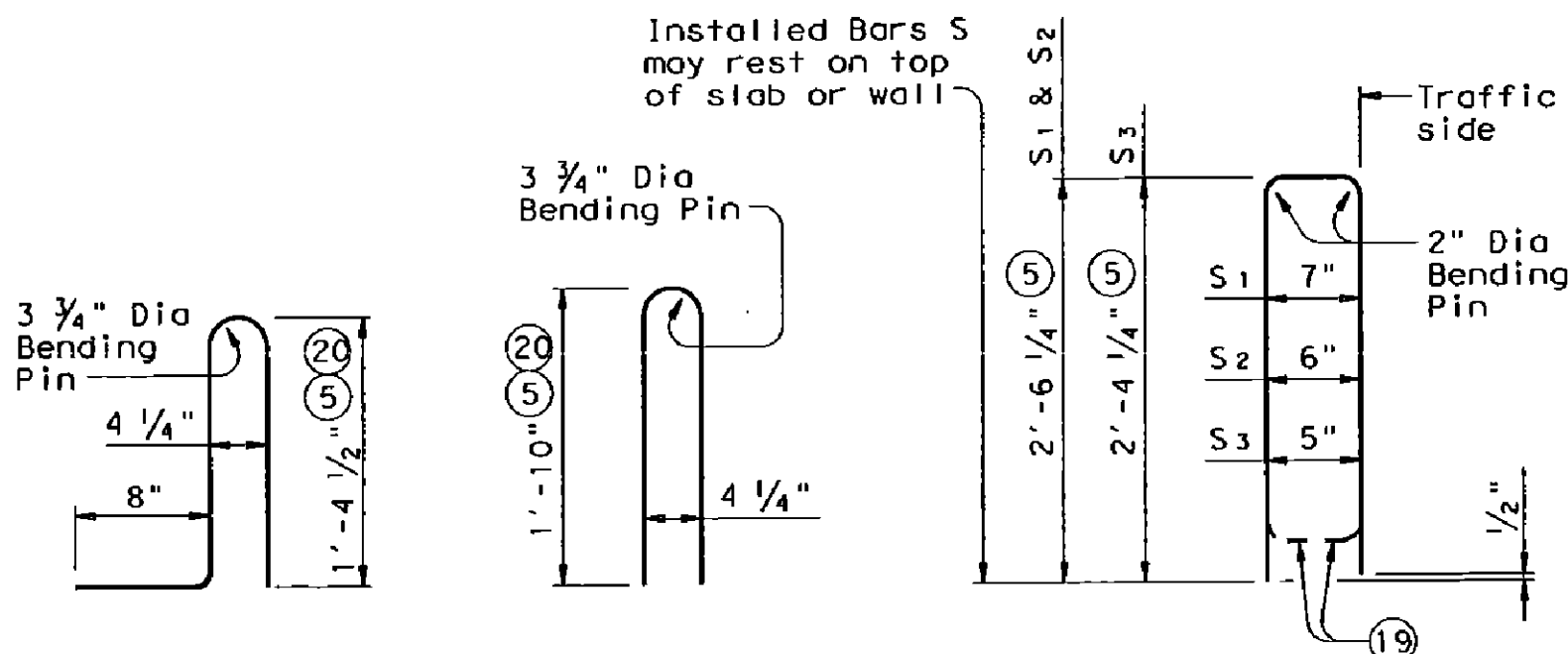
AT SPLICE OR EXP JTS



SECTION A-A

The difference between the outside dimension of sleeve and inside dimension of pipe rail must not exceed 0.167" before galvanizing. Minimum wall thickness of sleeve is 0.120".

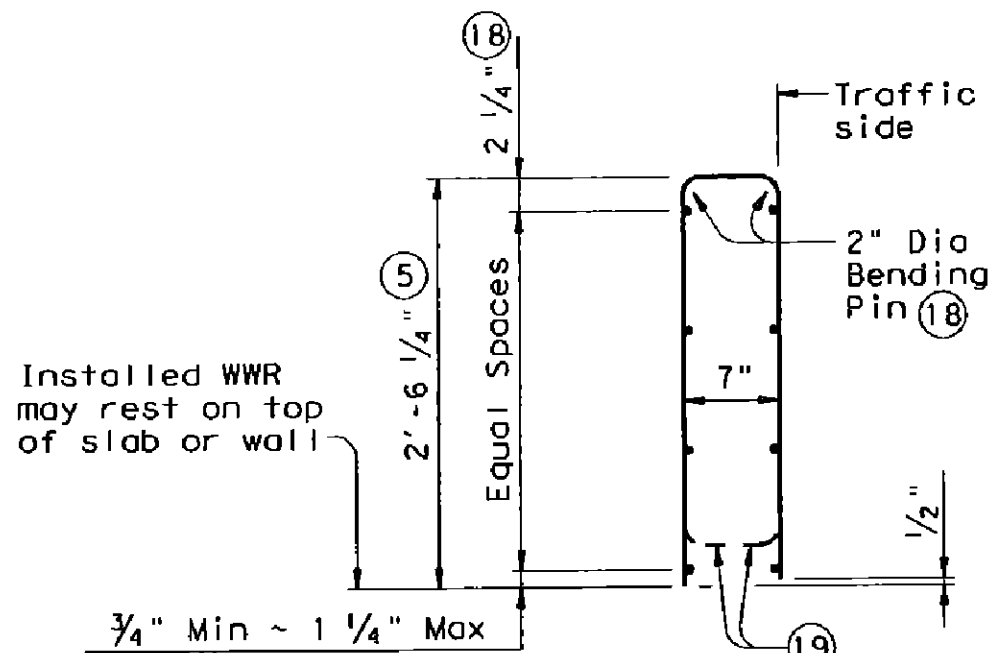
PIPE SPLICE DETAILS



BARS U (#4)

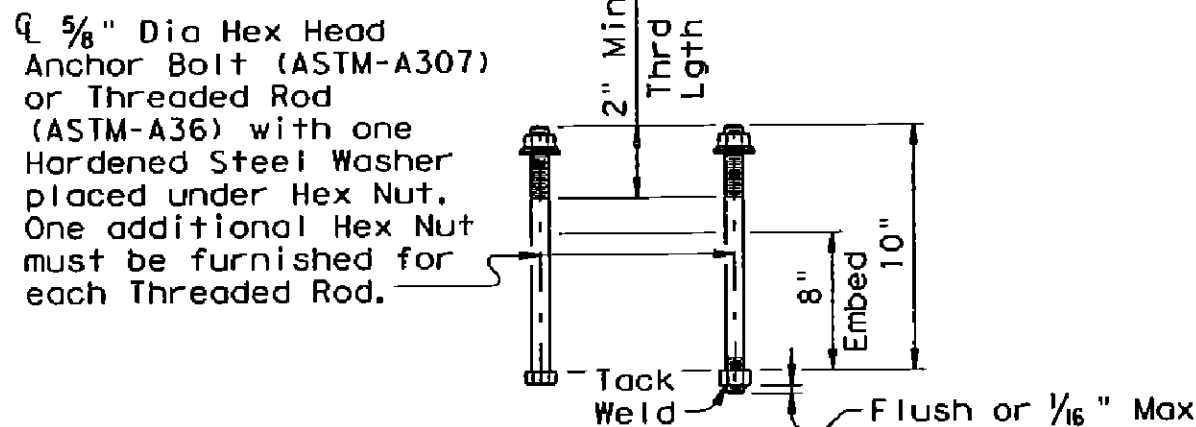
BARS WU (#4)

BARS S (#4)



OPTIONAL WELDED WIRE REINFORCEMENT (WWR)

DESCRIPTION	LONGITUDINAL WIRES	VERTICAL WIRES
Minimum (Cumulative Total) Wire Area	1.067 Sq In.	0.267 Sq In. per Ft
Minimum	No. of Wires	Spacing
Maximum	8	4"
	10	8"
Maximum Wire Size Differential	The smaller wire must have an area of 40% or more of the larger wire.	



CAST-IN-PLACE ANCHOR BOLT OPTIONS

- (5) Increase 2" for structures with overlay.
- (9) 2 1/2" Std Pipe (2.875" O.D. 0.203" wall thickness)
- (17) Shop drawings for approval required for tubular steel sections.
- (18) No longitudinal wires may be in top center of cage.
- (19) Bend or cut as required to clear drain slots.
- (20) For raised sidewalks, add sidewalk height to total bar height. Use sidewalk height at rail's location.
- (21) See "Material Notes" for anchor bolt information.

CONSTRUCTION NOTES:

This railing may be constructed with slip-forms when approved by the Engineer, with equipment approved by the Engineer and when epoxy adhesive anchor bolts are used. Slip-forming parapet is not allowed if anchor bolts are cast with parapet wall. Sensor control for both line and grade must be provided. Tack welding to provide bracing for slip-form operations is acceptable. Welding can be performed at a minimum spacing of 3 ft between the cage and the anchorage. It is permissible to weld to U, WU and S bars at any location on the cage. If increased bracing is needed, additional anchorage devices must be added and welding must be performed in the upper two thirds of the cage.

Face of rail, parapet must be plumb unless otherwise approved by the Engineer. Pipe rail posts must be square to the top of parapet. Use epoxy mortar under post base plates if gaps larger than 1/16" exist.

Exposed edges of pipe rail and pipe rail posts must be rounded or chamfered to approximately 1/16" by grinding.

At the contractor's option anchor bolts may be cast with the parapet (See Cast-in-Place Anchor Bolt Options).

Pipe rail sections must not include less than two posts, and no more than four (except at Abutment).

Chamfer all parapet exposed corners.

MATERIAL NOTES:

All steel components except reinforcing must be galvanized unless otherwise shown on plans.

Use Class "C" concrete. Use Class "C" (HPC) if required elsewhere.

All reinforcing must be Grade 60.

Epoxy coat all rail reinforcement if slab bars are epoxy coated.

Deformed welded wire reinforcement (WWR) may be used as an option to conventional reinforcement and must be made in accordance with ASTM A497 (Deformed Wire). Combinations of Reinforcing Steel and WWR or configurations of WWR other than shown will be permitted when the conditions in the table are satisfied.

Pipe for pipe rail must conform to ASTM A53 Grade B or A500 Grade B.

Anchor bolts must be 5/8" Dia ASTM A36 fully threaded rods with one hex nut and one hardened steel washer at each bolt. Embed threaded rods into parapet wall with a Type III Class C epoxy anchorage system. Minimum embedment depth is 3".

Anchorage system chosen must be able to achieve an ultimate tensile resistance of 8.4 kips per bolt. The Contractor must provide evidence to the Engineer that this can be achieved. Evidence of adequate tensile resistance can be based on the manufacturer's published values of ultimate tensile strength (anchor spacing and edge distance must be accounted for).

Anchor installation, including hole size, drilling, and clean-out, must be in accordance with the manufacturer's instructions.

Optional cast-in-place anchor bolts must be 5/8" Dia ASTM A307 Grade A bolts (or A36 threaded rods with one tack welded hex nut each) with one hex nut and one hardened steel washer at each bolt.

GENERAL NOTES:

This rail, without the pipe rail, has been evaluated and accepted to be of equal strength to railings with like geometry, which have been crash tested to meet NCHRP Report 350 TL-4 criteria. However, its use is limited to design speeds of 45 mph or less due to the presence of the pipe rail.

This railing cannot be used on bridges with expansion joints providing more than 5" movement.

Rail anchorage details shown on this standard may require modification for select structure types. See appropriate details elsewhere in plans for these modifications.

Erection drawings showing panel lengths, rail post spacing, and anchor bolt setting must be submitted to the Engineer for approval.

Average weight of railing with no overlay: 370 plf (Conc), 10 plf (Steel)

RECORD DRAWING
02/02/2015

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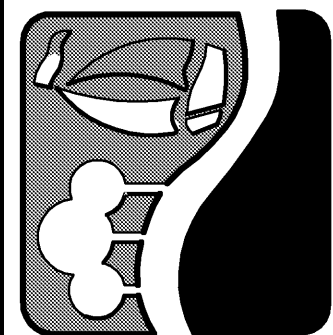
SHEET 3 OF 3

Texas Department of Transportation
Bridge Division

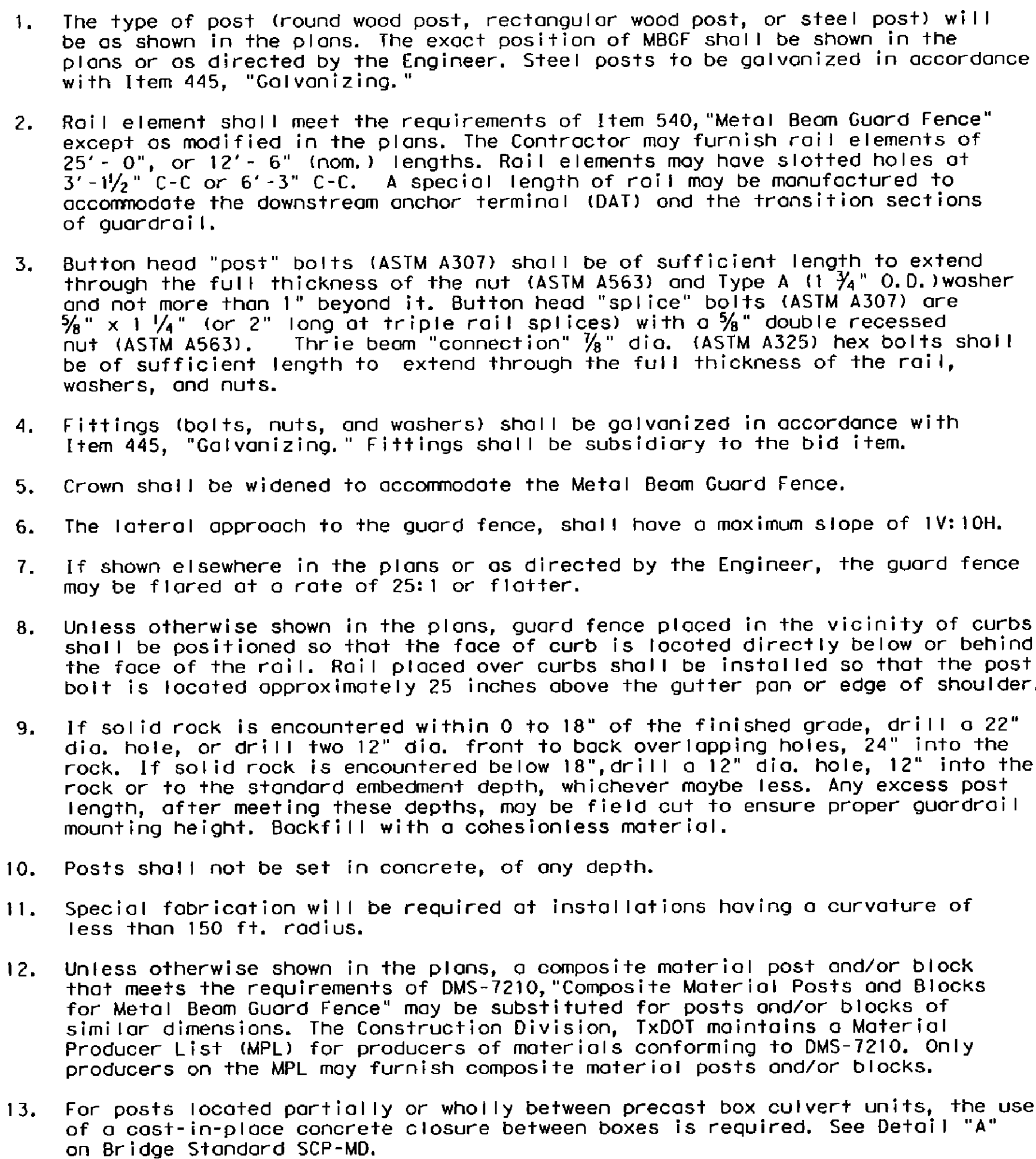
COMBINATION RAIL

TYPE C221

FILE: r1std018.dgn	DN: TXDOT	CK: TXDOT	DN: JTR	CK: JMH
TXDOT April 2009	DISTRICT	FEDERAL AID PROJECT	SHEET	
REVISIONS				
05-111 Wall Joint Note, 07-124 Guardrail Transition.				
COUNTY	CONTROL SECT	JOB	HIGHWAY	



TIME 13.11



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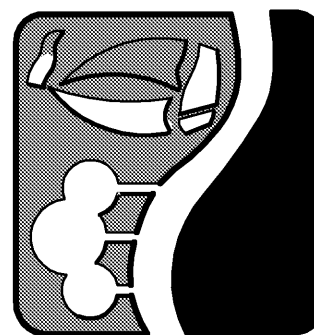
**Design
Division
Standard**

METAL BEAM GUARD FENCE

GF (31) - 11

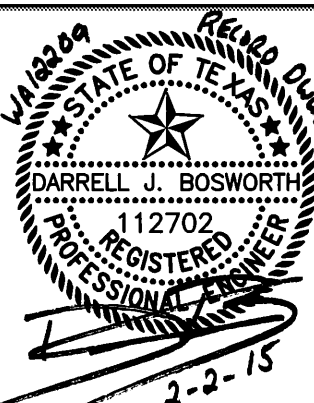
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(C) TxDOT December 2011	CONT	SECT	JOB		HIGHWAY
REVISIONS					
	DIST	COUNTY			SHEET NO.

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ROCKWALL TECHNOLOGY PARK PHASE IV

**TXDOT METAL
BEAM GUARD RAIL
GF (31) - 11**

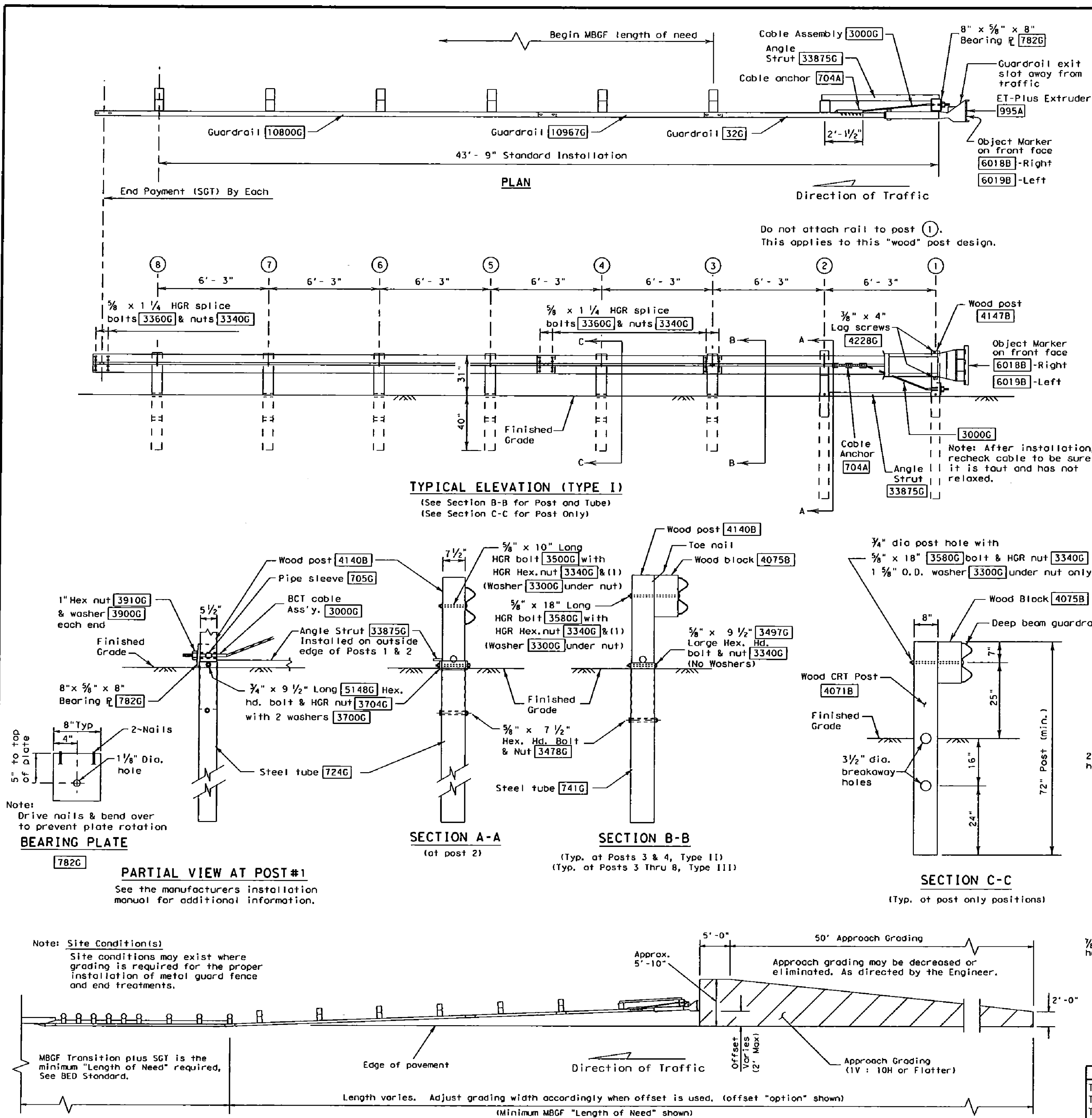


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LAST SHEET EDIT
DATE 09-30-2013
WA# 12209

SHEET NO
P304

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DATE: FILE:



GENERAL NOTES

- For additional information contact: Trinity Highway Products, 1-800-527-6050.
- The Type of SGT unit will be specified elsewhere in the plans. Numbers in circles indicate post position. The Type of SGT unit chosen is a maintenance consideration and does not affect the systems performance.

Post & Tube Options		Post Only	
Type I	Posts ① thru ②	Posts ③ thru ④	
Type II	Posts ① thru ④	Posts ⑤ thru ⑥	
Type III	Posts ① thru ⑥		
- SGT's placed within the "minimum" 150 ft. radius, shall be installed straight. Standard rail elements may be installed within the radius, without special fabrication.
- All bolts, nuts, cable assemblies, cable anchors, steel tubes & bearing plates shall be galvanized.
- A flare rate of 25:1 may be used to prevent the terminal head from encroaching on the shoulder. The flare may be decreased or eliminated for specific installations, if directed by the Engineer.
- The steel tubes shall not protrude more than 4 inches above ground. Site grading may be necessary to meet this requirement.
- The steel tubes may be driven with an approved driving head. They shall not be driven with the wood post in the tube. If the steel tubes are placed in drilled holes, the backfill material must be satisfactorily compacted to prevent tube settlement.
- If solid rock is encountered. See the manufacturer's installation manual for the proper installation guidance.
- The breakaway cable assembly must be taut. A locking device, (vice grips or channel lock pliers) should be used to prevent the cable from twisting when tightening the nuts.
- The wood blocks shall be "toe nailed" to the rectangular wood posts to prevent them from turning when the wood shrinks.
- For curb installations, the soil tubes and posts shall be installed at the proper ground elevation behind the curb. The posts will then require field drilling new holes to accommodate the rail to post connection bolt to maintain the proper height of the rail above the gutter pan. The excess post length above the rail will be removed as directed by the Engineer.
- An object marker shall be installed on the front of the impact head as detailed on D&M(VIA).
- A special site evaluation should be considered, prior to using this end treatment where there is less than 25 feet between the extrusion side of the end treatment and any adjacent driving lane.

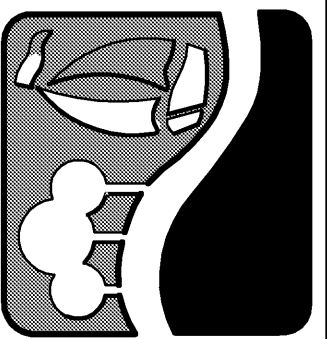
Code #	POST & TUBE OPTIONS			DESCRIPTION
	Type I	Type II	Type III	
32G	1	1	1	Guardrail (12 Ga) at 12'-6" (ANC)
10967G	1	1	1	Guardrail (12 Ga) at 9'-4 1/2"
10800G	1	1	1	Guardrail (12 Ga) at 25'-0"
724G	2	2	2	Steel Tube - 6"x 8"x 72"x 1/8" min
741G	0	2	6	Steel Tube - 6"x 8"x 54"x 1/8" min
4140B	2	4	8	Wood Posts - 5 1/2" x 7 1/2" x 48 1/4"
4071B	6	4	0	Wood CRT Posts - 6"x 8"x 72"
4075B	6	6	6	Wood Block - 6"x 8"x 14"
705G	1	1	1	Pipe Sleeve - 2" std. pipe x 5 1/2"
782G	1	1	1	Bearing Plate - 8"x 8"x 3/8"
704A	1	1	1	Cable Anchor Bracket
3000G	1	1	1	Cable Assembly (3/4" x 78")
33875G	1	1	1	Angle Strut
995A	1	1	1	ET-Plus Extruder
5148G	2	2	2	3/4" x 9 1/2" Hex Hd (Top of tubes 1&2)A325
3300G	7	7	7	3/4" Washers
3478G	2	4	8	3/4" x 7 1/2" Hex Bolt
3500G	1	1	1	3/4" x 10" Post Bolt (Post ②)
3580G	6	6	6	3/4" x 18" Post Bolt (Posts ③ thru ⑧)
3360G	24	24	24	3/4" x 1 1/4" Splice Bolt
3340G	33	37	45	3/4" Hex Nut
4228G	2	2	2	3/4" x 4" Lag Screw
3910G	2	2	2	1" Hex Nut
3900G	2	2	2	1" Washer
6018B	1	1	1	Right - Object Marker
6019B	1	1	1	Left - Object Marker
3700G	4	4	4	3/4" Washer
3704G	2	2	2	3/4" Heavy Hex Nut
3497G	0	2	6	3/4" x 9 1/2" Hex Hd (Top of Tubes 3-8)A307

POST & TUBE OPTIONS			
Type I	Posts ① thru ②		
Type II	Posts ① thru ④		
Type III	Posts ① thru ⑥		

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Texas Department of Transportation		Design Division Standard	
SINGLE GUARDRAIL TERMINAL (ET-31) (WOOD POST) SGT(7)31-11			
FILE: sgt73111.dgn	DN: TXDOT	CK: AM	DR: BD
① TXDOT December 2011	CON: SECT	JOB	HIGHWAY
REVISIONS	DIST	COUNTY	SHEET NO.



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**TXDOT
SINGLE GUARDRAIL
TERMINAL
SGT (7) 31 - 11**

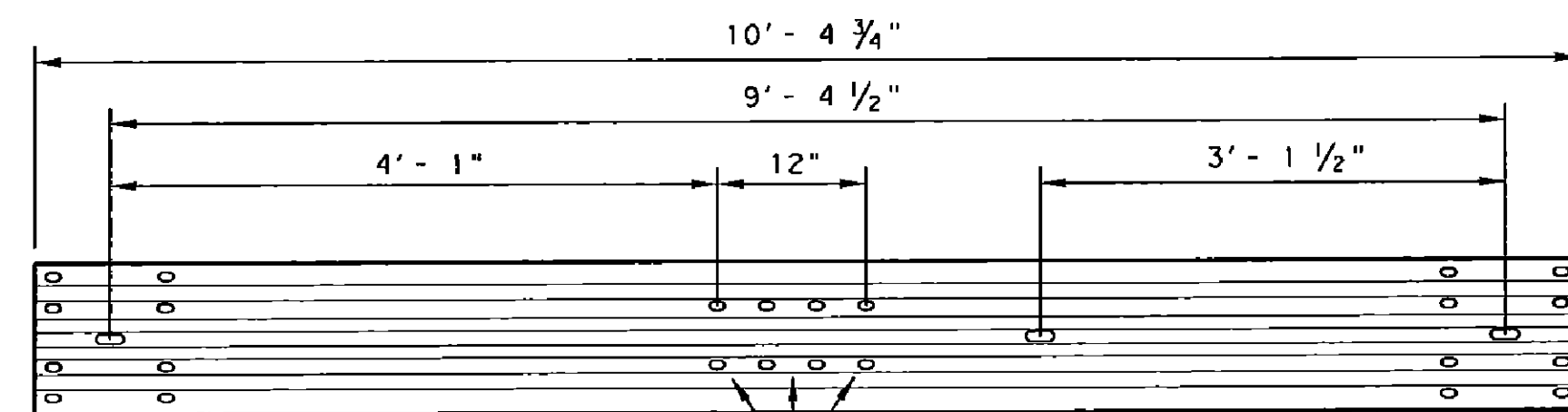


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TIME 13:14

DATE: _____
FILE: _____



Technical drawing of a bracket assembly showing three views: front, side, and end view.

Front View: Shows a bracket with a total height of 16". The top flange is 3" wide. The vertical section has a central dashed line. Dimensions from top to bottom: 2", 4", 4", 4", 4", 2".

Side View: Shows the bracket's profile. The top flange is 3" wide. The vertical section has a central dashed line. Dimensions from top to bottom: 2", 4", 4", 4", 4", 2".

End View: Shows the bracket's end. The top flange is 3" wide. The vertical section has a central dashed line. Dimensions from top to bottom: 2", 4", 4", 4", 4", 2".

Labels and Dimensions:

- Weld End Plate to Bracket
- 3"
- 1"
- 1/4"
- 5 1/2"
- Three Sides
- Bent Plate 16" x 12 1/2" x 3/16"
- 3"
- 1 1/2"
- 1" Dia. Hole
- 2 3/4"
- 1 3/8"
- 3/8"
- End Plate
- 3/4" Dia. Holes
- Bracket
- End Plate
- 3/8"
- 1 5/8"
- 2 3/4"
- 35°
- 2 1/4"
- 1 3/4"
- 2"
- 3/16"
- 2.5" (TYP)
- 3 3/8"
- 2 3/8"

Technical drawing of a splice bolt slot. The top view shows a 30° angle, a 3" MIN dimension, and a 6 1/4" R radius. The side view shows a 12" height and a 7 1/2" width. The end view shows a 2" width and an 8" length. The drawing is labeled "Splice Bolt Slot (TYP) 1" x 1 1/8\"

Diagram of a square plate with the following dimensions and features:

- Overall width: 8" (TYP)
- Overall height: 8"
- Distance from top edge to centerline: 5" to top of plate
- Distance from centerline to edge: 4"
- Plate thickness: 1 1/8" Dia
- Assembly instruction: 2-Nails
- Note: Drive nails and bend over to prevent plate rotation

Technical drawing of a rectangular box. The overall width is 6" and the overall height is 7 1/2". The top flange has a width of 3". The side flange has a height of 1 3/8". The bottom flange has a height of 1 1/2". The main body of the box has a width of 1 1/4" and a height of 3/4" x 1". The drawing shows a top view and a side view. The top view shows a rectangular box with a central slot. The side view shows the box with a central slot. The label "Slots (TYP)" is present.

Technical drawing of a terminal post showing three views: Side View, Front View, and an Elevation View.

- Side View:** Shows a vertical post with a total height of 31 1/2". The top section has a width of 5 1/4". There are two 7/8" diameter holes, one near the top and one near the bottom, with a vertical distance of 7" between their centers.
- Front View:** Shows a vertical post with a total height of 46". The top section has a width of 7 1/4". There is a 2 1/2" diameter hole near the bottom. The distance from the top of the post to the center of this hole is 28 1/2".
- Elevation View:** Shows two vertical posts. The left post has a top width of 6" and a height of 17" to the center of a 7/8" diameter hole. The right post has a top width of 8" and a total height of 72".

Technical drawing of a C-channel with two gussets. The channel has a height of 3 inches and a flange width of 1 1/2 inches. The gussets are 2 1/2 inches wide and 3/4 inch x 2 inches. The distance between the gussets is 80 inches. The gussets are attached to the channel with bolts.

If a mow strip is required with the DAT installation the leave-out area around the steel foundation tubes and the two channel struts may be omitted. This will require a full pour at the foundation tubes.



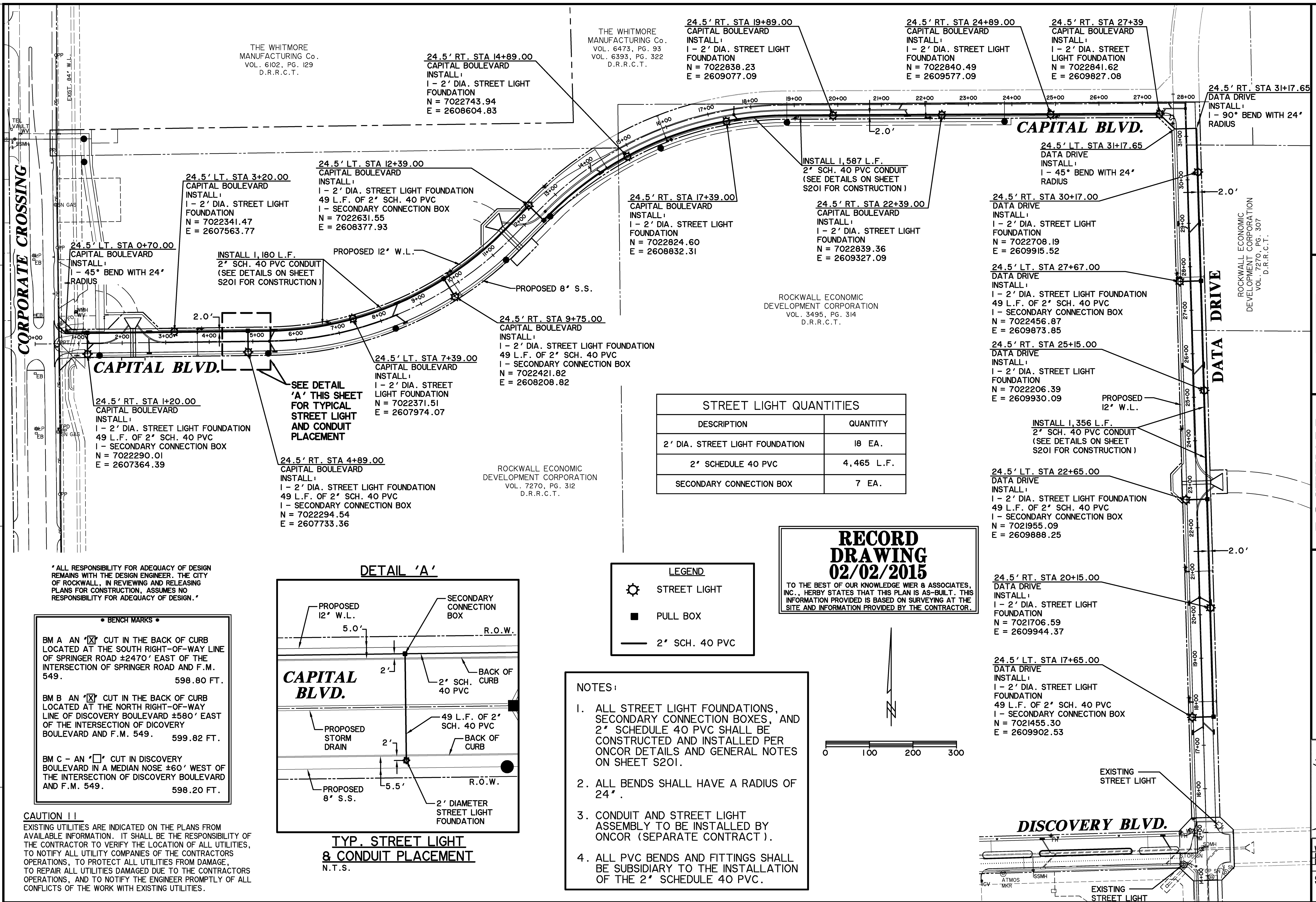
Texas Department of Transportation

Design Division
Standard

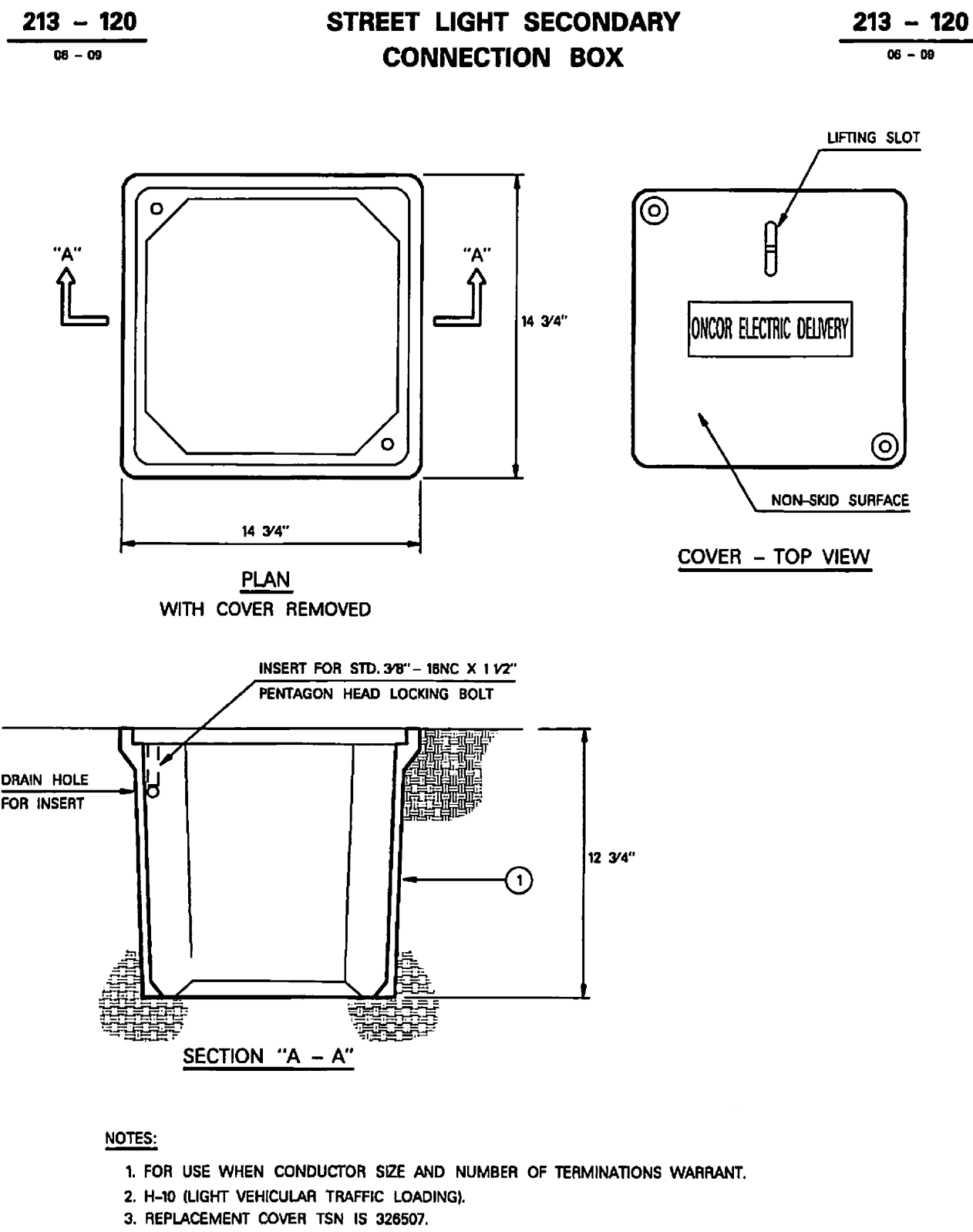
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ONCOR

208 - 030
01 - 05

CONDUIT BEND RADIUS AND MATERIAL

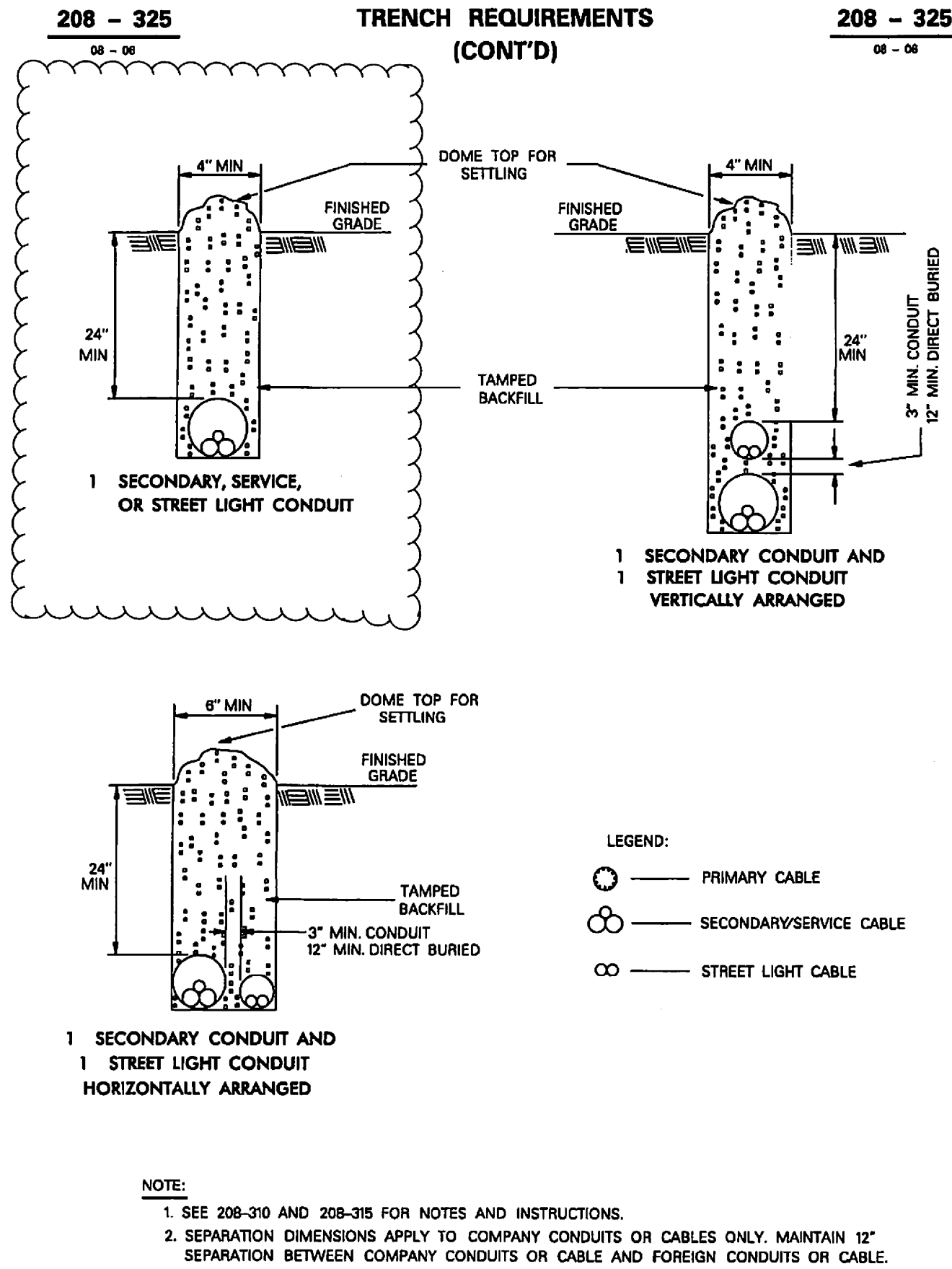
208 - 030
01 - 05

	CONDUIT NOMINAL SIZE (IN.)	MINIMUM BEND RADIUS (IN.)	TYPE OF BEND MATERIAL FOR PULLS:
1		18	PVC
2		24	PVC
3		24	PVC
4		24	PVC
6		36	PVC

ITEM	QTY	DESCRIPTION	TSN/REF	CU	MU
1	AS REQD	FITTING, CONDUIT, PVC, 1 INCH, BEND, SCH. 40, 45°			
2	AS REQD	FITTING, CONDUIT, PVC, 1 INCH, BEND, SCH. 40, 90°			
3	AS REQD	FITTING, CONDUIT, PVC, 2 INCH, BEND, SCH. 40, 45°	300338	CB4ED2P	
4	AS REQD	FITTING, CONDUIT, PVC, 2 INCH, BEND, SCH. 40, 90°	300343	CB1ER2P	
5	AS REQD	FITTING, CONDUIT, PVC, 3 INCH, BEND, SCH. 40, 45°	300338		
6	AS REQD	FITTING, CONDUIT, PVC, 3 INCH, BEND, SCH. 40, 90°	300342	CB1ER3P	
7	AS REQD	FITTING, CONDUIT, PVC, 4 INCH, BEND, SCH. 40, 45°			
8	AS REQD	FITTING, CONDUIT, PVC, 4 INCH, BEND, SCH. 40, 90°	287873	CB24R4P	
9	AS REQD	FITTING, CONDUIT, PVC, 6 INCH, BEND, SCH. 40, 45°	428788		
10	AS REQD	FITTING, CONDUIT, PVC, 6 INCH, BEND, SCH. 40, 90°	308171	CB36R6P	

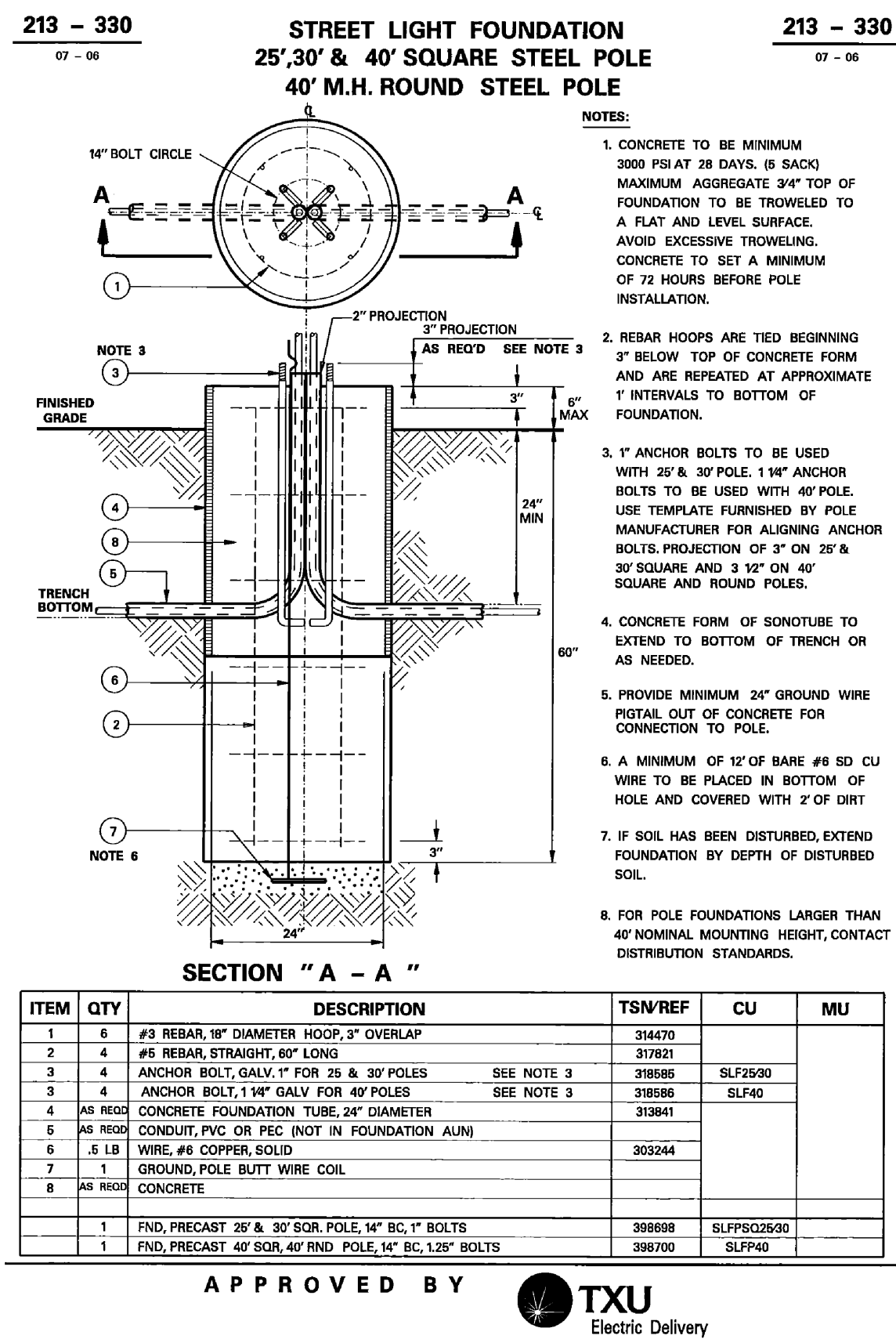
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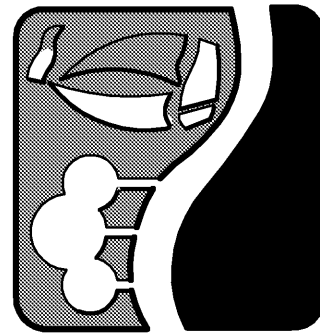
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02/02/2015

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GENERAL NOTES

1. TRENCH ALIGNMENT SHALL BE AS STRAIGHT AS CONDITIONS PERMIT. ANY DEVIATIONS FROM PLANNED ALIGNMENT SHALL HAVE PRIOR APPROVAL BY THE PROJECT ENGINEER/INSPECTOR. ALL TRENCH CUTS SHALL BE IN ACCORDANCE WITH EXISTING SAFETY REGULATIONS IN EFFECT.
2. TRENCH BOTTOM SHOULD BE UNDISTURBED, TAMPED, OR RELATIVELY SMOOTH EARTH. WHERE EXCAVATION IS IN ROCK, THE CONDUIT SHOULD BE LAID ON A LAYER OF CLEAN BACKFILL.
3. ALL BACKFILL SHOULD BE FREE OF DEBRIS OR OTHER MATERIAL THAT MAY DAMAGE THE CONDUIT SYSTEM OR CAUSE SETTLING. THE MATERIAL SHOULD FILL THE VOIDS AROUND THE CONDUIT TO PREVENT HOT SPOTS & SETTLING.
4. BACKFILL SHOULD BE ADEQUATELY COMPACTED. BACKFILL NOT UNDER PAVEMENT SHOULD BE COMPACTED TO THE DENSITY OF THE SURROUNDING UNDISTURBED SOIL. BACKFILL UNDER PAVEMENT SHOULD BE COMPACTED TO NOT LESS THAN 95% OF THE DENSITY OF UNDISTURBED SOIL AS DETERMINED BY ASTM DESIGNATION D-698.
5. SEE 208-315 FOR JOINING PVC CONDUIT & 208-330 FOR JOINING HDPE CONDUIT.
6. A 2500# OR 6000# PULL TAPE SHALL BE LEFT IN EACH CONDUIT. CONDUIT SHALL BE PLUGGED AT BOTH ENDS.
7. ALL CONDUIT THAT IS CONCRETE ENCASED (DUCT BANK) OR IN A CASING BORE THAT IS GROUT FILLED SHALL BE CHECKED BY PULLING A MANDREL THROUGH THE ENTIRE LENGTH OF EACH RUN OF CONDUIT AT THE COMPLETION OF THE CIVIL INSTALLATION.
8. EACH RUN OF CUSTOMER INSTALLED CONDUIT OF ALL TYPES WILL REQUIRE THAT A MANDREL CHECK BE MADE BY PULLING THROUGH THE ENTIRE LENGTH OF ALL CONDUITS IN THE INSTALLATION FOR FINAL CIVIL INSPECTION. (PLEASE SEE THE REQUIREMENTS OF THE TXU ELECTRIC DELIVERY DDS SPECIFICATIONS).
9. IT WILL NOT BE REQUIRED TO MAKE A MANDREL INSPECTION ON ANY TYPE OF COMPANY INSTALLED DIRECT BURIED OR DIRECTIONAL BORE CONDUIT INSTALLATION.

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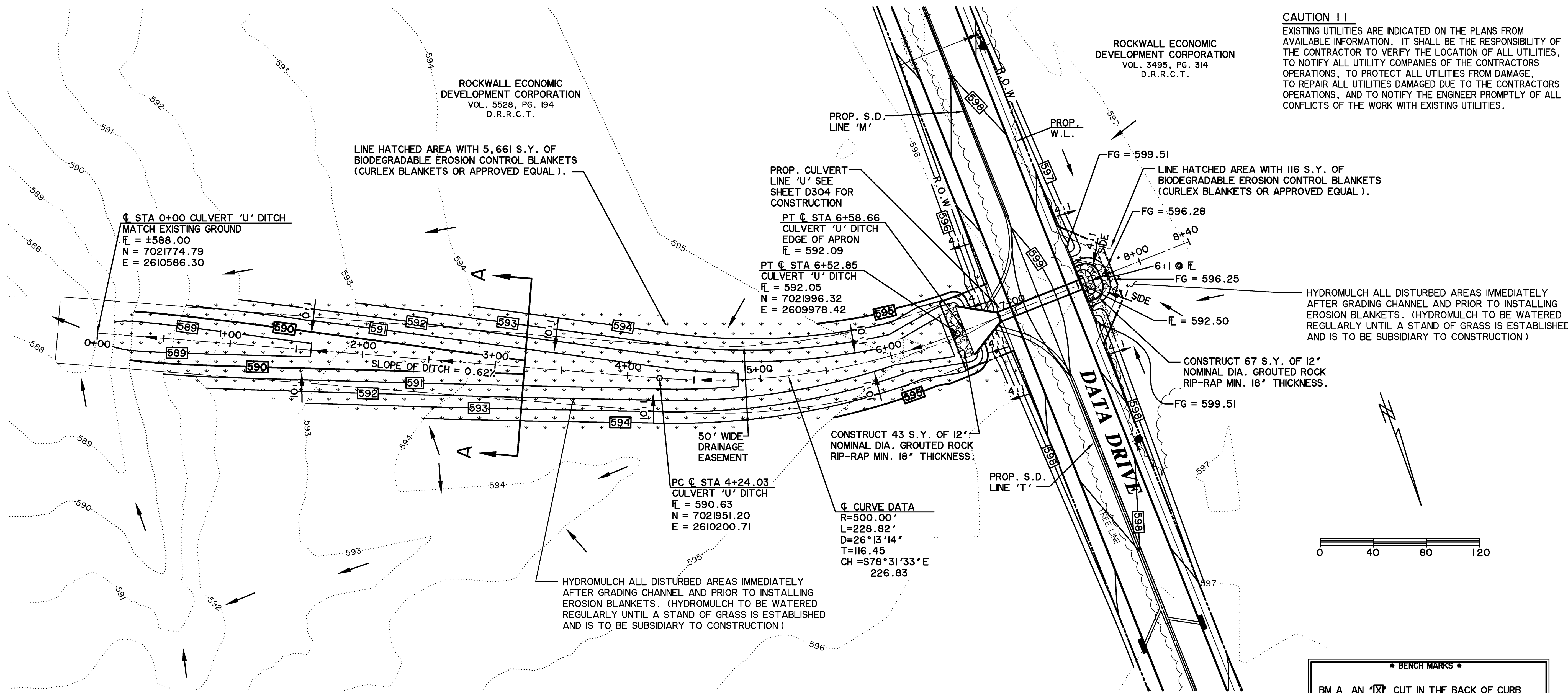


ROCKWALL TECHNOLOGY PARK
PHASE IV

STREET LIGHT DETAILS

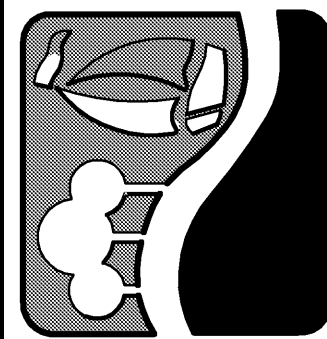


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SHEET NO. S201



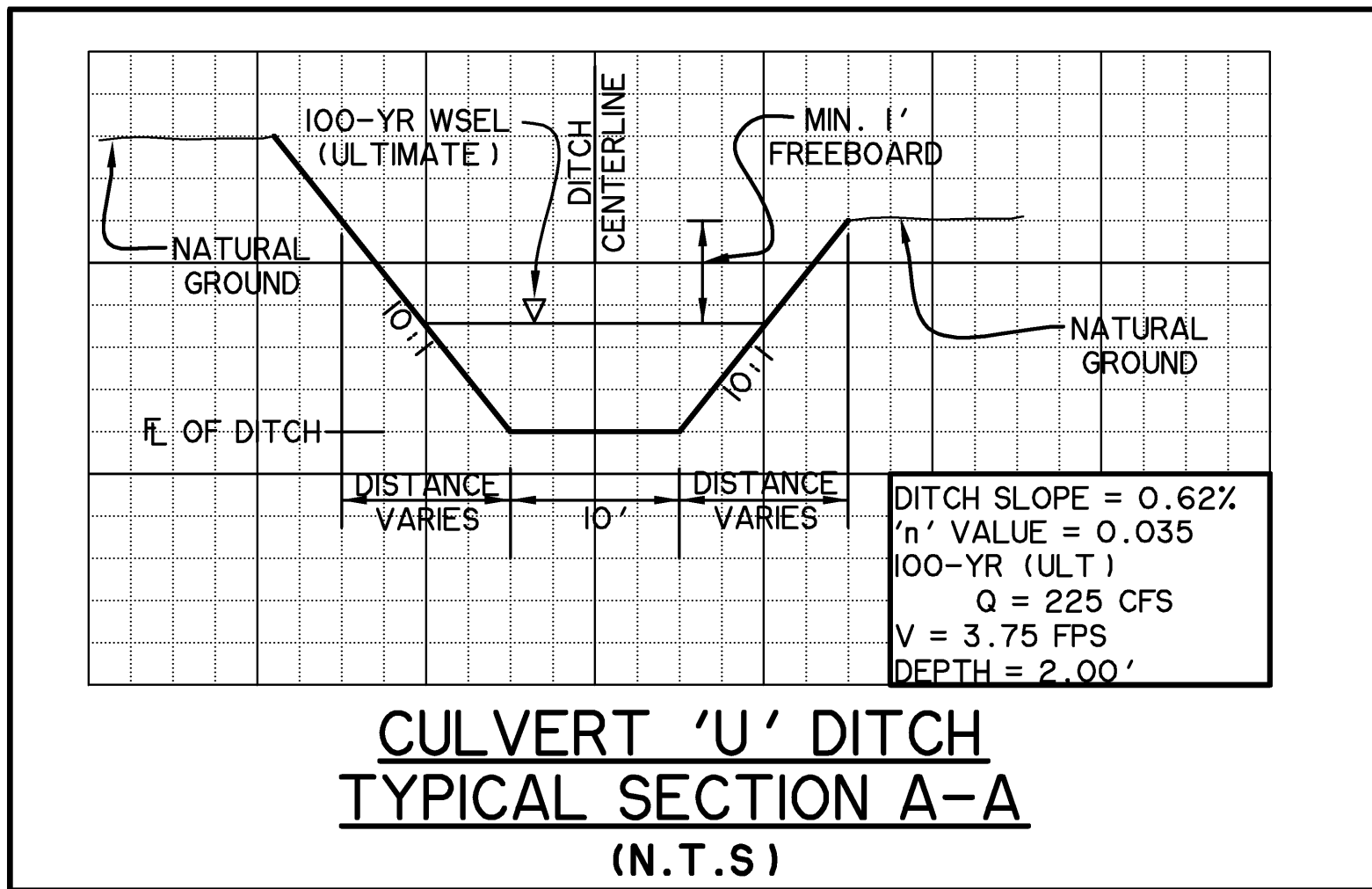
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
DITCHOUT GRADING
FOR CULVERT 'U'**



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*** BENCH MARKS ***

BM A - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.

BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.

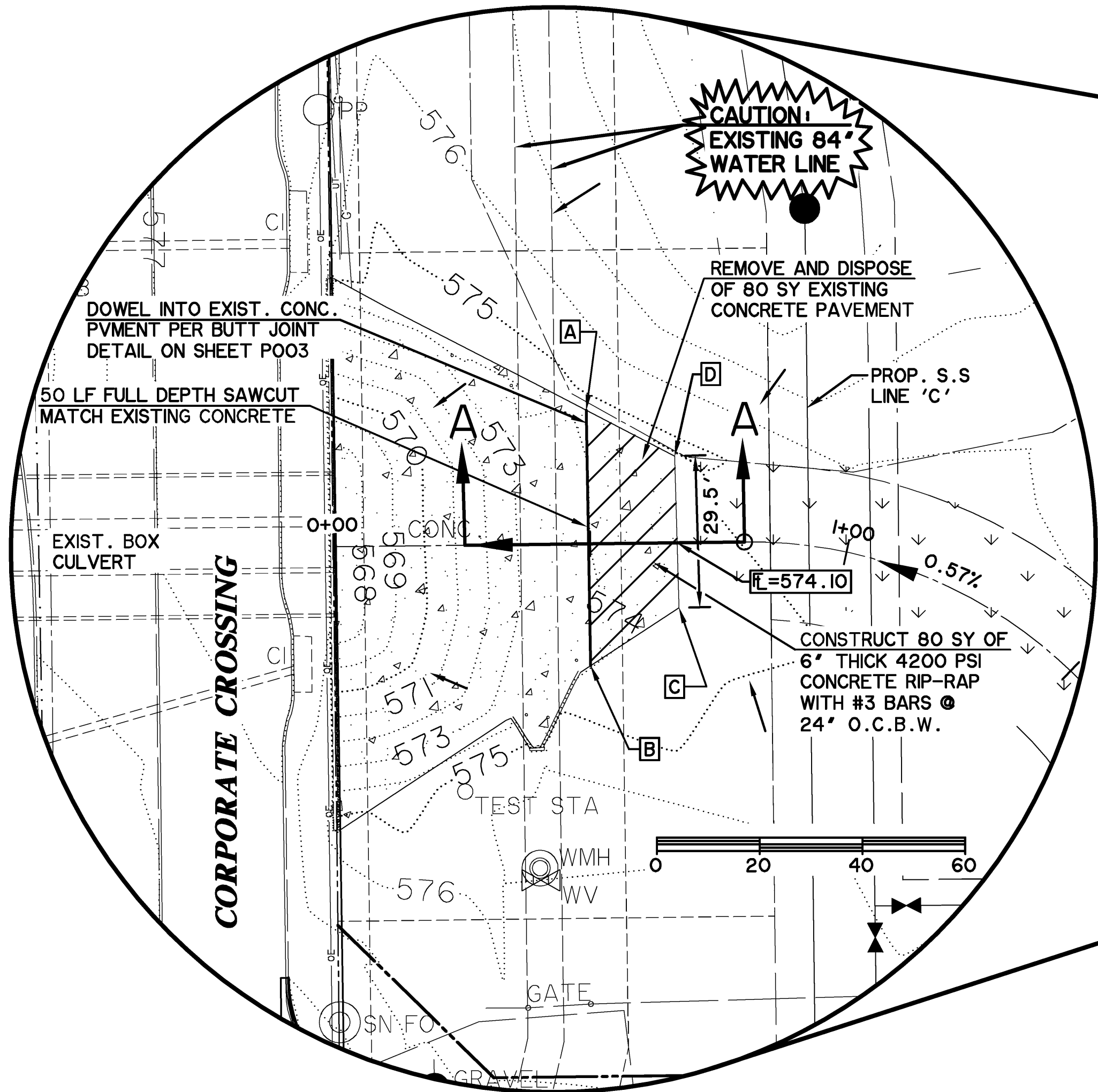
BM C - AN 'X' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

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02/02/2015**

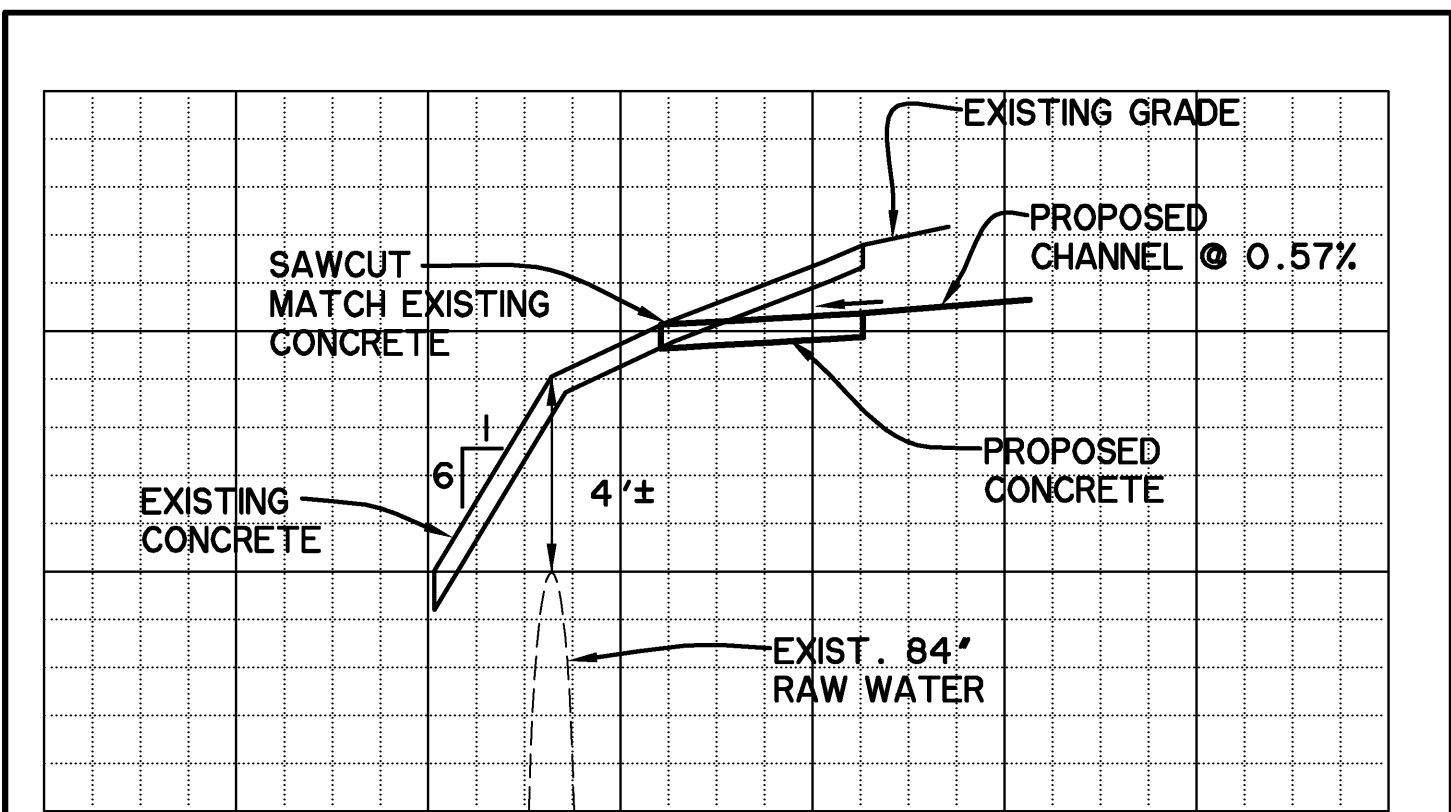
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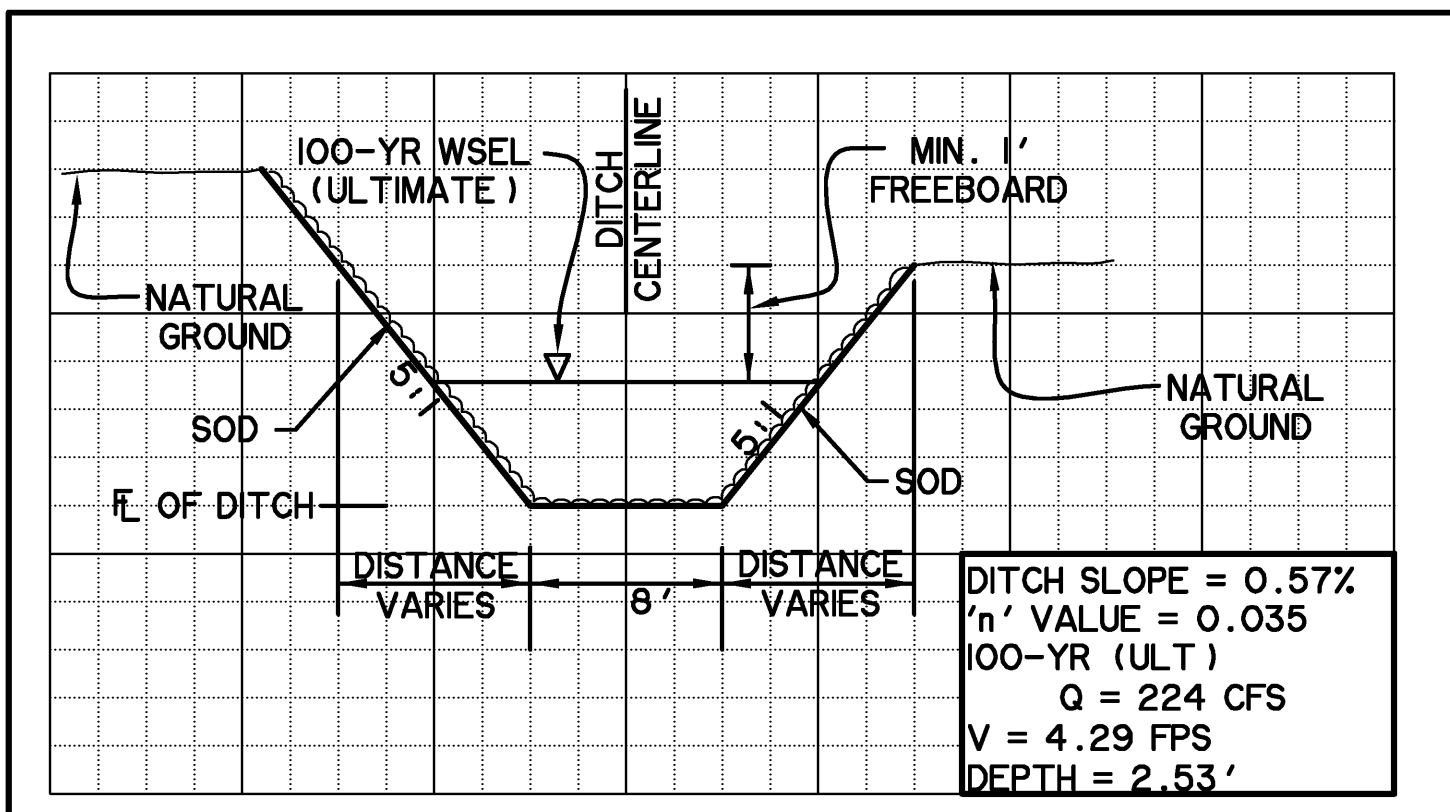
DITCHOUT FOR CULVERT 'V'



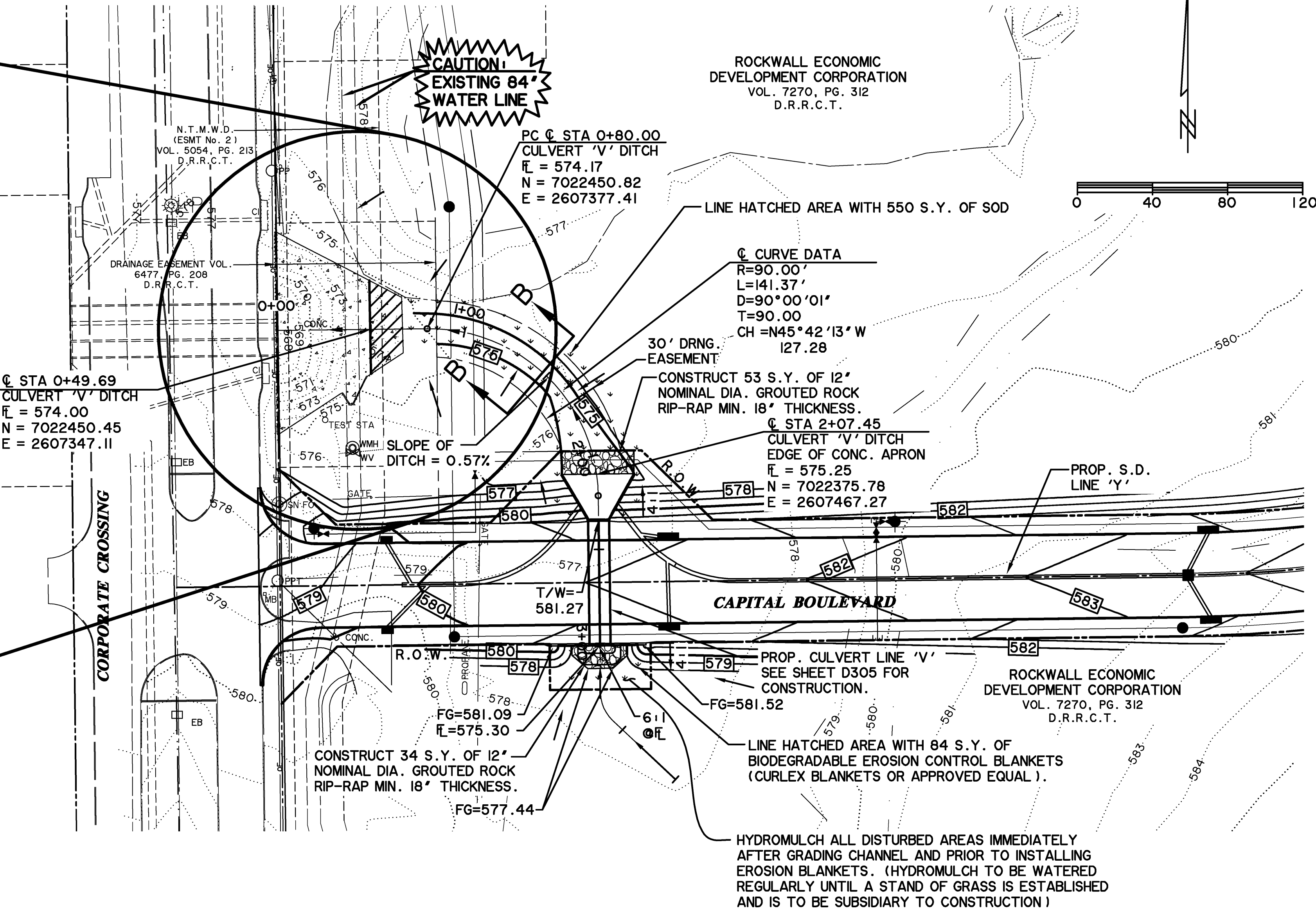
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B MATCH EXISTING CONCRETE TP = 574.00 N = 7022426.75 E = 2607347.50	D TOP OF CONCRETE TP = 574.10 N = 7022467.52 E = 2607363.82



CONCRETE APRON REPAIR
TYPICAL SECTION A-A
(N.T.S)



CULVERT 'V' DITCH
TYPICAL SECTION B-B
(N.T.S)

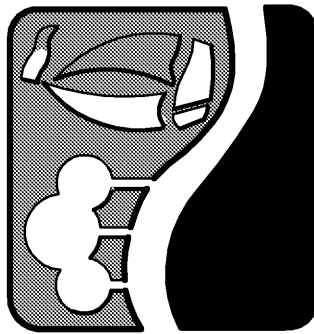


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BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	599.82 FT.
BM C - AN 'X' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	598.20 FT.

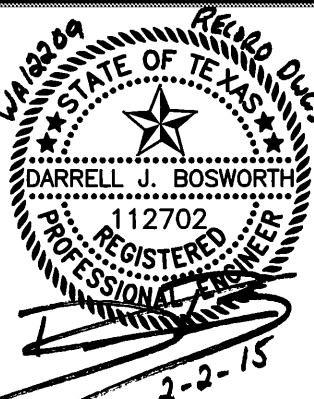
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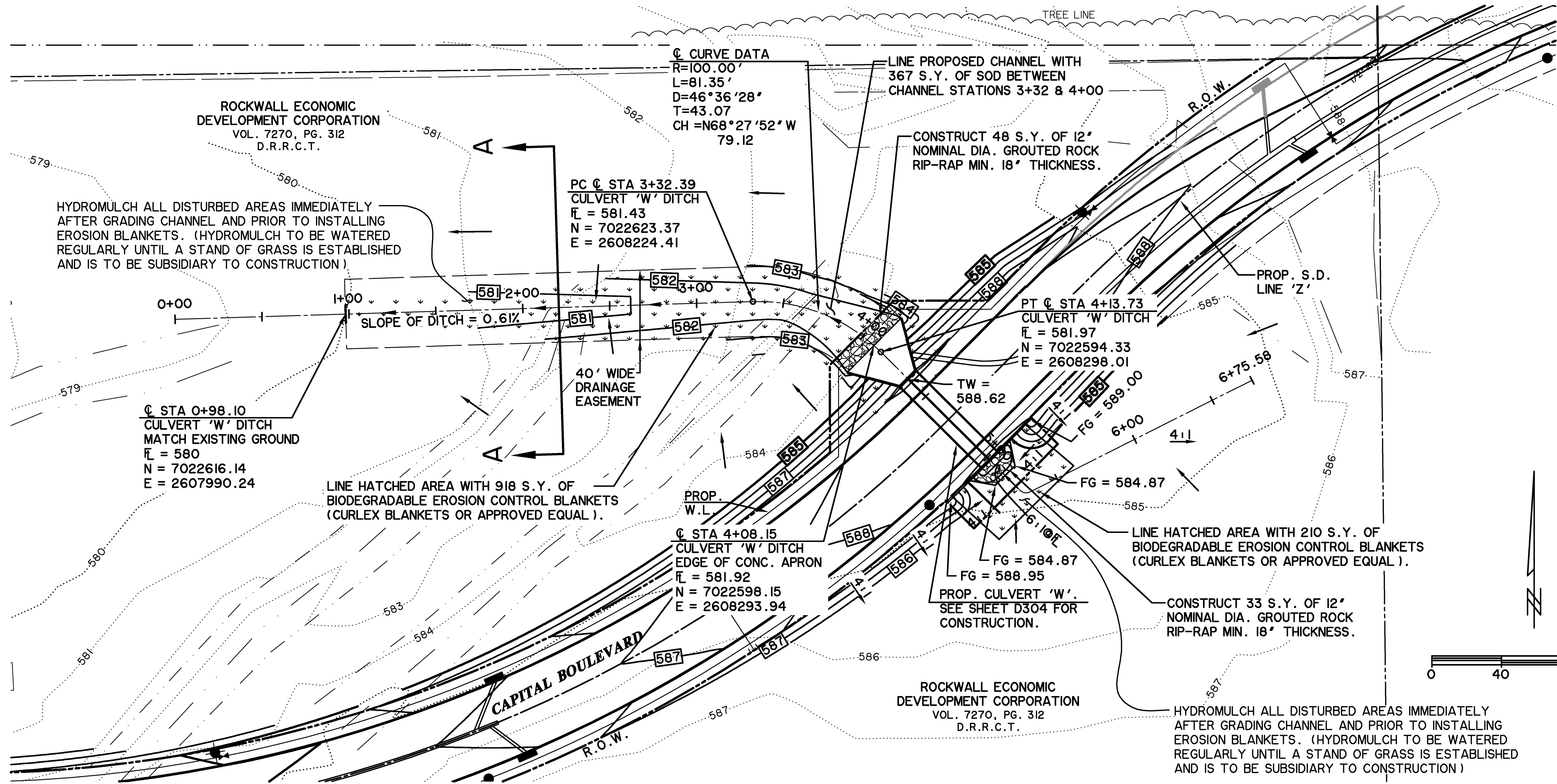
**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
DITCHOUT GRADING
CULVERT 'V'**

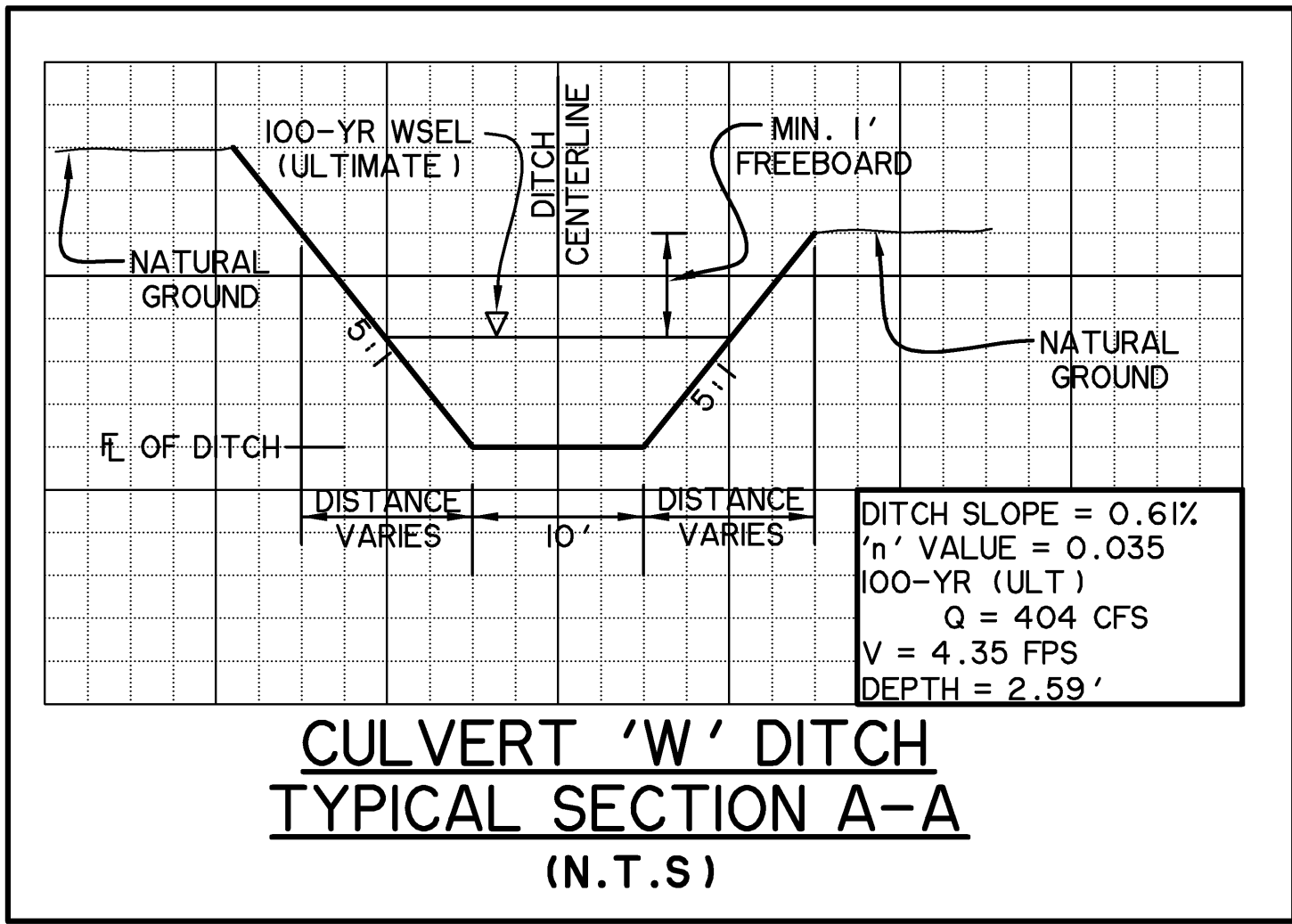


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DITCHOUT FOR CULVERT 'W'



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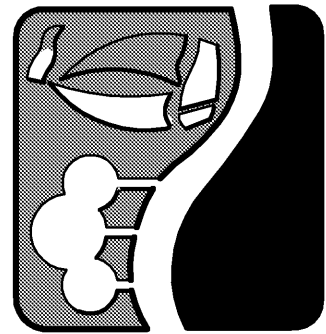


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BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	599.82 FT.
BM C - AN '□' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	598.20 FT.

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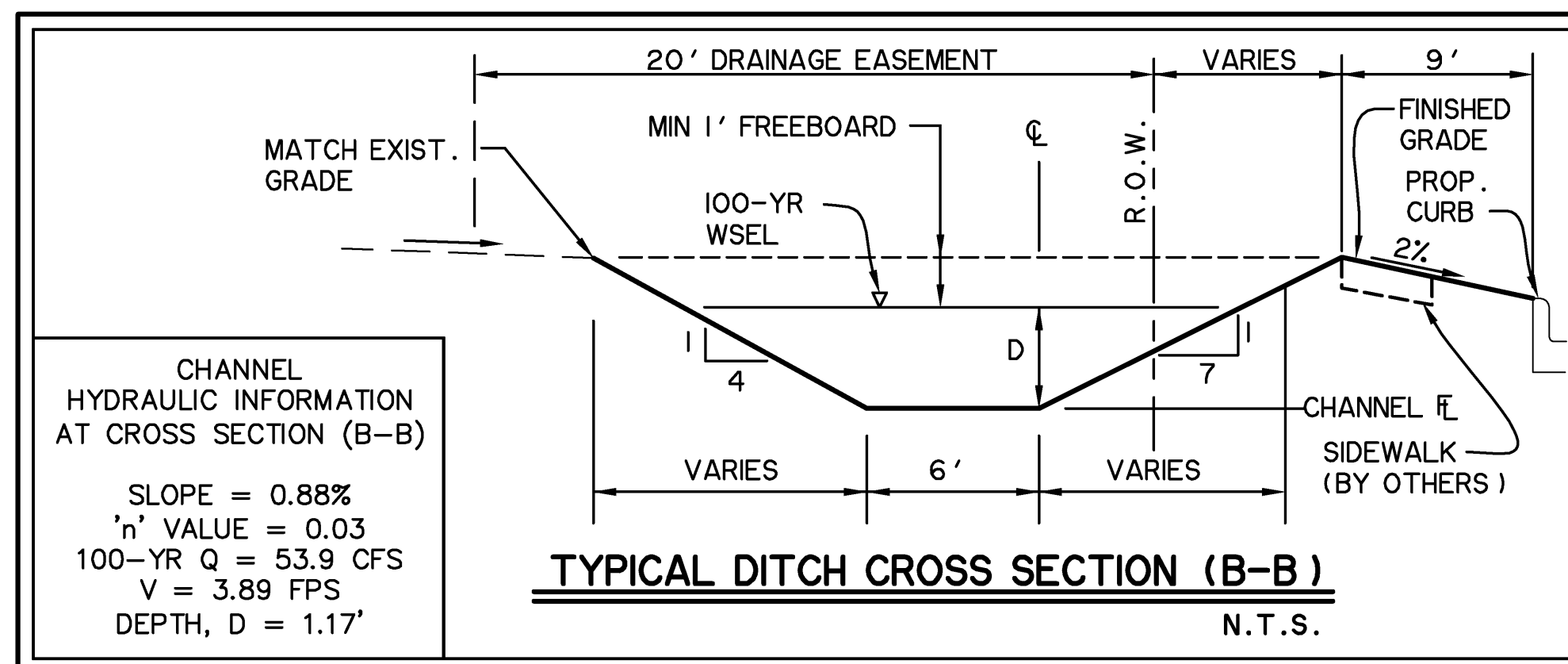
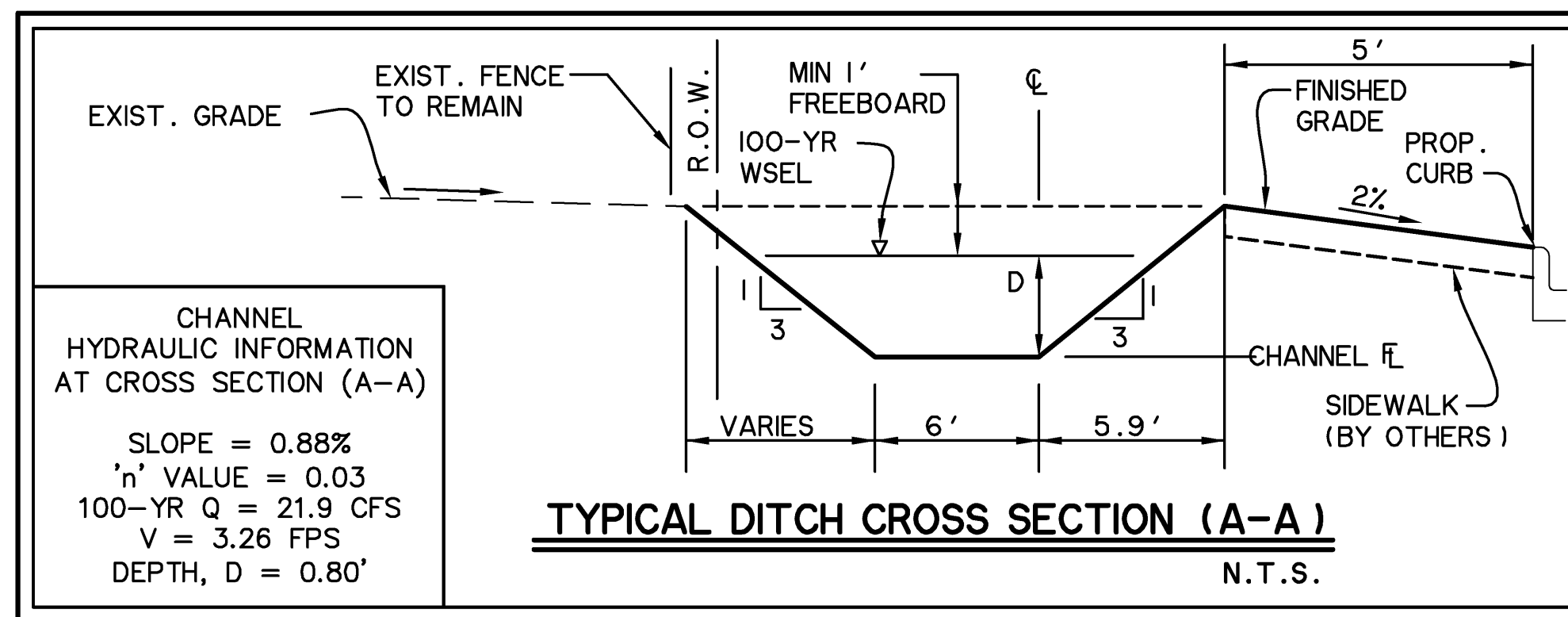
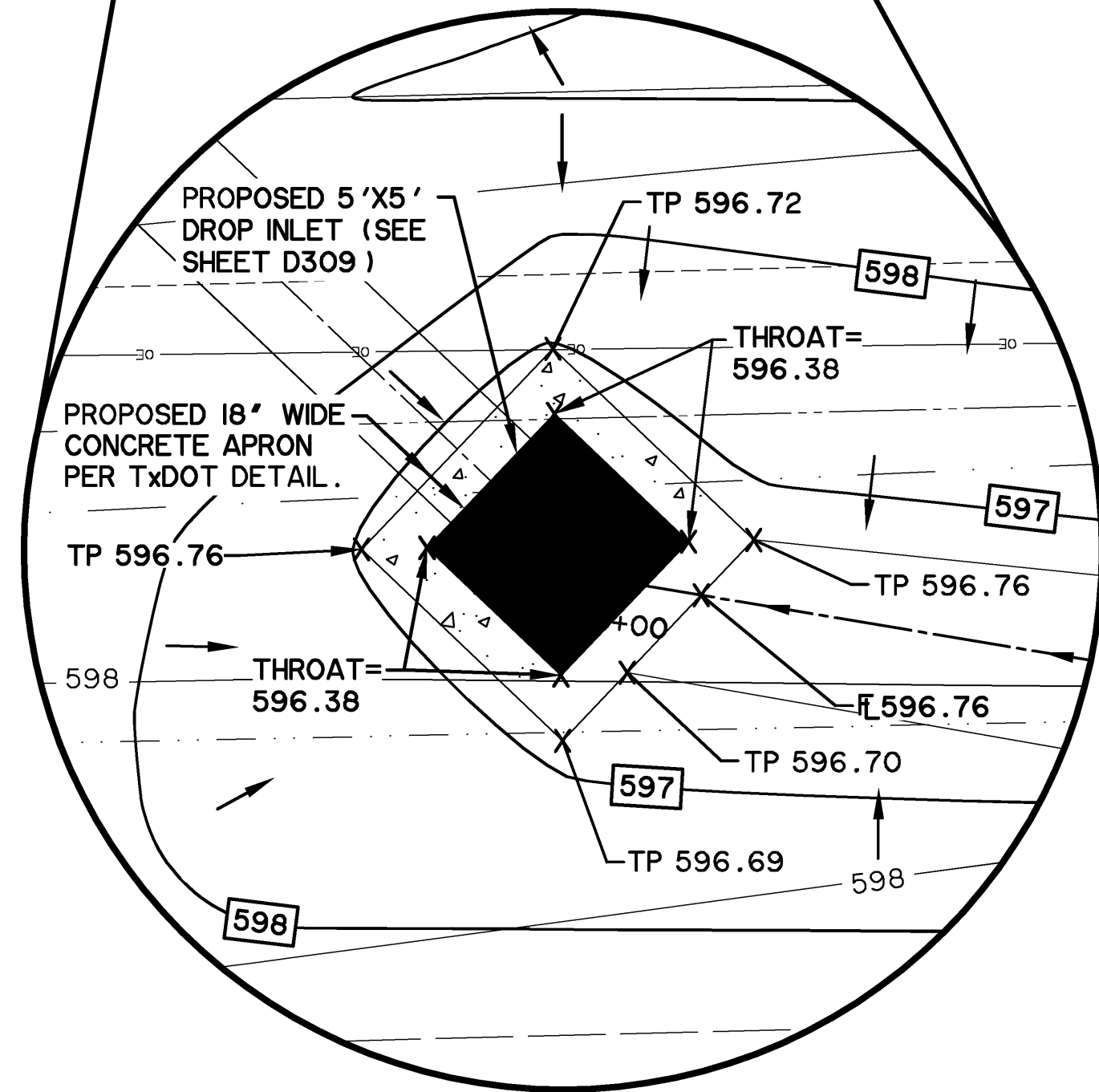
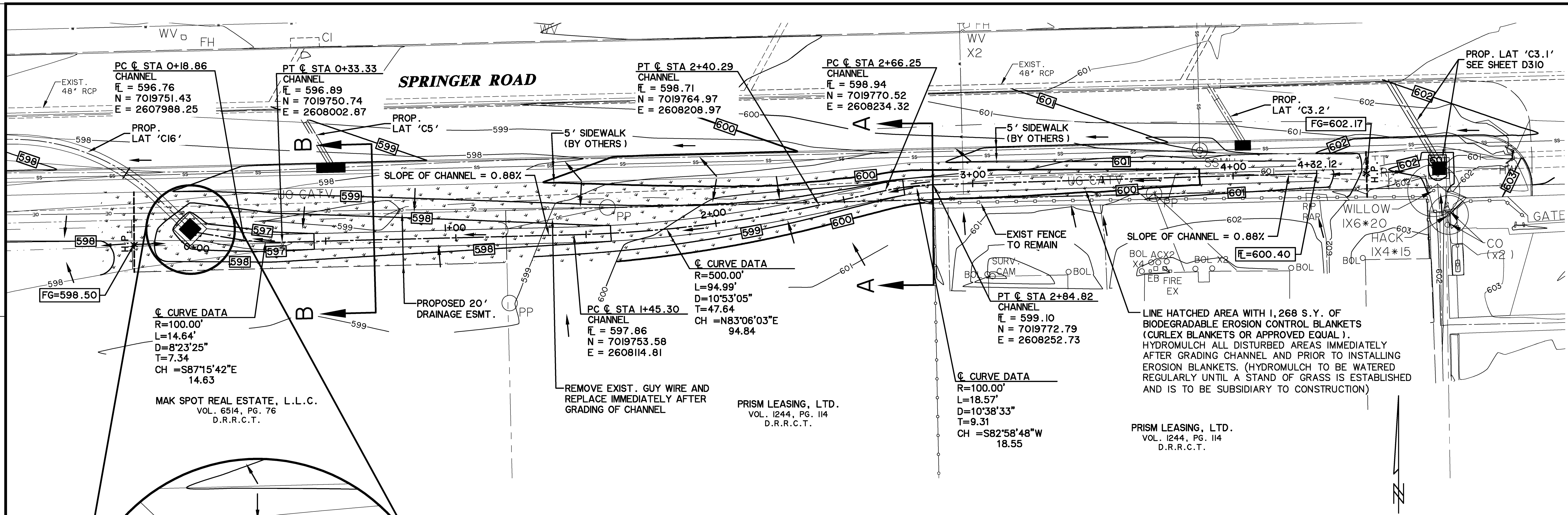


**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
DITCHOUT GRADING
CULVERT 'W'**



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*** BENCH MARKS ***

BM A - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.

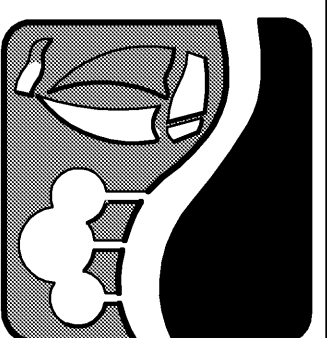
BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.

BM C - AN 'X' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**GRADING PLAN
SOUTH SIDE OF
SPRINGER ROAD
TO LATERAL 'C16'**



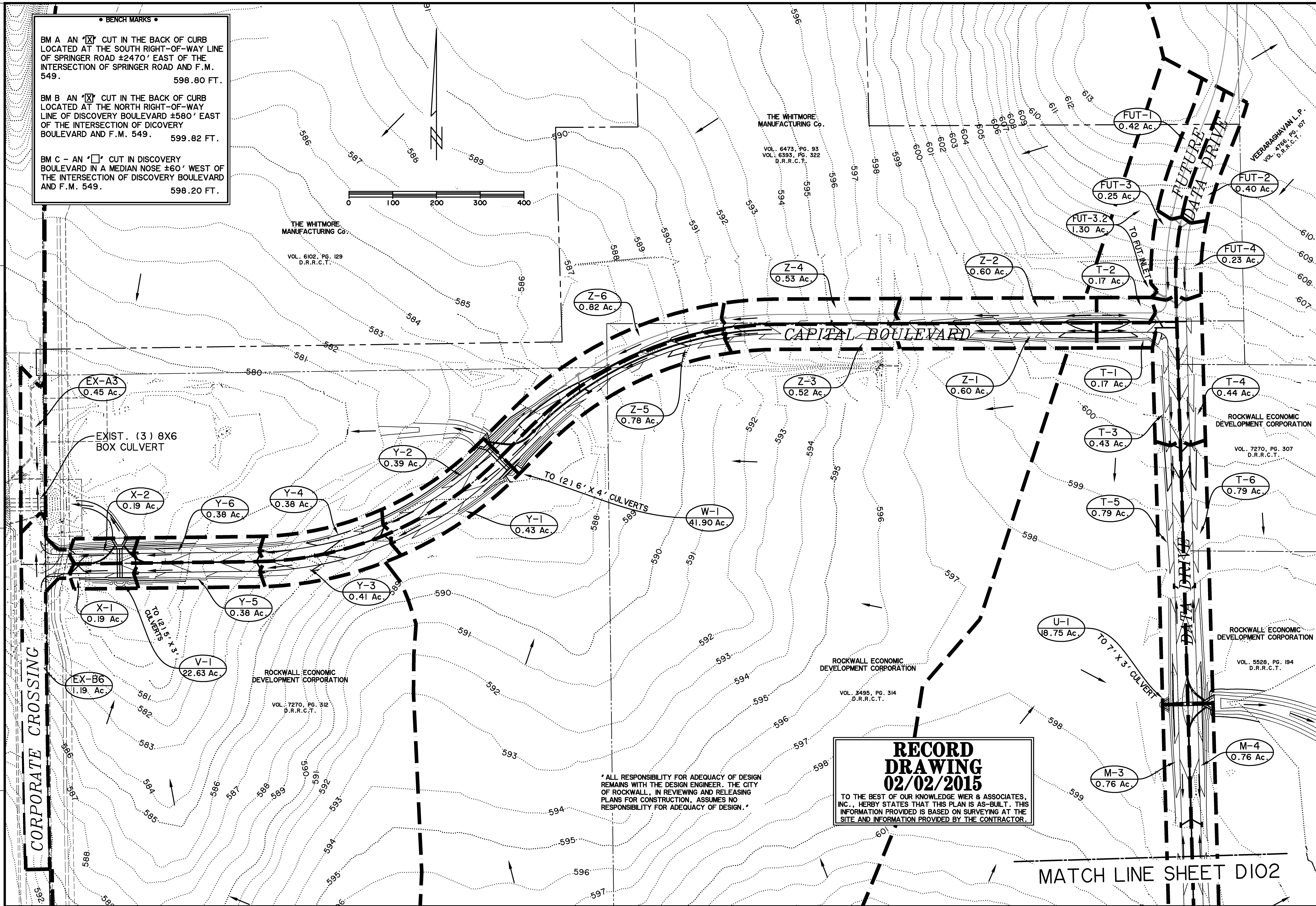
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G104**

598.20 FT.

SHEET NO.
D101

.....E97.....

TIME 12:01 FILE: D103-DAMAP-12209.dwg



• BENCH MARKS •

BM A - AN 'X' CUT IN THE BACK OF CURB
LOCATED AT THE SOUTH RIGHT-OF-WAY LINE
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549. 598.80 FT.

BM B - AN 'X' CUT IN THE BACK OF CURB
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BOULEVARD IN A MEDIAN NOSE ±60' WEST OF
THE INTERSECTION OF DISCOVERY BOULEVARD
AND F.M. 549. 598.20 FT.

THE WHITMORE
MANUFACTURING CO.
VOL. 6102, PG. 129
D.R.R.C.T.

THE WHITMORE
MANUFACTURING CO.
VOL. 6473, PG. 93
VOL. 6393, PG. 322
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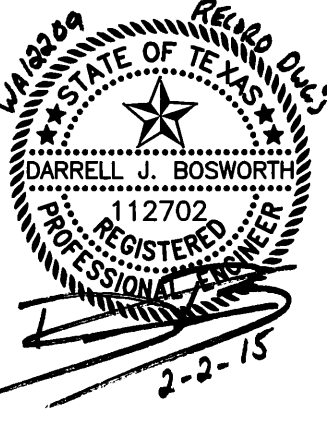
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**OVERALL
DRAINAGE
AREA MAP**



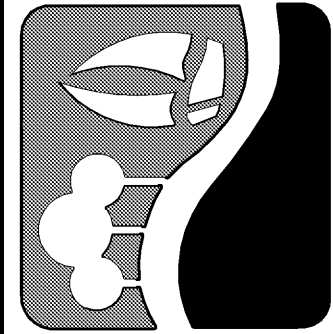
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DATE 10-07-2013
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**SHEET NO.
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MATCH LINE SHEET D102

Fully Developed Drainage Area Calculations						
Drainage Area Designation	Acreage (ac)	C Factor	C x A	Time of Concentration (min)	I (in/hr)	Q (cfs)
					100 Yr	100 Yr
A-1	0.28	0.90	0.25	10	9.80	2.5
A-2	0.31	0.90	0.28	10	9.80	2.7
A-3	0.45	0.90	0.41	10	9.80	4.0
A-4	0.49	0.90	0.44	10	9.80	4.3
A-5	0.46	0.90	0.41	10	9.80	4.1
A-6	0.47	0.90	0.42	10	9.80	4.1
A-7	0.63	0.90	0.57	10	9.80	5.6
A-8	0.67	0.90	0.60	10	9.80	5.9
A-9	0.46	0.90	0.41	10	9.80	4.1
A-10	0.42	0.90	0.38	10	9.80	3.7
A-11	0.52	0.90	0.47	10	9.80	4.6
A-12	0.50	0.90	0.45	10	9.80	4.4
B-1	0.44	0.90	0.40	10	9.80	3.9
B-2	0.44	0.90	0.40	10	9.80	3.9
B-3	0.44	0.90	0.40	10	9.80	3.9
B-4	0.44	0.90	0.40	10	9.80	3.9
B-5	0.44	0.90	0.40	10	9.80	3.9
B-6	0.44	0.90	0.40	10	9.80	3.9
C-1	0.29	0.90	0.26	10	9.80	2.6
C-2	0.57	0.90	0.51	10	9.80	5.0
C-3.1	3.34	0.90	3.01	10	9.80	29.5
C-3.2	0.27	0.90	0.24	10	9.80	2.4
C-4	0.72	0.90	0.65	10	9.80	6.4
C-5	0.24	0.90	0.22	10	9.80	2.1
C-6	0.73	0.90	0.66	10	9.80	6.4
C-8	0.62	0.90	0.56	10	9.80	5.5
C-10	0.51	0.90	0.46	10	9.80	4.5
C-16.1A	2.48	0.90	2.23	10	9.80	21.9
C-16.1B	3.63	0.90	3.27	10	9.80	32.0
C-16.7	0.48	0.90	0.43	10	9.80	4.2
C-16.9	0.61	0.90	0.55	10	9.80	5.4
D-1*	0.66	0.90	0.59	10	9.80	5.8
D-2*	0.21	0.90	0.19	10	9.80	1.9
D-3*	0.53	0.90	0.48	10	9.80	4.7
D-4*	0.18	0.90	0.16	10	9.80	1.6
E-1*	0.44	0.90	0.40	10	9.80	3.9
E-2*	1.05	0.90	0.95	10	9.80	9.3
F-1**	2.91	0.90	2.62	10	9.80	25.7
F-2**	5.39	0.90	4.85	10	9.80	47.5
G-1**	2.99	0.90	2.69	10	9.80	26.4
H-2**	8.53	0.90	7.68	10	9.80	75.2
H-3**	5.16	0.90	4.64	10	9.80	45.5
H-4**	5.53	0.90	4.98	10	9.80	48.8
I-1*	0.28	0.90	0.25	10	9.80	2.5
I-2*	0.74	0.90	0.67	10	9.80	6.5
J-1*	27.10	0.90	24.39	10	9.80	239.0
K-1*	7.40	0.90	6.66	10	9.80	65.3
M-1	0.40	0.90	0.36	10	9.80	3.5
M-2	0.30	0.90	0.27	10	9.80	2.6
M-3	0.76	0.90	0.68	10	9.80	6.7
M-4	0.76	0.90	0.68	10	9.80	6.7
N-1	0.09	0.90	0.08	10	9.80	0.8
N-2	0.45	0.90	0.41	10	9.80	4.0
P-1	0.38	0.90	0.34	10	9.80	3.4
P-2	1.02	0.90	0.92	10	9.80	9.0
FUT-1	0.42	0.90	0.38	10	9.80	3.7
FUT-2	0.40	0.90	0.36	10	9.80	3.5
FUT-3	0.25	0.90	0.23	10	9.80	2.2
FUT-3.2	1.30	0.90	1.17	10	9.80	11.5
FUT-4	0.23	0.90	0.21	10	9.80	2.0
T-1	0.17	0.90	0.15	10	9.80	1.5
T-2	0.17	0.90	0.15	10	9.80	1.5
T-3	0.43	0.90	0.39	10	9.80	3.8
T-4	0.44	0.90	0.40	10	9.80	3.9
T-5	0.79	0.90	0.71	10	9.80	7.0
T-6	0.79	0.90	0.71	10	9.80	7.0
U-1	18.75	0.90	16.88	10	9.80	165.4
V-1**	22.63	0.90	20.37	10	9.80	199.6
W-1**	41.90	0.90	37.71	10	9.80	369.6
X-1	0.19	0.90	0.17	10	9.80	1.7
X-2	0.19	0.90	0.17	10	9.80	1.7
Y-1	0.43	0.90	0.39	10	9.80	3.8
Y-2	0.39	0.90	0.35	10	9.80	3.4
Y-3	0.41	0.90	0.37	10	9.80	3.6
Y-4	0.38	0.90	0.34	10	9.80	3.4
Y-5	0.38	0.90	0.34	10	9.80	3.4
Y-6	0.38	0.90	0.34	10	9.80	3.4
Z-1	0.60	0.90	0.54	10	9.80	5.3
Z-2	0.60	0.90	0.54	10	9.80	5.3
Z-3	0.52	0.90	0.47	10	9.80	4.6
Z-4	0.53	0.90	0.48	10	9.80	4.7
Z-5	0.78	0.90	0.70	10	9.80	6.9
Z-6	0.82	0.90	0.74	10	9.80	7.2
EX-B6	1.19	0.90	1.07	10	9.80	10.5
EX-A3	0.45	0.90	0.41	10	9.80	4.0
Remarks: *DRAINAGE AREAS "D", "E", "I", "J" & "K" DRAIN TO EXISTING DETENTION PONDS **DETENTION REQUIRED, FLOWS MUST BE DETAINED TO PREDEVELOPED (C = 0.35, Tc = 20 min, I100 = 8.3)						

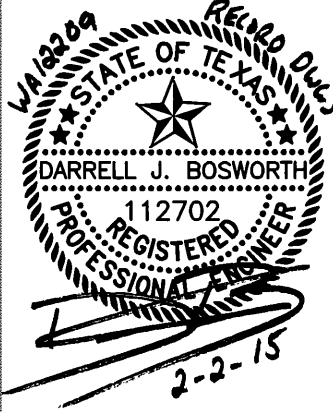
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ROCKWALL
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PARK
PHASE IV

DRAINAGE AREA
CALCULATIONS



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TIME 10:31 FILE: D202-DRAINCAL C2--I2209.dwg

100 YR. STORM INLET AND STREET FLOW CALCULATIONS																						
STREET STA	INLET NO.	CONTRIBUTING DRAINAGE AREA	DESIGN STORM FREQUENCY (yr)	TIME OF CONC (min)	RAINFALL INTENSITY (in/hr)	DRAINAGE AREA (Ac)	C' FACTOR	CxA	CA INTERCEPTED	Q FROM DA (cfs)	UPSTREAM CARRY-OVER (cfs)	GUTTER FLOW (cfs)	GUTTER SLOPE (%)	STREET SECTION	CROWN	DEPTH OF FLOW AT INLET (ft)	WIDTH OF FLOW IN STREET @ GUTTER (ft)	INLET LENGTH (ft)	INLET CAPACITY (cfs)	FLOW COLLECTED (cfs)	FLOW BYPASSED (cfs)	REMARKS
17+07.40 DISCOVERY	A1	A1	100	10	9.80	0.28	0.90	0.25	0.25	2.5	0.0	2.5	1.42	TRIANGULAR	0.5	0.18	8.8	5	2.4	2.4	0.1	EXISTING INLET BYPASS TO A3
16+91.39 DISCOVERY	A2	A2	100	10	9.80	0.31	0.90	0.28	0.26	2.7	0.0	2.7	1.42	TRIANGULAR	0.5	0.18	9.1	5	2.5	2.5	0.2	ON GRADE INLET / PH IV BYPASS TO A4
13+99.75 DISCOVERY	A3	A3	100	10	9.80	0.45	0.90	0.41	0.30	4.0	0.1	4.0	1.8	TRIANGULAR	0.5	0.20	10.1	5	2.9	2.9	1.1	EXISTING INLET BYPASS TO A5
13+90.02 DISCOVERY	A4	A4	100	10	9.80	0.49	0.90	0.44	0.31	4.3	0.2	4.6	1.7	TRIANGULAR	0.5	0.21	10.7	5	3.0	3.0	1.6	ON GRADE INLET / PH IV BYPASS TO A6
11+01.17 DISCOVERY	A5	A5	100	10	9.80	0.46	0.90	0.41	0.53	4.1	1.1	5.2	0.78	TRIANGULAR	0.5	0.26	13	10	5.5	5.2	0.0	EXISTING INLET
10+91.17 DISCOVERY	A6	A6	100	10	9.80	0.47	0.90	0.42	0.58	4.2	1.6	5.7	0.78	TRIANGULAR	0.5	0.27	13.5	10	5.7	5.7	0.0	EXISTING INLET
8+49.97 DISCOVERY	A7	A7	100	10	9.80	0.63	0.90	0.57	0.57	5.6	0.0	5.6	-	TRIANGULAR	0.5	0.27	-	15	-	5.6	0.0	EXISTING SUMP INLET
8+41.98 DISCOVERY	A8	A8	100	10	9.80	0.67	0.90	0.60	0.60	5.9	0.0	5.9	-	TRIANGULAR	0.5	0.28	-	15	-	5.9	0.0	EXISTING SUMP INLET
3+85.00 DISCOVERY	A9	A9	100	10	9.80	0.46	0.90	0.41	0.31	4.1	0.0	4.1	1.09	TRIANGULAR	0.5	0.22	11.1	5	3.0	3.0	1.1	EXISTING INLET BYPASS TO A12
3+75.00 DISCOVERY	A10	A10	100	10	9.80	0.42	0.90	0.38	0.30	3.7	0.0	3.7	1.09	TRIANGULAR	0.5	0.21	10.7	5	2.9	2.9	0.8	EXISTING INLET BYPASS TO A11
2+26.92 DISCOVERY	A11	A11	100	10	9.80	0.52	0.90	0.47	0.55	4.6	0.8	5.4	-	TRIANGULAR	0.5	0.27	-	20	-	5.4	0.0	EXISTING SUMP INLET
2+18.92 DISCOVERY	A12	A12	100	10	9.80	0.50	0.90	0.45	0.56	4.4	1.1	5.5	-	TRIANGULAR	0.5	0.27	-	20	-	5.5	0.0	EXISTING SUMP INLET
21+70.90 DISCOVERY	B1	B1	100	10	9.80	0.44	0.90	0.40	0.31	3.9	0.0	3.9	1.33	TRIANGULAR	0.5	0.23	11.7	5	3.0	3.0	0.9	ON GRADE INLET / PH IV BYPASS TO B3
21+70.90 DISCOVERY	B2	B2	100	10	9.80	0.44	0.90	0.40	0.31	3.9	0.0	3.9	1.33	TRIANGULAR	0.5	0.23	11.7	5	3.0	3.0	0.9	EXISTING INLET BYPASS TO B4
24+52.90 DISCOVERY	B3	B3	100	10	9.80	0.44	0.90	0.40	0.49	3.9	0.9	4.8	1.33	TRIANGULAR	0.5	0.25	12.7	10	5.4	4.8	0.0	ON GRADE INLET / PH IV
24+52.90 DISCOVERY	B4	B4	100	10	9.80	0.44	0.90	0.40	0.49	3.9	0.9	4.8	1.33	TRIANGULAR	0.5	0.25	12.7	10	5.4	4.8	0.0	EXISTING INLET
27+29.60 DISCOVERY	B5	B5	100	10	9.80	0.44	0.90	0.40	0.40	3.9	0.0	3.9	1.33	TRIANGULAR	0.5	0.23	11.7	10	5.2	3.9	0.0	ON GRADE INLET / PH IV
27+34.89 DISCOVERY	B6	B6	100	10	9.80	0.44	0.90	0.40	0.40	3.9	0.0	3.9	1.33	TRIANGULAR	0.5	0.23	11.7	10	5.2	3.9	0.0	EXISTING INLET
13+88.34 SPRINGER	C1	C1	100	10	9.80	0.29	0.90	0.26	0.26	2.6	0.0	2.6	0.6	TRIANGULAR	0.5	0.22	11.1	5	2.6	2.6	0.0	ON GRADE INLET / PH IV
14+23.86 SPRINGER	C2	C2	100	10	9.80	0.57	0.90	0.51	0.36	5.0	0.0	5.0	0.6	TRIANGULAR	0.5	0.28	14.2	5	3.5	3.5	1.5	EXISTING INLET BYPASS TO C4
11+59.11 SPRINGER	C3.1	C3.1	100	10	9.80	3.34	0.90	3.01	3.01	-	-	-	-	-	-	0.84	-	4'x4'	50.2	29.5	0.0	OFFSITE TYPE IL-HL DROP INLET
10+84.30 SPRINGER	C3.2	C3.2	100	10	9.80	0.27	0.90	0.24	0.24	2.4	0.0	2.4	0.7	TRIANGULAR	0.5	0.21	10.4	5	2.6	2.4	0.0	ON GRADE INLET / PH IV
10+73.86 SPRINGER	C4	C4	100	10	9.80	0.72	0.90	0.65	0.71	6.4	1.5	7.9	0.7	TRIANGULAR	0.5	0.33	16.3	10	7.0	7.0	0.9	EXISTING INLET BYPASS TO C6
7+34.14 SPRINGER	C5	C5	100	10	9.80	0.24	0.90	0.22	0.22	2.1	0.0	2.1	0.7	TRIANGULAR	0.5	0.20	9.9	5	2.5	2.1	0.0	ON GRADE INLET / PH IV
7+23.86 SPRINGER	C6	C6	100	10	9.80	0.73	0.90	0.66	0.68	6.4	0.9	7.3	0.7	TRIANGULAR	0.5	0.32	15.9	10	6.7	6.7	0.6	EXISTING INLET BYPASS TO C8
4+23.48 SPRINGER	C8	C8	100	10	9.80	0.62	0.90	0.56	0.61	5.5	0.6	6.1	0.7	TRIANGULAR	0.5	0.30	14.9	10	6.0	6.0	0.1	EXISTING INLET BYPASS TO C10
4+23.48 SPRINGER	C10	C10	100	10	9.80	0.51	0.90	0.46	0.47	4.5	0.1	4.6	-	TRIANGULAR	1.5	0.27	-	10	-	4.6	0.0	EXISTING SAG INLET
6+77.83 SPRINGER	C16.1	C16.1	100	10	9.80	6.11	0.90	5.50	5.50	-	-	-	-	-	-	1.50	-	5'x5'	70.6	53.9	0.0	OFFSITE TYPE IL-HL DROP INLET
4+33.76 SPRINGER	C16.7	C16.7	100	10	9.80	0.48	0.90	0.43	0.43	4.2	0.0	4.2	0.7	TRIANGULAR	0.5	0.26	12.9	10	5.5	4.2	0.0	ON GRADE INLET / PH IV
1+83.25 SPRINGER	C16.9	C16.9	100	10	9.80	0.61	0.90	0.55	0.55	5.4	0.0	5.4	-	TRIANGULAR	1.5	0.28	-	10	-	5.4	0.0	SAG INLET / PH IV
20+37.61 SPRINGER	D1	D1	100	10	9.80	0.66	0.90	0.59	0.59	5.8	0.0	5.8	0.84	TRIANGULAR	0.5	0.30	15	10	5.9	5.8	0.0	EXISTING INLET
20+37.61 SPRINGER	D2	D2	100	10	9.80	0.21	0.90	0.19	0.19	1.9	0.0	1.9	0.84	TRIANGULAR	0.5	0.20	9.8	10	5.0	1.9	0.0	ON GRADE INLET / PH IV
22+87.61 SPRINGER	D3	D3	100	10	9.80	0.53	0.90	0.48	0.48	4.7	0.0	4.7	0.7	TRIANGULAR	0.5	0.29	14.3	10	5.8	4.7	0.0	EXISTING INLET
22+87.61 SPRINGER	D4	D4	100	10	9.80	0.18	0.90	0.16	0.16	1.6	0.0	1.6	0.7	TRIANGULAR	0.5	0.19	9.6	10	4.9	1.6	0.0	ON GRADE INLET / PH IV
4+50.62 DATA	E1	E1	100	10	9.80	0.44	0.90	0.40	0.40	3.9	0.0	3.9	-	TRIANGULAR	0.5	0.27	-	10	-	3.9	0.0	SAG INLET / PH IV
4+50.62 DATA	E2	E2	100	10	9.80	1.05	0.90	0.94	0.94	9.3	0.0	9.3	-	TRIANGULAR	0.5	0.37	-	10	-	9.3	0.0	EXISTING SAG INLET
25+10.31 SPRINGER	I1	I1	100	10	9.80	0.28	0.90	0.25	0.25	2.5	0.0	2.5	-	TRIANGULAR	0.5	0.22	-	10	-	2.5	0.0	SAG INLET / PH IV
25+08.01 SPRINGER	I2	I2	100	10	9.80	0.74	0.90	0.67	0.67	6.5	0.0	6.5	-	TRIANGULAR	0.5	0.32	-	10	-	6.5	0.0	EXISTING SAG INLET
9+92.15 DATA	P1	P1	100	10	9.80	0.38	0.90	0.34	0.34	3.3	0.0	3.3	-	TRIANGULAR	0.5	0.25	-	10	-	3.3	0.0	SAG INLET / PH IV
9+92.15 DATA	P2	P2	100	10	9.80	1.02	0.90	0.92	0.92	9.0	0.0	9.0	-	TRIANGULAR	0.5	0.36	-	10	-	9.0	0.0	EXISTING SAG INLET
17+30.00 DATA	M1	M1	100	10	9.80	0.40	0.90	0.36	0.36	3.5	0.0	3.5	0.73	TRIANGULAR	0.5	0.24	11.7	10	5.3	3.5	0.0	ON GRADE INLET / PH IV
17+30.00 DATA	M2	M2	100	10	9.80	0.30	0.90	0.27	0.27	2.6	0.0	2.6	0.73	TRIANGULAR	0.5	0.22	10.4	10	5.1	2.6	0.0	ON GRADE INLET / PH IV
20+27.38 DATA	M3	M3	100	10	9.80	0.76	0.90	0.68	0.68	6.7	0.0	6.7	-	TRIANGULAR	0.5	0.33	-	10	-	6.7	0.0	SAG INLET / PH IV
20+27.38 DATA	M4	M4	100	10	9.80	0.76	0.90	0.68	0.68	6.7	0.0	6.7	-	TRIANGULAR	0.5	0.33	-	10	-	6.7	0.0	SAG INLET / PH IV
34+70 DATA	FUT1	FUT1	100	10	9.80	0.42	0.90	0.38	0.38	3.7	0.0	3.7	1.06	TRIANGULAR	0.5	0.23	11.1	10	-	3.7	0.0	ON GRADE INLET / FUTURE
34+70 DATA	FUT2	FUT2	100	10	9.80	0.40	0.90	0.36	0.36	3.5	0.0	3.5	1.06	TRIANGULAR	0.5	0.23	10.9	10	-	3.5	0.0	ON GRADE INLET / FUTURE
32+32 DATA	FUT3	FUT3	100	10	9.80	0.25	0.90	0.22	0.22	2.2	0.0	2.2	1.06	TRIANGULAR	0.5	0.19	9.1	10	-	2.2	0.0	ON GRADE INLET / FUTURE
32+32 DATA	FUT4	FUT4	100	10	9.80	0.23	0.90	0.21	0.21	2.0	0.0	2.0	1.06	TRIANGULAR	0.5	0.18	8.8	10	-	2.0	0.0	ON GRADE INLET / FUTURE
27+47.08 CAPITAL	T1	T1	100	10	9.80	0.17	0.90	0.15	0.15	1.5	0.0	1.5	1.55	TRIANGULAR	0.5	0.15	7.4	5	2.3	1.5	0.0	ON GRADE INLET / PH IV
27+47.08 CAPITAL	T2	T2	100	10	9.80	0.17	0.90	0.15	0.15	1.5	0.0	1.5	1.55	TRIANGULAR	0.5	0.15	7.4	5	2.3	1.5	0.0	ON GRADE INLET / PH IV
29+00 DATA	T3	T3	100	10	9.80	0.43	0.90	0.39	0.39	3.8	0.0	3.8	1.06	TRIANGULAR	0.5	0.23	11.2	10	5.2	3.8	0.0	ON GRADE INLET / PH IV
29+00 DATA	T4	T4	100	10	9.80	0.44	0.90	0.40	0.40	3.9	0.0	3.9	1.06	TRIANGULAR	0.5	0.24	11.3	10	5.3	3.9	0.0	ON GRADE INLET / PH IV
25.73.58 DATA	T5	T5	100	10	9.80	0.79	0.90	0.71	0.71	7.0	0.0	7.0	-	TRIANGULAR	0.5	0.29	-	10	-	7.0	0.0	SAG INLET / PH IV
25.73.58 DATA	T6	T6	100	10	9.80	0.79	0.90	0.71	0.71	7.0	0.0	7.0	-	TRIANGULAR	0.5	0.29	-	10	-	7.0	0.0	SAG INLET / PH IV
1+12 CAPITAL	X1	X1	100	10	9.80	0.19	0.90	0.17	0.17	1.7	0.0	1.7	0.85	TRIANGULAR	0.5	0.18	8.6	5	2.4	1.7	0.0	ON GRADE INLET / PH IV
1+12 CAPITAL	X2	X2	100	10	9.80	0.19	0.90	0.17	0.17	1.7	0.0	1.7	0.85	TRIANGULAR	0.5	0.18	8.6	5	2.4	1.7	0.0	ON GRADE INLET / PH IV
8+50 CAPITAL	Y1	Y1	100	10	9.80	0.43	0.90	0.39	0.39	3.8	0.0	3.8	0.85	TRIANGULAR	0.5	0.24	11.7	10	5.3	3.8	0.0	ON GRADE INLET / PH IV
8+50 CAPITAL	Y2	Y2	100	10	9.80	0.39	0.90	0.35	0.35	3.4	0.0	3.4	0.85	TRIANGULAR	0.5	0.23	11.2	10	5.2	3.4	0.0	ON GRADE INLET / PH IV
5+50 CAPITAL	Y3	Y3	100	10	9.80	0.41	0.90	0.37	0.37	3.6	0.0	3.6	0.85	TRIANGULAR	0.5	0.24	11.5	10	5.3	3.6	0.0	ON GRADE INLET / PH IV
5+50 CAPITAL	Y4	Y4	100	10	9.80	0.38	0.90	0.34	0.34	3.4	0.0	3.4	0.85	TRIANGULAR	0.5	0.23	11.2	10	5.2	3.4	0.0	ON GRADE INLET / PH IV
2+62 CAPITAL	Y5	Y5	100	10	9.80	0.3																

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STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LINE A																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET A1	17+12.81	35.8	0.24	10	0.24	10.0	100	9.80	2.4	18	1.4	0.0005	1.25	0.04	0.1	10.1	608.12	PROPOSED INLET
17+12.81	17+00.81	12	-	-	0.24	10.1	100	9.78	2.3	18	1.3	0.0005	0.5	0.01	0.0	10.1	605.02	
INLET A2	17+00.81	36.75	0.26	10	0.26	10.0	100	9.80	2.5	18	1.4	0.0006	1.25	0.04	0.1	10.1	607.98	
17+00.81	14+14.48	286.33	-	-	0.50	10.1	100	9.78	4.9	18	2.8	0.0022	0.5	0.11	0.7	10.8	604.98	
14+14.48	14+09.48	5	-	-	0.50	10.8	100	9.64	4.8	24	1.5	0.0005	0.5	0.00	0.1	10.9	601.33	
INLET A3	14+09.48	36.3	0.30	10	0.30	10.0	100	9.80	2.9	18	1.6	0.0008	1.25	0.05	0.1	10.1	601.56	PROPOSED INLET
14+09.48	13+99.85	9.63	-	-	0.80	10.9	100	9.62	7.7	24	2.5	0.0012	0.5	0.08	0.1	11.0	601.33	
INLET A4	13+99.85	36.39	0.31	10	0.31	10.0	100	9.80	3.0	18	1.7	0.0008	1.25	0.06	0.1	10.1	601.39	
13+99.85	13+89.53	10.32	-	-	1.11	11.0	100	9.61	10.7	24	3.4	0.0022	0.5	0.13	0.0	11.0	601.24	
13+89.53	12+50	139.53	-	-	1.11	11.0	100	9.61	10.7	24	3.4	0.0022	0.5	0.09	0.3	11.3	601.07	
12+50	11+11.04	138.96	-	-	1.11	11.3	100	9.55	10.6	24	3.4	0.0022	0.25	0.04	0.7	12.0	599.32	CALCS UPDATED FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09 WITH NEW STARTING HG = 584.80 ('LAT C1+03' IN SD LINE 'C')
INLET A5	11+11.04	36.37	0.53	10	0.53	10.0	100	9.80	5.2	18	2.9	0.0025	1.25	0.17	0.2	10.2	599.23	
11+11.04	11+01.04	10	-	-	1.64	12.0	100	9.43	15.5	33	2.6	0.0009	0.5	0.02	0.1	12.1	598.97	
INLET A6	11+01.04	36.37	0.58	10	0.58	10.0	100	9.80	5.7	18	3.2	0.0029	1.25	0.20	0.2	10.2	599.25	
11+01.04	8+59.98	241.06	-	-	2.22	12.1	100	9.42	20.9	33	3.5	0.0016	0.5	0.14	0.8	12.9	598.94	
INLET A7	8+59.98	36.16	0.57	10	0.57	10.0	100	9.80	5.6	24	1.8	0.0006	1.25	0.06	0.3	10.3	598.45	CALCS UPDATED FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09 WITH NEW STARTING HG = 584.80 ('LAT C1+03' IN SD LINE 'C')
8+59.98	8+51.35	8.63	-	-	2.79	12.9	100	9.29	25.9	36	3.7	0.0015	0.5	0.12	0.0	12.9	598.37	
INLET A8	0+23.05	14.27	0.60	10	0.60	10.0	100	9.80	5.9	24	1.9	0.0007	1.25	0.07	0.1	10.1	598.34	
0+23.05	8+51.35	23.05	-	-	0.60	10.1	100	9.78	5.9	24	1.9	0.0007	0	0.00	0.2	10.3	598.26	
8+51.35	4+00.84	450.51	-	-	3.39	12.9	100	9.29	31.5	36	4.5	0.0022	0.5	0.21	1.0	13.9	598.24	
4+00.84	3+95.72	5.12	-	-	3.39	13.9	100	9.15	31.0	36	4.4	0.0022	0	0.00	0.0	13.9	596.36	
INLET A9	3+95.72	36.03	0.31	10	0.31	10.0	100	9.80	3.0	18	1.7	0.0008	1.25	0.06	0.4	10.4	596.44	
3+95.72	3+84.43	11.29	-	-	3.70	13.9	100	9.15	33.9	36	4.8	0.0026	0.5	0.21	0.0	13.9	596.35	CALCS UPDATED FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09 WITH NEW STARTING HG = 584.80 ('LAT C1+03' IN SD LINE 'C')
INLET A10	3+84.43	36.27	0.30	10	0.30	10.0	100	9.80	2.9	18	1.6	0.0008	1.25	0.05	0.4	10.4	596.19	
3+84.43	2+45.79	138.64	-	-	4.00	13.9	100	9.15	36.6	36	5.2	0.0030	0.5	0.24	0.4	14.3	596.11	
2+45.79	2+36.79	9	-	-	4.00	14.3	100	9.09	36.4	4x3	3.0	0.0009	0.5	0.07	0.1	14.4	595.45	
INLET A11	2+36.79	36.37	0.55	10	0.55	10.0	100	9.80	5.4	24	1.7	0.0006	1.25	0.06	0.4	10.4	595.45	
2+36.79	2+28.79	8	-	-	4.55	14.4	100	9.08	41.3	4x3	3.4	0.0011	0.5	0.11	0.0	14.4	595.37	
INLET A12	2+28.79	36.37	0.56	10	0.56	10.0	100	9.80	5.5	24	1.8	0.0006	1.25	0.06	0.3	10.3	595.33	
2+28.79	1+88.57	40.22	-	-	5.11	14.4	100	9.08	46.4	4x3	3.9	0.0014	0.5	0.15	0.1	14.5	595.25	CALCS UPDATED FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09 WITH NEW STARTING HG = 584.80 ('LAT C1+03' IN SD LINE 'C')
1+88.57	1+03.24	85.33	-	-	5.11	14.5	100	9.06	46.3	4x3	3.9	0.0014	0.5	0.12	0.2	14.7	595.04	
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LINE B																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET B1	6+30.19	36.37	0.31	10	0.31	10.0	100	9.80	3.0	18	1.7	0.0008	1.25	0.06	0.4	10.4	607.08	PROPOSED INLET
6+30.19	6+16.88	13.31	-	-	0.31	10.4	100	9.72	3.0	24	1.0	0.0002	0.35	0.00	0.0	10.4	606.99	
INLET B2	6+16.88	44.55	0.31	10	0.31	10.0	100	9.80	3.0	18	1.7	0.0008	1.25	0.06	0.4	10.4	606.93	
6+16.88	3+48.19	268.69	-	-	0.62	10.4	100	9.72	6.0	24	1.9	0.0007	0.75	0.04	0.6	11.0	606.83	
INLET B3	3+48.19	36.37	0.49	10	0.49	10.0	100	9.80	4.8	18	2.7	0.0021	1.25	0.14	0.1	10.1	603.25	
3+48.19	3+34.88	13.31	-	-	1.11	11.0	100	9.61	10.7	24	3.4	0.0022	0.75	0.14	0.1	11.1	603.01	PROPOSED INLET
INLET B4	3+34.88	44.55	0.49	10	0.49	10.0	100	9.80	4.8	18	2.7	0.0021	1.25	0.14	0.1	10.1	603.25	
3+34.88	0+71.20	263.68	-	-	1.60	11.1	100	9.59	15.3	24	4.9	0.0046	0.75	0.24	0.5	11.6	602.84	
INLET B5	0+71.20	36.37	0.40	10	0.40	10.0	100	9.80	3.9	18	2.2	0.0014	1.25	0.09	0.3	10.3	600.48	
0+71.20	0+52.39	18.81	-	-	2.00	11.6	100	9.50	19.0	24	6.1	0.0071	0.75	0.30	0.1	11.7	600.34	
INLET B6	0+52.39	45.25	0.40	10	0.40	10.0	100	9.80	3.9	18	2.2	0.0014	1.25	0.09	0.3	10.3	600.06	PROPOSED INLET
0+52.39	1+49.97	52.39	-	-	2.40	11.7	100	9.48	22.8	24	7.3	0.0102	0.75	0.41	0.1	11.8	599.91	
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LINE C																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET C2	14+50.75	25.48	0.36	10	0.36	10.0	100	9.80	3.5	18	2.0	0.0011	1.25	0.08	0.0	10.0	601.60	EXISTING INLET
14+50.75	14+20.56	30.19	-	-	0.36	10.0	100	9.80	3.5	24	1.1	0.0002	0.5	0.00	0.1	10.1		

STORM DRAIN CALCULATIONS FOR STORM DRAIN LAT C16																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET C16.1	4+18.06	262.10	5.50	10	5.50	10.0	100	9.80	53.9	42	5.6	0.0029	1.25	0.61	0.4	10.4	596.71	
INLET C16.7	4+18.06	14.43	0.43	10	0.43	10.0	100	9.80	4.2	18	2.4	0.0016	1.25	0.11	0.1	10.1	595.10	
4+18.06	1+67.55	250.51	-	-	5.93	10.4	100	9.72	57.6	42	6.0	0.0033	0.2	0.46	0.7	11.1	594.97	
INLET C16.9	1+67.55	14.43	0.55	10	0.55	10.0	100	9.80	5.4	18	3.1	0.0026	1.25	0.18	0.1	10.1	593.90	
1+67.55	0+67.61	100.06	-	-	6.48	11.1	100	9.59	62.1	42	6.5	0.0038	0.2	0.54	0.3	11.4	593.68	
0+67.61	0+62.61	5.00	-	-	6.48	11.4	100	9.53	61.8	42	6.4	0.0038	0.45	0.29	0.0	11.4	592.76	
0+62.61	7+04.08	54.28	-	-	6.48	11.4	100	9.53	61.8	45	5.6	0.0026	0	0.00	0.2	11.6	592.45	EXISTING STUBOUT
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LINE D																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET D1	4+30.15	25.40	0.59	10	0.59	10.0	100	9.80	5.8	18	3.3	0.0030	1.25	0.21	0.1	10.1	601.34	
4+30.15	4+20.85	9.3	-	-	0.59	10.1	100	9.78	5.8	24	1.8	0.0007	0.35	0.02	0.1	10.2	600.95	
INLET D2	4+20.85	31.11	0.19	10	0.19	10.0	100	9.80	1.9	18	1.1	0.0003	1.25	0.02	0.1	10.1	601.23	PROPOSED INLET, UPDATED CA FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09
4+20.85	1+80.15	240.7	-	-	0.78	10.2	100	9.76	7.6	24	2.4	0.0011	0.75	0.05	0.8	11.0	600.92	
INLET D3	1+80.15	31.11	0.48	10	0.48	10.0	100	9.80	4.7	21	2.0	0.0009	1.25	0.07	0.3	10.3	599.76	
1+80.15	1+70.85	9.3	-	-	1.26	11.0	100	9.61	12.1	27	3.0	0.0015	0.75	0.07	0.1	11.1	599.66	
INLET D4	1+70.85	31.11	0.16	10	0.16	10.0	100	9.80	1.6	21	0.7	0.0001	1.25	0.01	0.7	10.7	599.59	PROPOSED INLET, UPDATED CA FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09
1+70.85	1+53.92	16.93	-	-	1.42	11.1	100	9.59	13.6	27	3.4	0.0019	0.75	0.07	0.1	11.2	599.58	
1+53.92	0+35	118.92	-	-	1.42	11.2	100	9.57	13.6	27	3.4	0.0019	0.35	0.06	0.3	11.5	599.48	STARTING HG USED FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LINE E																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET E1	INLET E2	45	0.40	10	0.40	10.0	100	9.80	3.9	21	1.6	0.0006	1.25	0.05	0.5	10.5	598.59	PROPOSED INLET
INLET E2	0+35.33	120.66	0.94	10	1.34	10.5	100	9.70	13.0	21	5.4	0.0067	0.35	0.16	0.4	10.9	598.51	
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LINE I																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET I1	INLET I2	45	0.25	10	0.25	10.0	100	9.80	2.5	18	1.4	0.0006	1.25	0.04	0.2	10.2	598.37	PROPOSED INLET, UPDATED CA FROM PH-III PLANS BY WIER & ASSOCIATES DATED 4/19/09
INLET I2	0+34.30	80.45	0.67	10	0.92	10.2	100	9.76	9.0	21	3.7	0.0032	0.35	0.08	0.2	10.4	598.21	
STORM DRAIN CALCULATIONS FOR STORM DRAIN LINE M																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET M1	6+73.00	25.98	0.36	10	0.36	10.0	100	9.80	3.5	18	2.0	0.0011	1.25	0.08	0.1	10.1	597.07	
6+73.00	6+63.49	9.51	-	-	0.36	10.1	100	9.78	3.5	24	1.1	0.0002	0.6	0.00	0.0	10.1	596.78	
INLET M2	6+63.49	31.82	0.27	10	0.27	10.0	100	9.80	2.6	18	1.5	0.0006	1.25	0.04	0.1	10.1	597.03	
6+63.49	3+76.79	286.7	-	-	0.63	10.1	100	9.78	6.2	24	2.0	0.0008	0.4	0.05	0.9	11.0	596.77	
INLET M3	3+76.79	23.66	0.68	10	0.68	10.0	100	9.80	6.7	18	3.8	0.0041	1.25	0.28	0.1	10.1	596.39	
INLET M4	3+76.79	23.66	0.68	10	0.68	10.0	100	9.80	6.7	18	3.8	0.0041	1.25	0.28	0.1	10.1	596.39	
3+76.79	1+04.57	272.22	-	-	2.00	11.0	100	9.61	19.2	27	4.8	0.0038	0.5	0.33	1.0	12.0	596.01	
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LINE P																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET P1	4+65.42	26.35	0.34	10	0.34	10.0	100	9.80	3.3	18	1.9	0.0010	1.25	0.07	0.2	10.2	599.14	PROPOSED INLET
4+65.42	4+57.92	7.5	-	-	0.34	10.2	100	9.76	3.3	27	0.8	0.0001	0.75	0.00	0.2	10.4	599.04	
INLET P2	4+57.92	31.11	0.92	10	0.92	10.0	100	9.80	9.0	21	3.7	0.0032	1.25	0.27	0.1	10.1	599.41	
4+57.92	1+13.35	457.92	-	-	1.26	10.4	100	9.72	12.2	27	3.1	0.0016	0.75	0.14	2.5	12.9	599.04	
STORM DRAIN CALCULATIONS FOR STORM DRAIN LINE T																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET FUT1	2+61.86	25.98	0.38	10	0.38	10.0	100	9.80	3.7	18	2.1	0.0012	1.25	0.09	0.0	10.0	604.32	
2+61.86	2+52.35	9.51	-	-	0.38	10.0	100	9.80	3.7	24	1.2	0.0003	0.6	0.00	0.0	10.0	602.76	
INLET FUT2	2+52.35	31.32	0.36	10	0.36	10.0	100	9.80	3.5	18	2.0	0.0011	1.25	0.08	0.1	10.1	604.31	
2+52.35	0+53.85	198.5	-	-	0.74	10.1	100	9.78	7.2	24	2.3	0.0010	0.4	0.07	0.5	10.6	602.71	
FIFUT5	INLET FUT3	25	1.17	10	1.17	10.0	100	9.80	11.5	24	3.7	0.0026	1.25	0.26	0.1	10.1	602.07	
INLET FUT3	0+53.85	25.98	0.22	10	1.40	10.1	100	9.78	13.6	24	4.3	0.0036	0.5	0.18	0.1	10.2	601.74	
0+53.85	0+44.34	9.51	-	-	2.13	10.6	100	9.68	20.6	27	5.2	0.0044	0.6	0.37	0.0	10.6	601.47	
INLET FUT4	0+44.34	31.82	0.21	10	0.21	10.0	100	9.80	2.0	18	1.1	0.0004	1.25	0.02	0.1	10.1	602.23	
0+44.34	JBOX	44.34	-	-	2.34	10.6	100	9.68	22.7	27	5.7	0.0054	0					

STORM DRAIN CALCULATIONS FOR STORM DRAIN LINE X																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET X1	2+09.72	25.98	0.17	10	0.17	10.0	100	9.80	1.7	18	1.0	0.0003	1.25	0.02	0.4	10.4	577.80	
2+09.72	2+00.21	9.51	-	-	0.17	10.4	100	9.72	1.7	24	0.5	0.0001	0.6	0.00	0.0	10.4	577.77	
INLET X2	2+00.21	31.82	0.17	10	0.17	10.0	100	9.80	1.7	18	1.0	0.0003	1.25	0.02	0.5	10.5	577.75	
2+00.21	1+03.65	95.96	-	-	0.34	10.5	100	9.70	3.3	24	1.1	0.0002	0.4	0.02	0.4	10.9	577.72	
STORM DRAIN CALCULATIONS FOR STORM DRAIN LINE Y																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET Y1	7+21.65	26.11	0.39	10	0.39	10.0	100	9.80	3.8	18	2.2	0.0013	1.25	0.09	0.0	10.0	584.03	
7+21.65	7+12.52	9.13	-	-	0.39	10.0	100	9.80	3.8	24	1.2	0.0003	0.6	0.00	0.0	10.0	583.24	
INLET Y2	7+12.52	31.37	0.35	10	0.35	10.0	100	9.80	3.4	18	1.9	0.0010	1.25	0.07	0.1	10.1	584.01	
7+12.52	4+23.33	289.19	-	-	0.74	10.1	100	9.78	7.2	24	2.3	0.0010	0.4	0.07	0.8	10.9	583.22	
INLET Y3	4+23.33	25.52	0.37	10	0.37	10.0	100	9.80	3.6	18	2.0	0.0012	1.25	0.08	0.0	10.0	581.46	
INLET Y4	4+23.33	25.38	0.34	10	0.34	10.0	100	9.80	3.4	18	1.9	0.0010	1.25	0.07	0.0	10.0	581.45	
4+23.33	1+43.68	279.65	-	-	1.45	10.9	100	9.62	13.9	24	4.4	0.0038	0.25	0.28	0.6	11.5	580.54	
INLET Y5	1+43.68	34.03	0.34	10	0.34	10.0	100	9.80	3.4	18	1.9	0.0010	1.25	0.07	0.1	10.1	578.99	
1+43.68	1+23.42	20.26	-	-	1.79	11.5	100	9.52	17.1	27	4.3	0.0030	0.3	0.20	0.1	11.6	578.17	
INLET Y6	1+23.42	10.41	0.34	10	0.34	10.0	100	9.80	3.4	18	1.9	0.0010	1.25	0.07	0.0	10.0	578.99	
1+23.42	1+03.67	20.3	-	-	2.13	11.6	100	9.50	20.3	27	5.1	0.0043	0.4	0.29	0.1	11.7	577.91	
STORM DRAIN CALCULATIONS FOR STORM DRAIN LINE Z																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET Z1	11+04.06	12.12	0.54	10	0.54	10.0	100	9.80	5.3	18	3.0	0.0025	1.25	0.17	0.0	10.0	595.17	
11+04.06	10+90.20	13.86	-	-	0.54	10.0	100	9.80	5.3	24	1.7	0.0005	0.6	0.00	0.0	10.0	593.70	
INLET Z2	10+90.20	39.84	0.54	10	0.54	10.0	100	9.80	5.3	18	3.0	0.0025	1.25	0.17	0.1	10.1	595.17	
10+90.20	7+10.85	379.35	-	-	1.08	10.1	100	9.78	10.6	24	3.4	0.0022	0.4	0.16	0.8	10.9	593.67	
INLET Z3	7+10.85	12.4	0.47	10	0.47	10.0	100	9.80	4.6	18	2.6	0.0019	1.25	0.13	0.0	10.0	589.40	
INLET Z4	7+10.85	35.08	0.48	10	0.48	10.0	100	9.80	4.7	18	2.7	0.0020	1.25	0.14	0.1	10.1	589.41	
7+10.85	4+98.53	212.15	-	-	2.03	10.9	100	9.62	19.5	27	4.9	0.0040	0.25	0.33	0.4	11.3	588.72	
4+98.53	3+66.67	132.03	-	-	2.03	11.3	100	9.55	19.3	30	3.9	0.0022	0	0.00	0.6	11.9	586.42	
INLET Z5	3+66.67	12.04	0.70	10	0.70	10.0	100	9.80	6.9	18	3.9	0.0043	1.25	0.30	0.1	10.1	586.48	
3+66.67	3+53.64	13.03	-	-	2.73	11.9	100	9.45	25.8	36	3.6	0.0015	0.5	0.08	0.1	12.0	586.13	
INLET Z6	3+53.64	39.56	0.74	10	0.74	10.0	100	9.80	7.2	18	4.1	0.0047	1.25	0.32	0.2	10.2	586.54	
3+53.64	2+15.58	138.06	-	-	3.47	12.0	100	9.43	32.7	36	4.6	0.0024	0.3	0.27	0.5	12.5	586.03	
2+15.58	1+04.87	110.71	-	-	3.47	12.5	100	9.35	32.4	36	4.6	0.0024	0.5	0.16	0.4	12.9	585.43	
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LATERAL EX-B6																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET B6	1+67.38	80.07	0.82	10	0.82	10.0	100	9.80	8.0	18	4.5	0.0058	1.25	0.40	0.1	10.1	574.94	SEE SD LINE B IN PLANS FOR CORPORATE CROSSING BY WIER & ASSOCIATES DATED 05/17/12
STORM DRAIN CALCULATIONS FOR EXISTING STORM DRAIN LATERAL EX-A3																		
FROM	TO	LENGTH (FT)	CxA	INLET TIME (min.)	TOTAL INTERCEPTED CxA	TIME AT UPSTREAM OF REACH (min)	DESIGN STORM FREQUENCY (yrs)	RAINFALL INTENSITY (in/hr)	INTERCEPTED FLOW (cfs)	STORM DRAIN DIAMETER (in)	VELOCITY (ft/s)	SLOPE OF FRICTION GRADIENT (ft/ft)	STRUCTURE LOSS COEFFICIENT	STRUCTURE LOSS AT UPSTREAM OF REACH	FLOW TIME IN DRAIN (min)	TIME AT DOWNSTREAM OF REACH (min)	H.G. AT UPSTREAM OF REACH (ft)	REMARKS
INLET A3	1+83.83	70.48	0.67	10	0.67	10.0	100	9.80	6.6	21	2.7	0.0017	1.25	0.15	0.1	10.1	574.58	SEE SD LINE B IN PLANS FOR CORPORATE CROSSING BY WIER & ASSOCIATES DATED 05/17/12

RECORD
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02/02/2015

TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

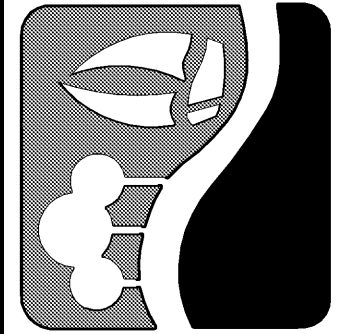
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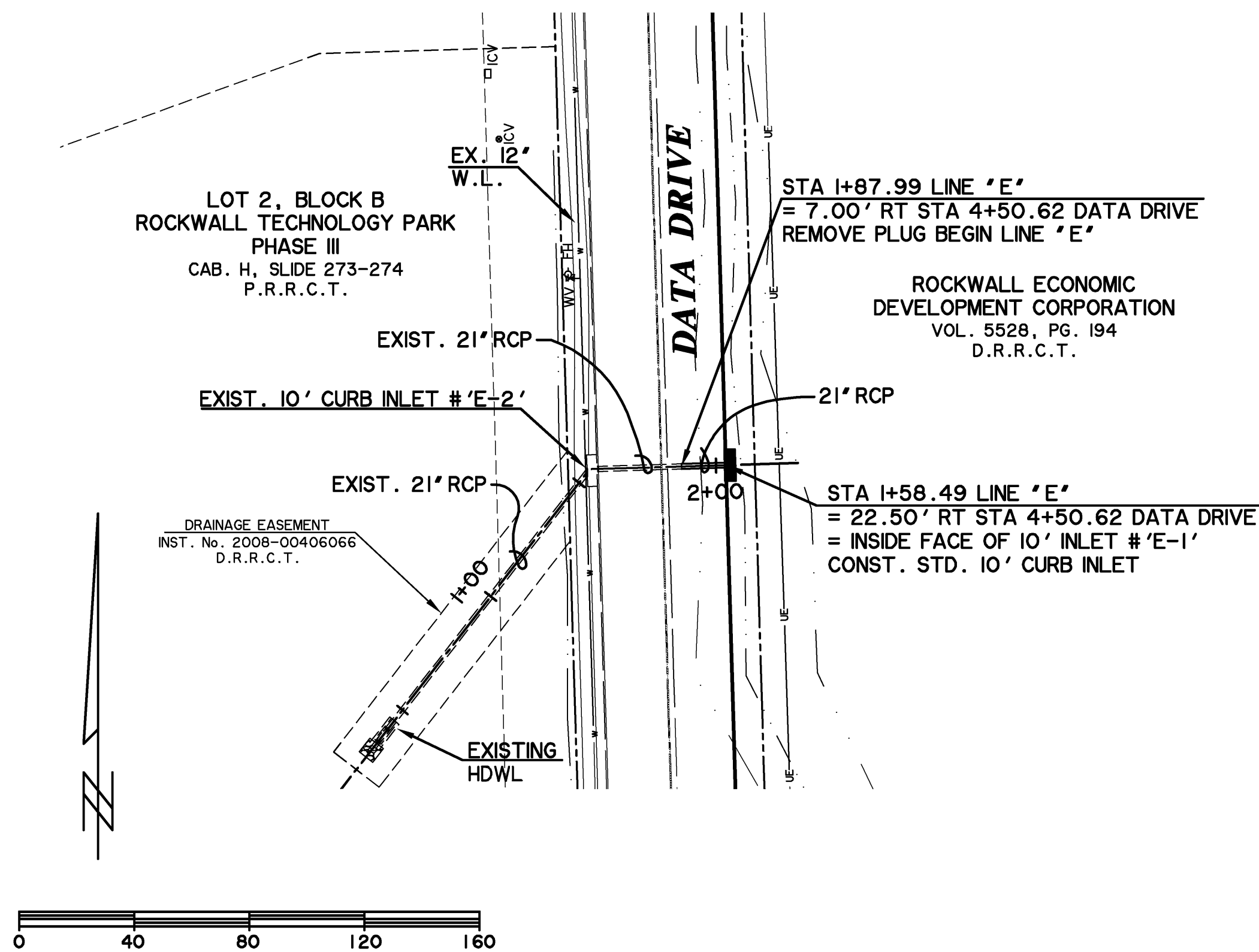
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CULVERT - LINE 'U' DESIGN CALCULATIONS																														
Culvert Location: Culvert - Line 'U'										RDWY. Elev. 599.50 U.S. Culv. F.L. 592.90 Difference 6.60 ft Req'd Freeboard 1.00 ft Allow. Headwater 5.60 ft										U.S. Culv. F.L. 592.5 D.S. Culv. F.L. 592.2 Difference 0.3 Length (ft) 67 Culv. Slope, $s_c = \frac{\text{Diff Ft}}{\text{Length Ft}}$ 0.45%										
Total Discharge, Q (cfs) 165.4 Roughness Coeff., n 0.015 Tailwater (ft) 2.0 Entrance Description: Type 2A, 90° Headwall										Design Storm Freq. 100 yr Max. Vel. (ft/s) 12.0 D.S. Channel Width (ft) 12.0																				
TRIAL CULVERT										HEADWATER CALCULATION																			The Greater Controlling Head Water (Inlet or Outlet) (feet)	Selected Conduit Size (feet)
Trial Area of Opening T*A=Q/V (sq. ft.)	Channel Width "W" (feet)	DEPTH RANGE		Trial Depth "D" (feet)	POSSIBLE CULVERT SIZES					INLET CONTROL					Entrance Coeff, Ke	OUTLET CONTROL														
		T*Ac/W (feet)	AHW (feet)		No. Openings	Width of Box "B" (feet)	Box Depth or Pipe Diameter "D" (feet)	Total Culvert Area "Ac" (sq.ft.)	"Q" Each Opening (c.f.s.)	Entrance Type	Case NO.	Q/B (c.f.s.)	HW/D	"HW" (feet)		CASE III				CASE IV										
																"H" (feet)	"TW" (feet)	LxSo (feet)	"HW" (feet)	"H" (feet)	ho=dc+D/2 or ho=TW (use larger)				LxSo	"HW" (feet)				
		dc (feet)	dc+D/2 (feet)	TW (feet)	ho (feet)																									
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29		
13.78	12.00	1.15	5.60	3.00	1.00	7.00	3.00	21.00	165.40	Type 2A	Case 2	23.63	1.60	4.80	0.50	1.80	2.00	0.30	3.50	1.80	2.60	2.80	2.00	2.80	0.30	4.30	4.80	7"x3' Box		

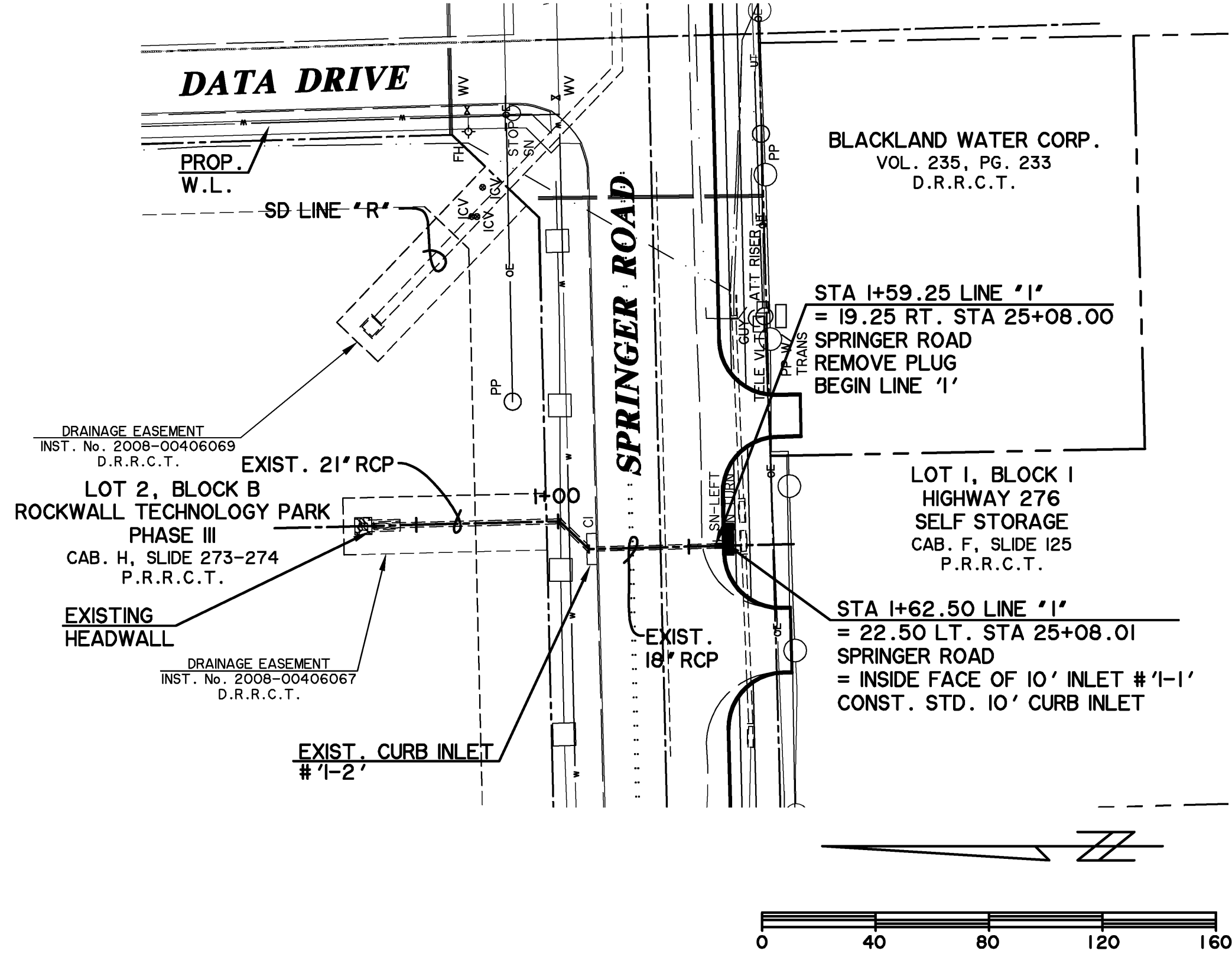
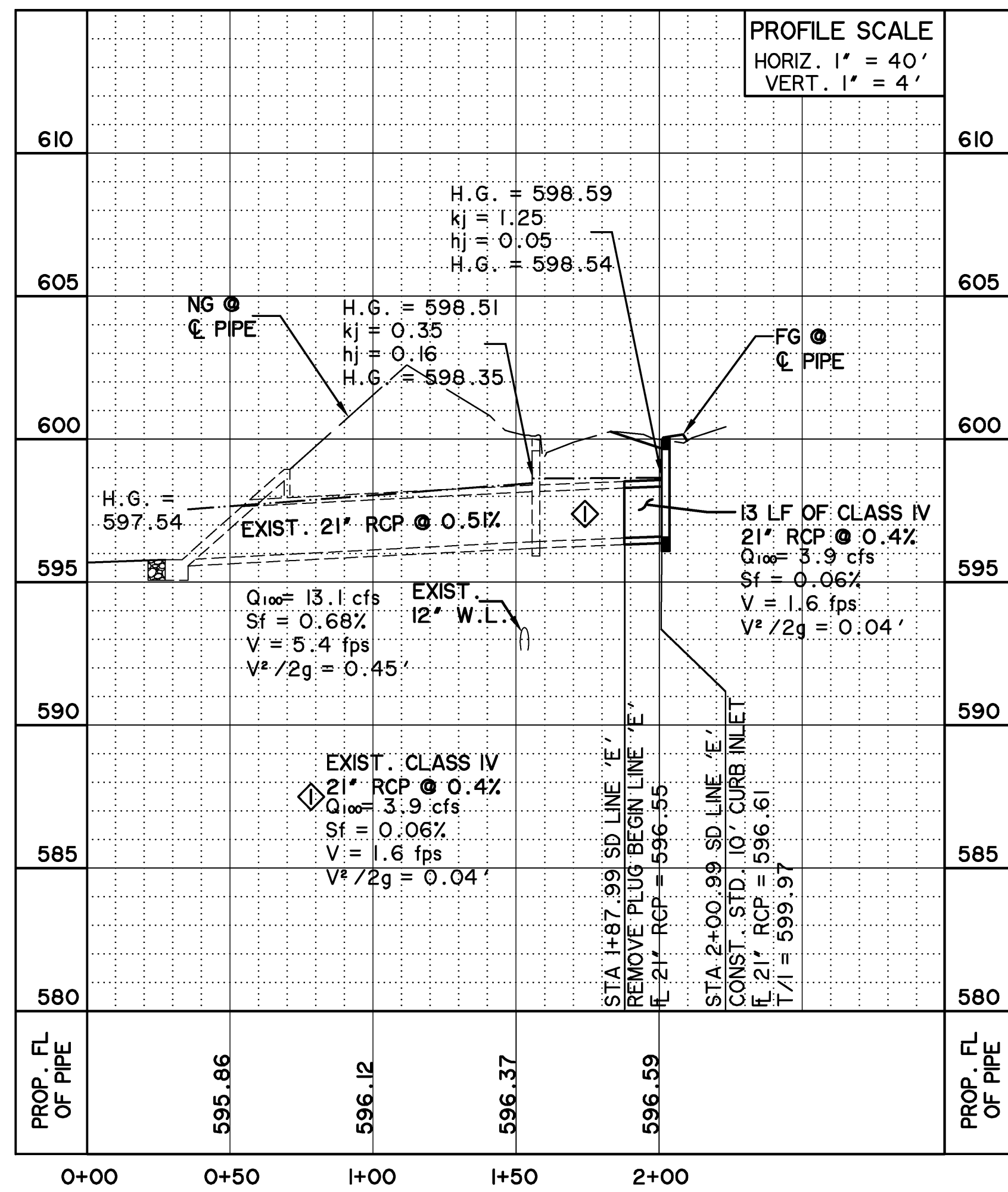
										CULVERT - LINE 'V' DESIGN CALCULATIONS																			
										Culvert Location: Culvert - Line 'V'					RDWY. Elev. 581.31		U.S. Culv. F.L. 575.3												
Total Discharge, Q (cfs) 199.6					Design Storm Freq. 100 yr					U.S. Culv. F.L. 575.30		D.S. Culv. F.L. 575																	
Roughness Coeff., n 0.015										Difference 6.01 ft		Difference 0.3																	
Tailwater (ft) 2.5					D.S. Channel Width (ft) 12.0					Req'd Freeboard 1.00 ft		Length (ft) 67																	
Entrance Description: Type 2A, 90° Headwall										Allow. Headwater 5.01 ft		Culv. Slope, $s_c = \frac{\text{Diff Ft}}{\text{Length Ft}}$ 0.45%																	
TRIAL CULVERT										HEADWATER CALCULATION										The Greater Controlling Head Water (Inlet or Outlet) (feet)	Selected Conduit Size (feet)								
Trial Area of Opening T*A=Q/V (sq. ft.)	Channel Width "W" (feet)	DEPTH RANGE		Trial Depth "D" (feet)	POSSIBLE CULVERT SIZES					INLET CONTROL					OUTLET CONTROL														
		T*Ac/W (feet)	AHW (feet)		No. Openings	Width of Box "B" (feet)	Box Depth or Pipe Diameter "D" (feet)	Total Culvert Area "Ac" (sq.ft.)	"Q" Each Opening (c.f.s.)	Entrance Type	Case NO.	Q/B (c.f.s.)	HW/D	"HW" (feet)	Entrance Coeff, Ke	CASE III						CASE IV							
																"H" (feet)	"TW" (feet)	LxSo (feet)	"HW" (feet)			"H" (feet)	ho=dc+D/2 or ho=TW (use larger)				LxSo	"HW" (feet)	
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27			28
16.63	12.00	1.39	5.01	3.00	2.00	5.00	3.00	30.00	99.80	Type 2A	Case 2	19.96	1.36	4.08	0.50	1.20	2.53	0.30	3.43	1.20	2.30	2.65	2.53	2.65	0.30	3.55	4.08	(2) 5'x3' Box	

										CULVERT - LINE 'W' DESIGN CALCULATIONS																										
										Culvert Location: Culvert - Line 'W'					RDWY. Elev. 589.05 U.S. Culv. F.L. 582.30 Total Discharge, Q (cfs) 369.6 Roughness Coeff., n 0.015 Tailwater (ft) 2.6 Entrance Description: Type 2A, 90° Headwall															Design Storm Freq. 100 yr Max. Vel. (ft/s) 12.0 D.S. Channel Width (ft) 12.0 Differece 6.75 ft Req'd Freeboard 1.00 ft Allow. Headwater 5.75 ft U.S. Culv. F.L. 582.3 D.S. Culv. F.L. 582 Difference 0.3 Length (ft) 67 Culv. Slope, $s_c = \frac{\text{Diff Ft}}{\text{Length Ft}}$ 0.45%						
TRIAL CULVERT																									HEADWATER CALCULATION										The Greater Controlling Head Water (Inlet or Outlet) (feet)	Selected Conduit Size (feet)
Trial Area of Opening T*A=Q/V (sq. ft.)	Channel Width "W" (feet)	DEPTH RANGE		Trial Depth "D" (feet)	POSSIBLE CULVERT SIZES					INLET CONTROL					Entrance Coeff, Ke	OUTLET CONTROL																				
		T*Ac/W (feet)	AHW (feet)		No. Openings	Width of Box "B" (feet)	Box Depth or Pipe Diameter "D" (feet)	Total Culvert Area "Ac" (sq.ft.)	"Q" Each Opening (c.f.s.)	Entrance Type	Case NO.	Q/B (c.f.s.)	HW/D	"HW" (feet)		CASE III				CASE IV																
															"H" (feet)	"TW" (feet)	LxSo (feet)	"HW" (feet)	"H" (feet)	ho=dc+D/2 or ho=TW (use larger)				LxSo	"HW" (feet)											
																				dc (feet)	dc+D/2 (feet)	TW (feet)	ho (feet)													
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29								
30.80	12.00	2.57	5.75	3.00	2.00	6.00	4.00	48.00	184.80	Type 2A	Case 2	30.80	1.38	5.52	0.50	1.50	2.59	0.30	3.79	1.50	3.20	3.60	2.59	3.60	0.30	4.80	5.52	(2) 6'x4' Box								

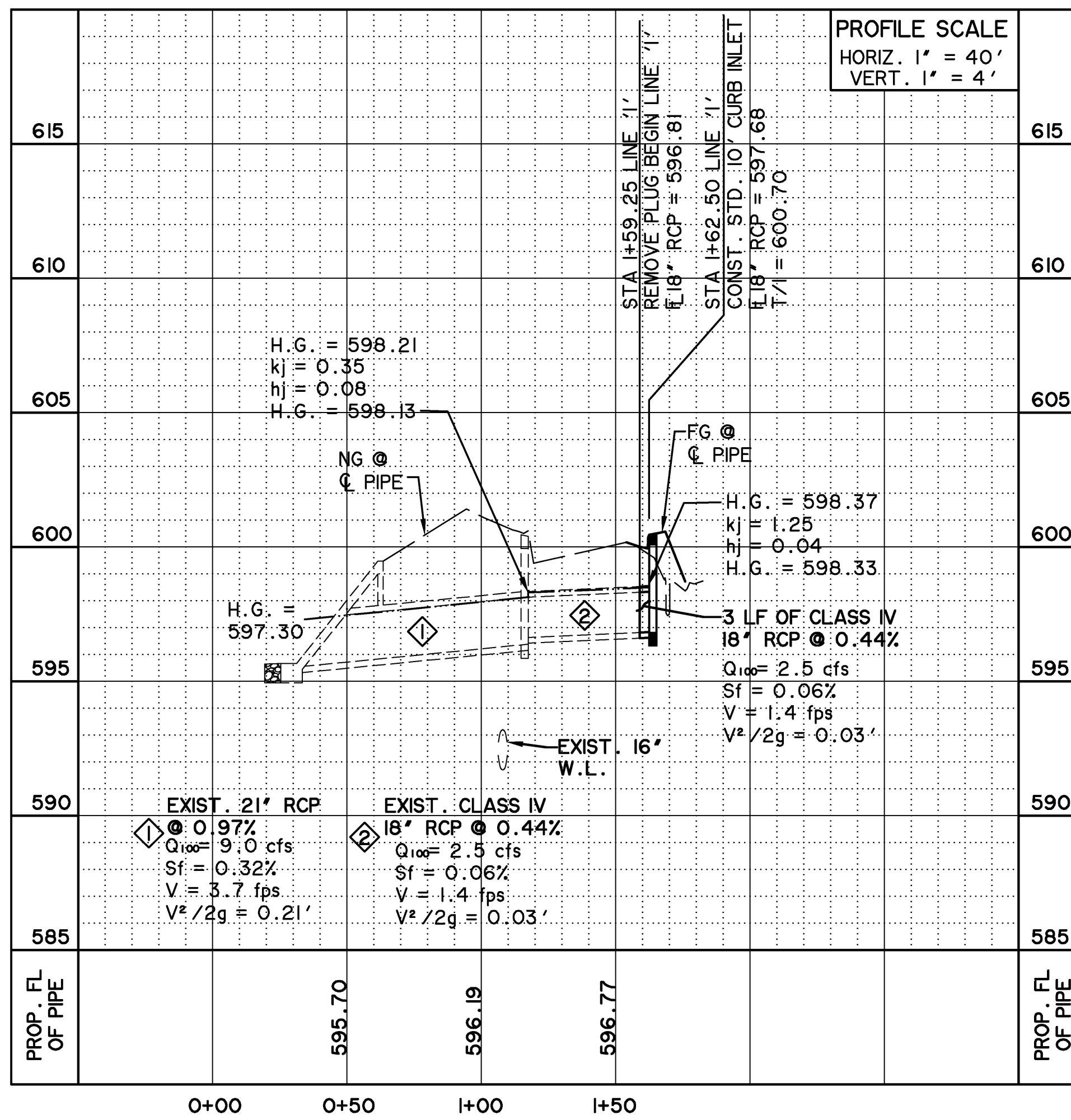
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STORM DRAIN LINE 'E'



STORM DRAIN LINE 'I'



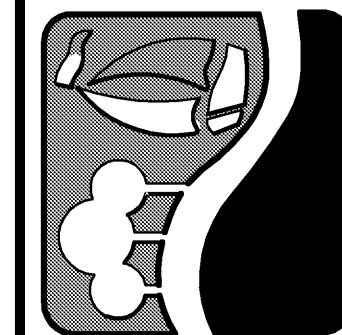
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* BENCH MARKS *	
BM A - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549.	598.80 FT.
BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	599.82 FT.
BM C - AN 'X' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549.	598.20 FT.

* ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OF DESIGN.*

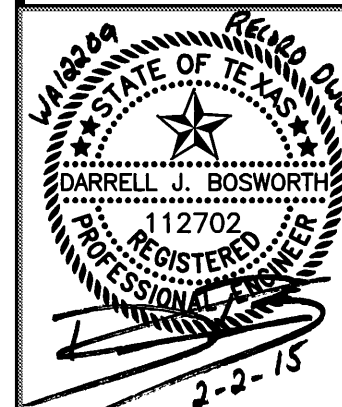
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
PLAN AND PROFILE
LINE 'E' & 'I'**

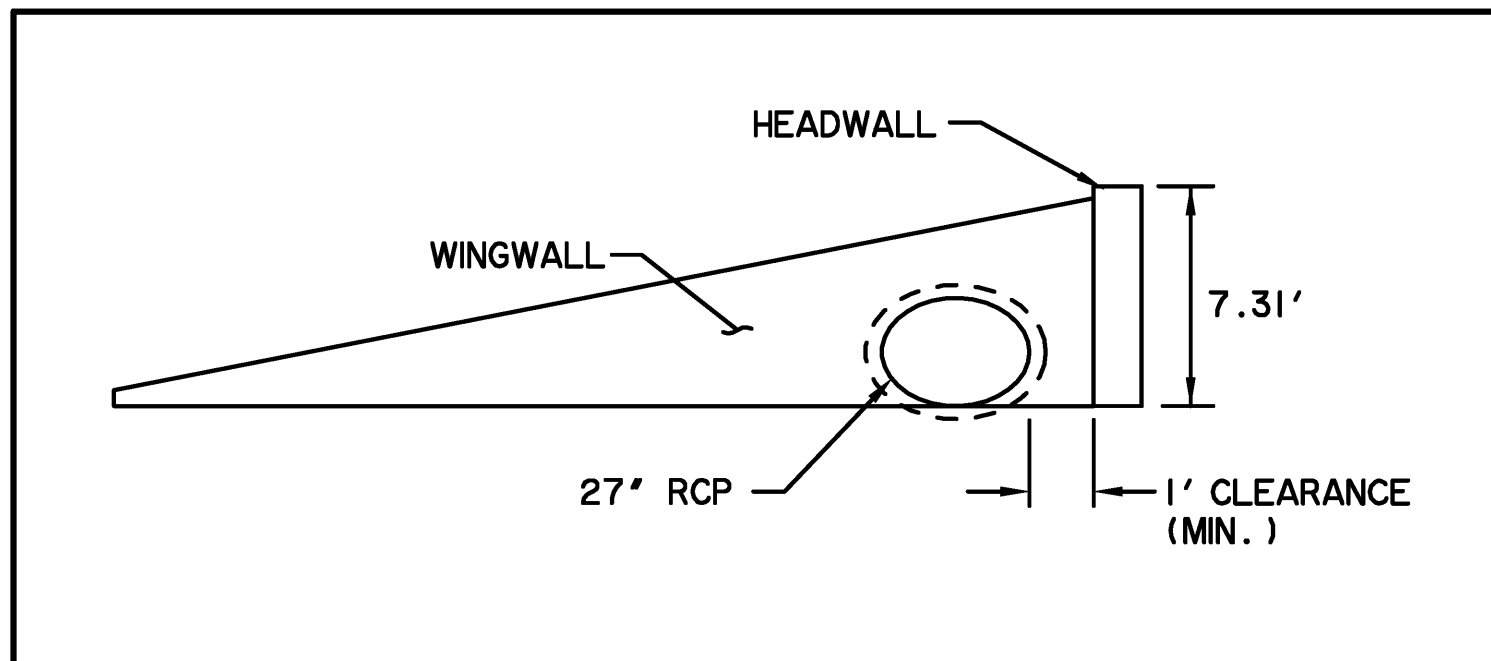
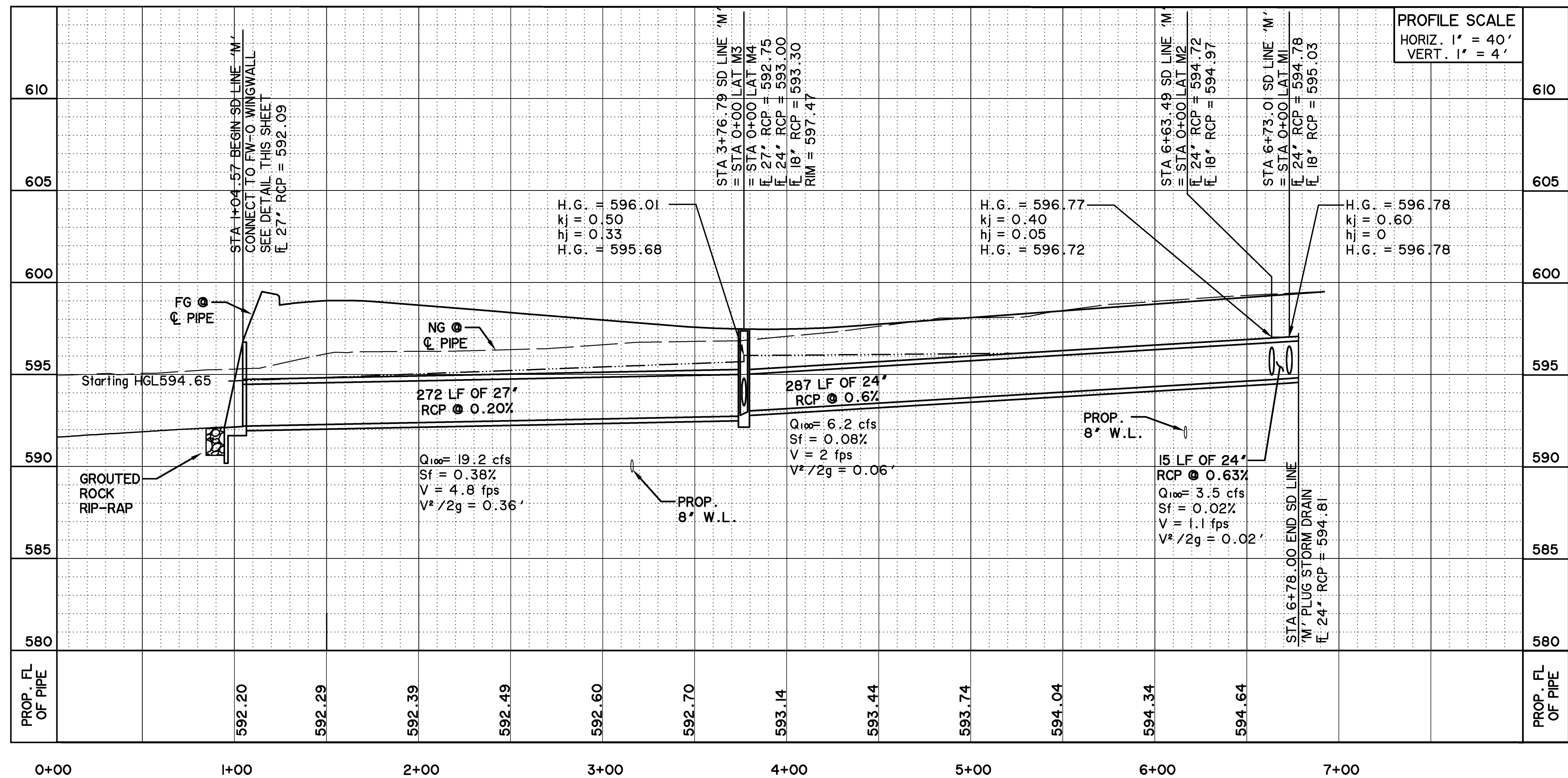
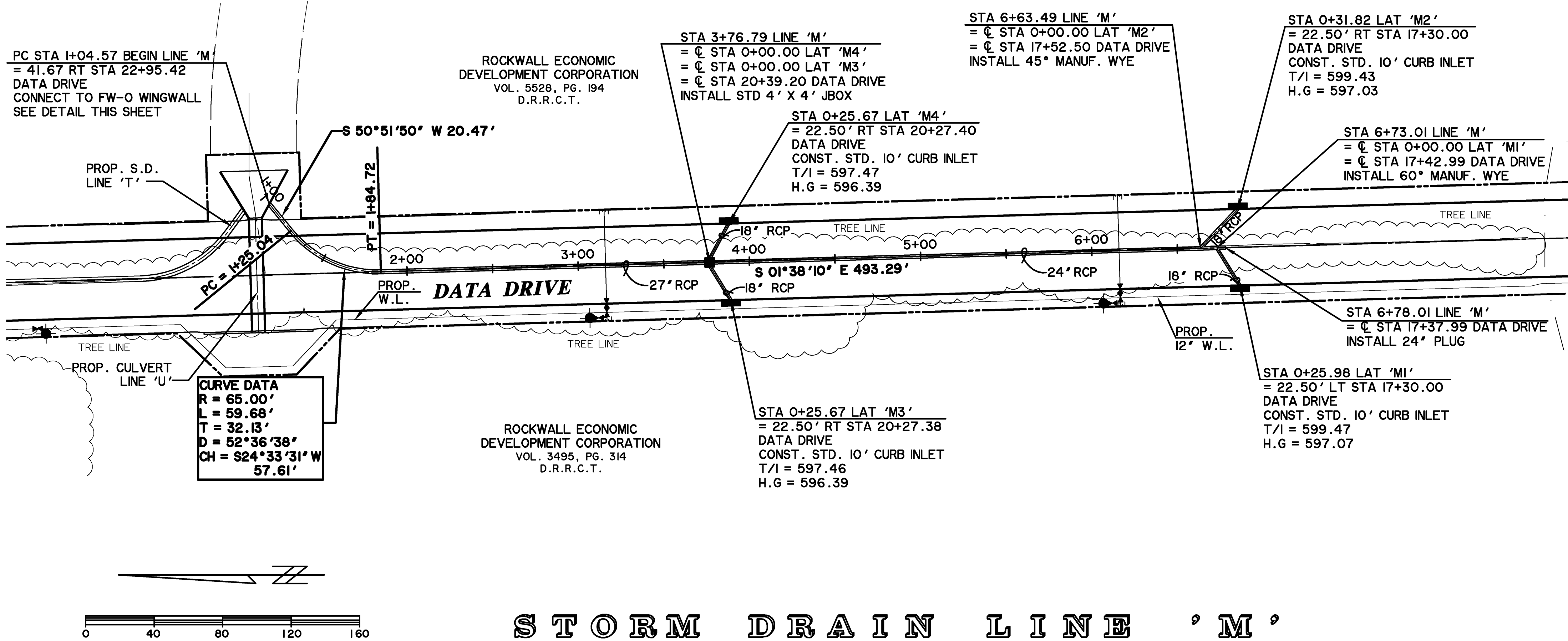


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**SHEET NO.
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701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
www.WierAssociates.com
Texas Firm Registration No. F-2776

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WINGWALL CONNECTION DETAIL
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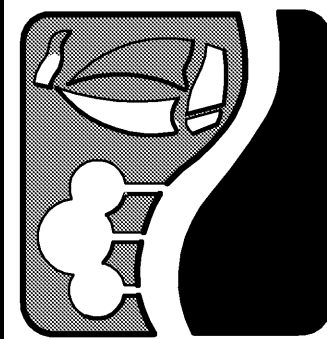
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549. 598.80 FT.

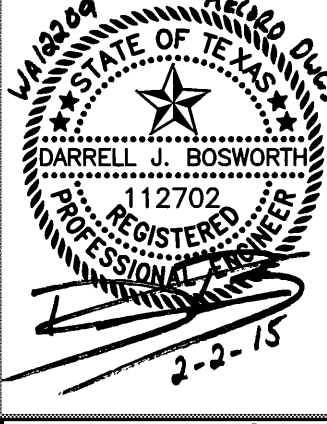
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LINE OF DISCOVERY BOULEVARD ±580' EAST
OF THE INTERSECTION OF DISCOVERY
BOULEVARD AND F.M. 549. 599.82 FT.

BM C - AN 'X' CUT IN DISCOVERY
BOULEVARD IN A MEDIAN NOSE ±60' WEST OF
THE INTERSECTION OF DISCOVERY BOULEVARD
AND F.M. 549. 598.20 FT.



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

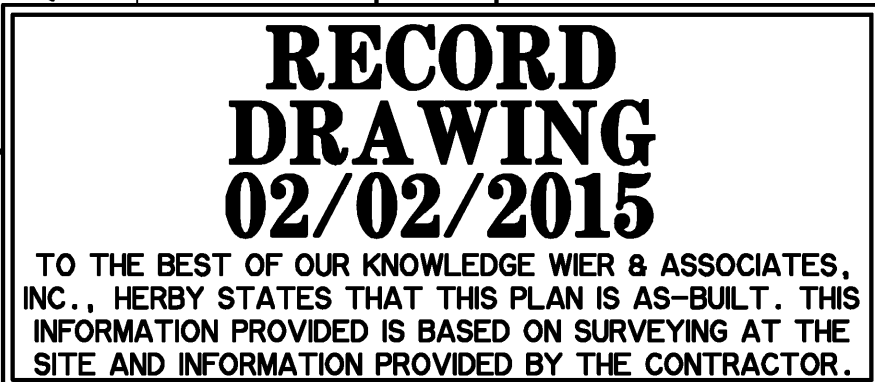
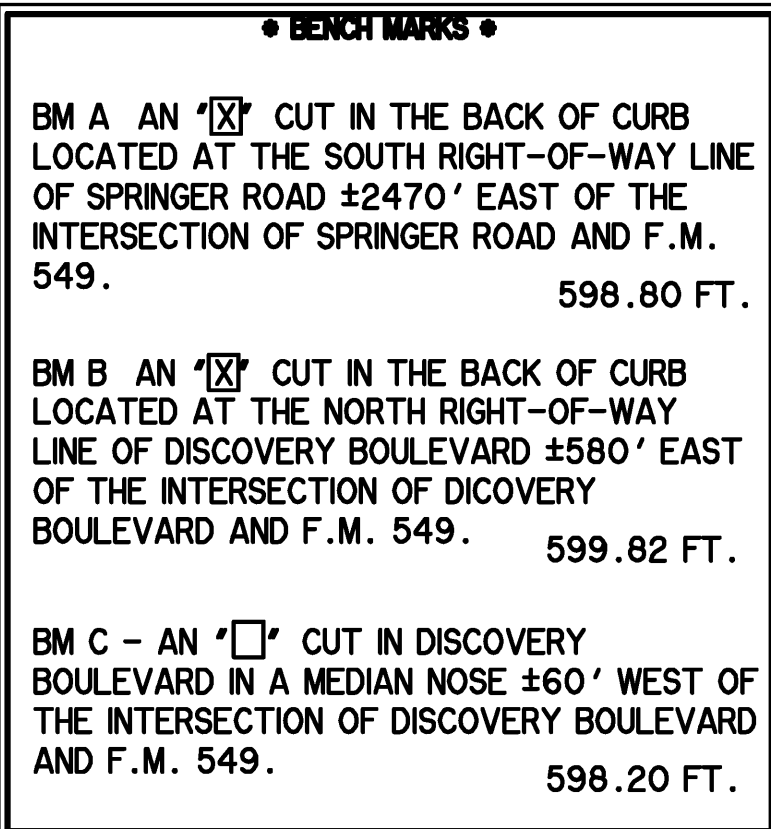
**STORM DRAIN
PLAN AND PROFILE
LINE 'M'**



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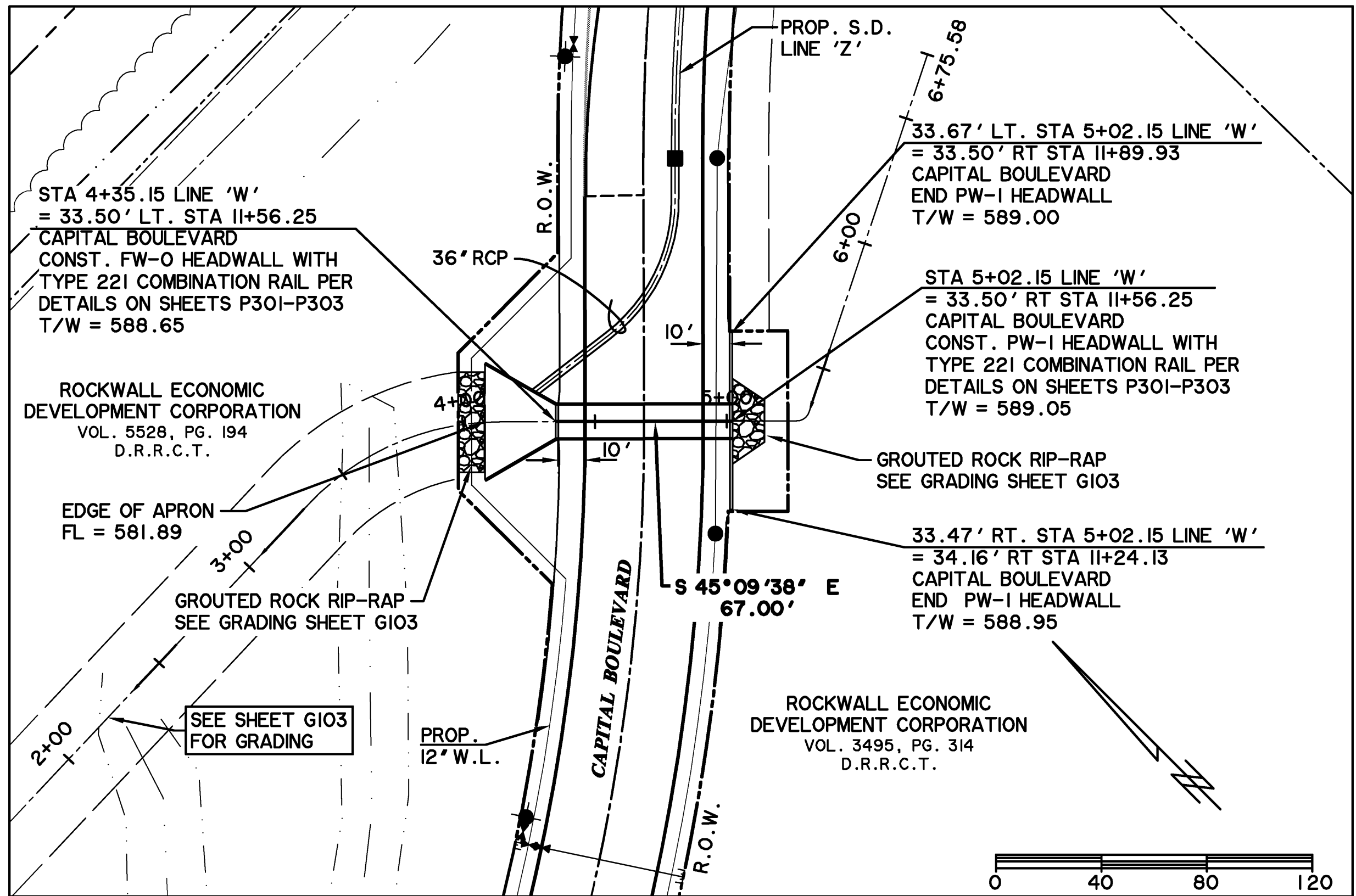
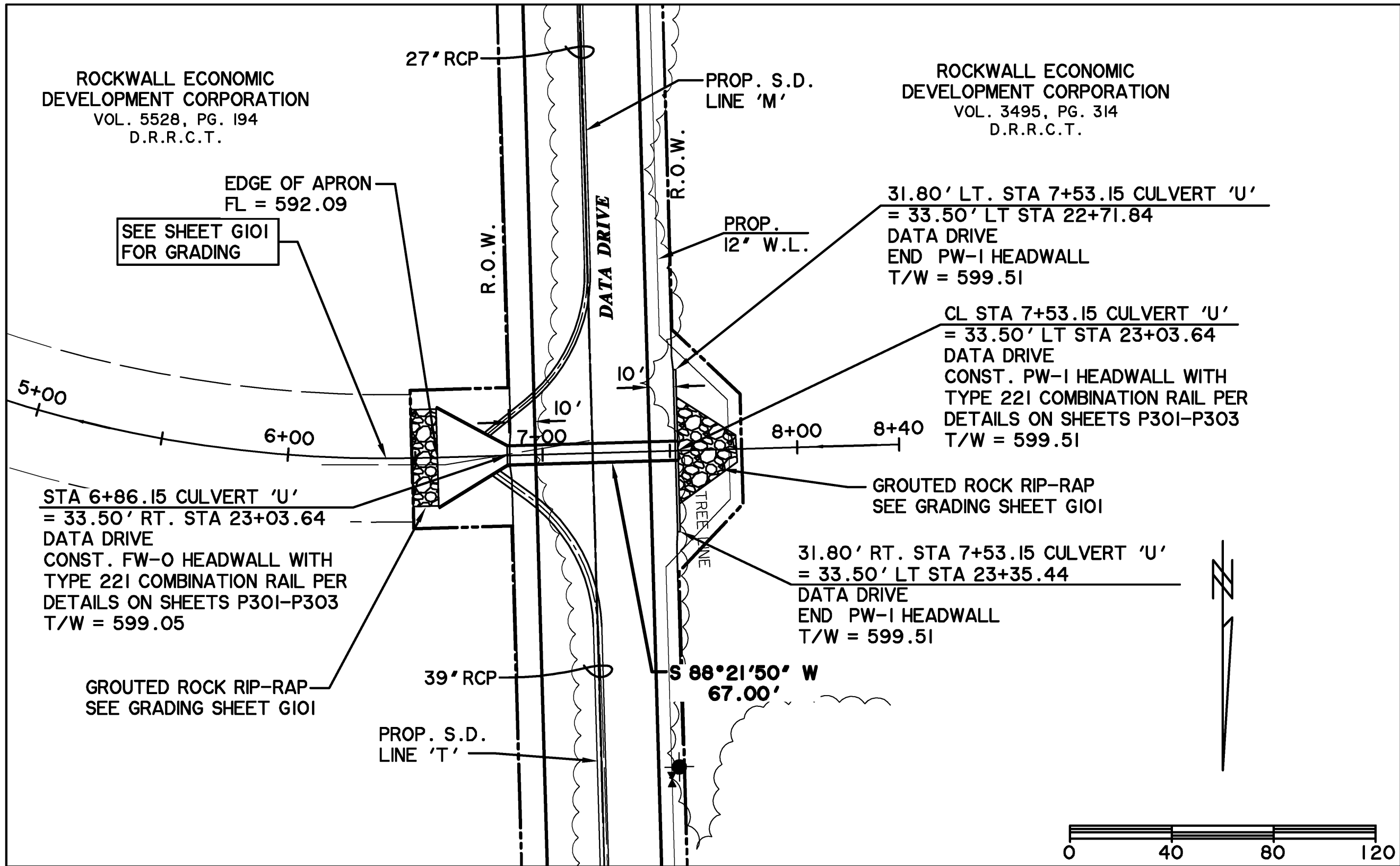
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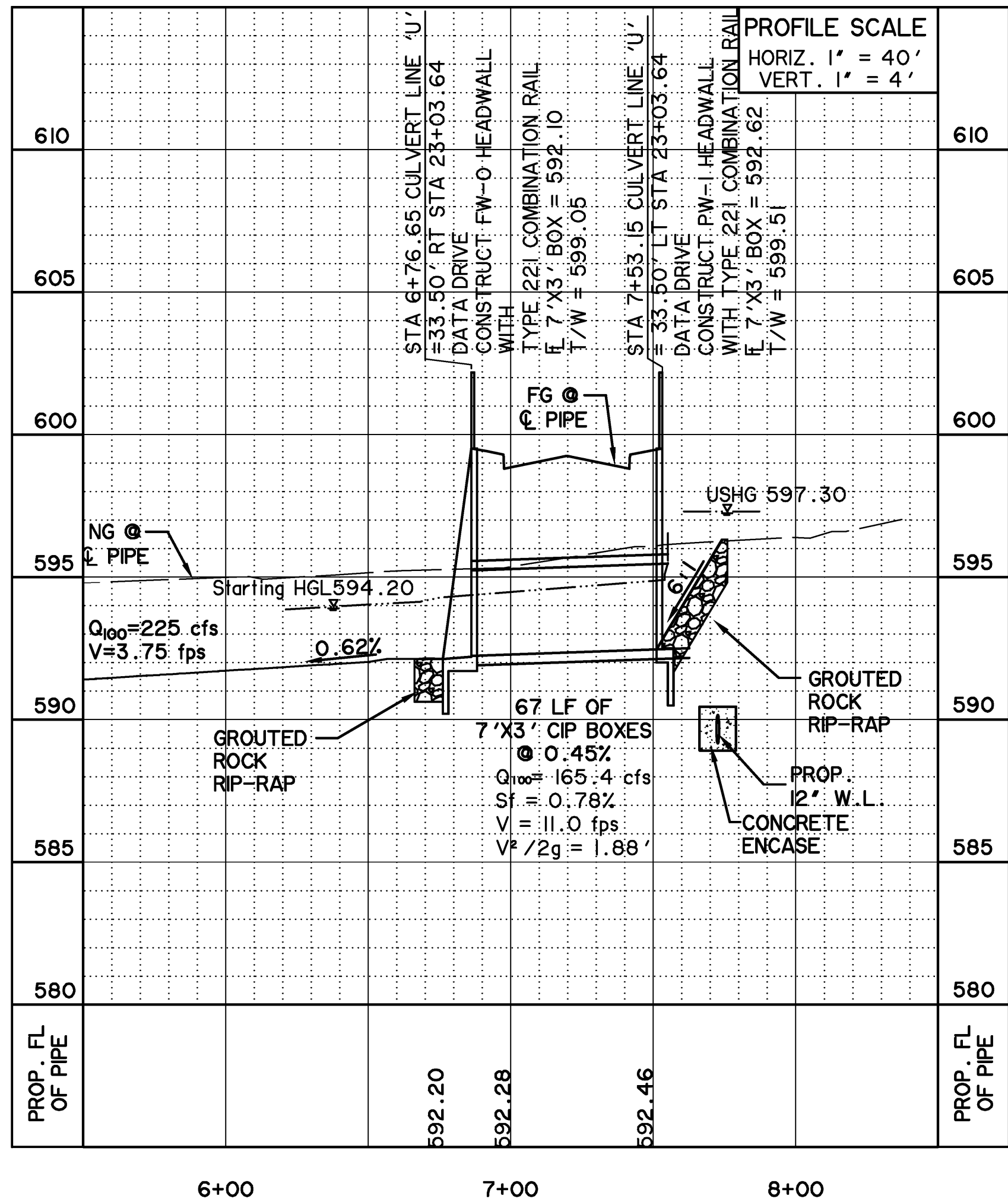


STORM DRAIN PLAN AND PROFILE LINE 'T'

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CULVERT LINE 'U'



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*** BENCH MARKS ***

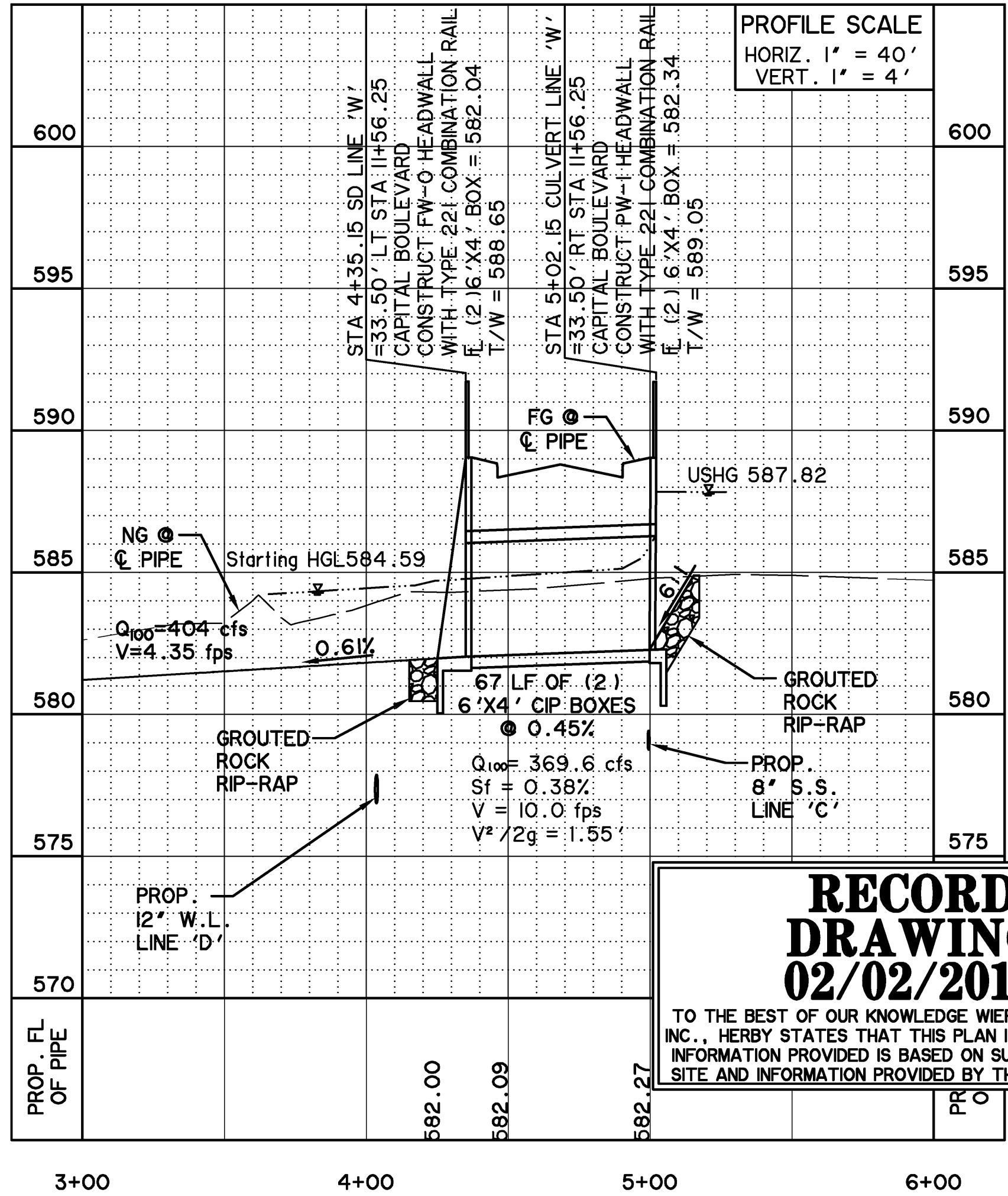
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BM C - AN "□" CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

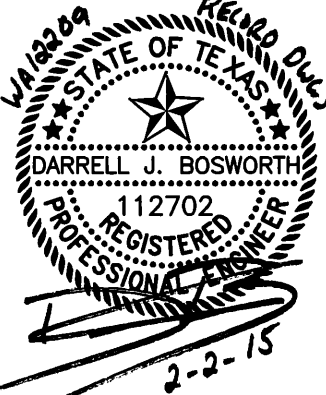
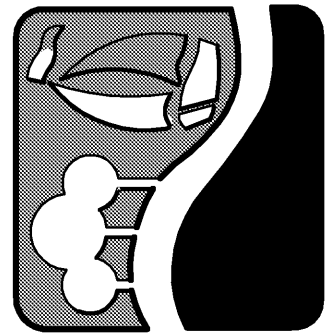
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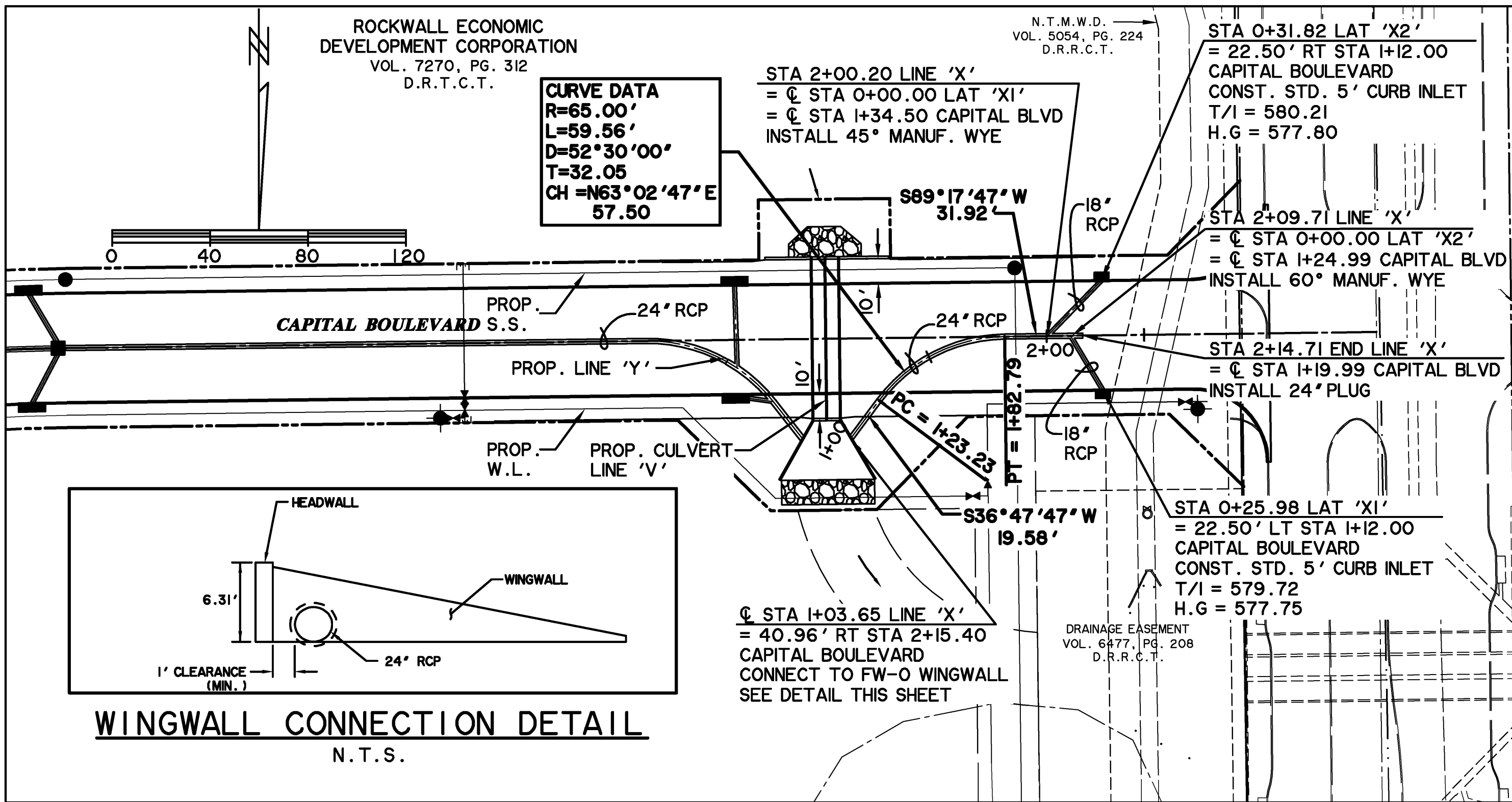
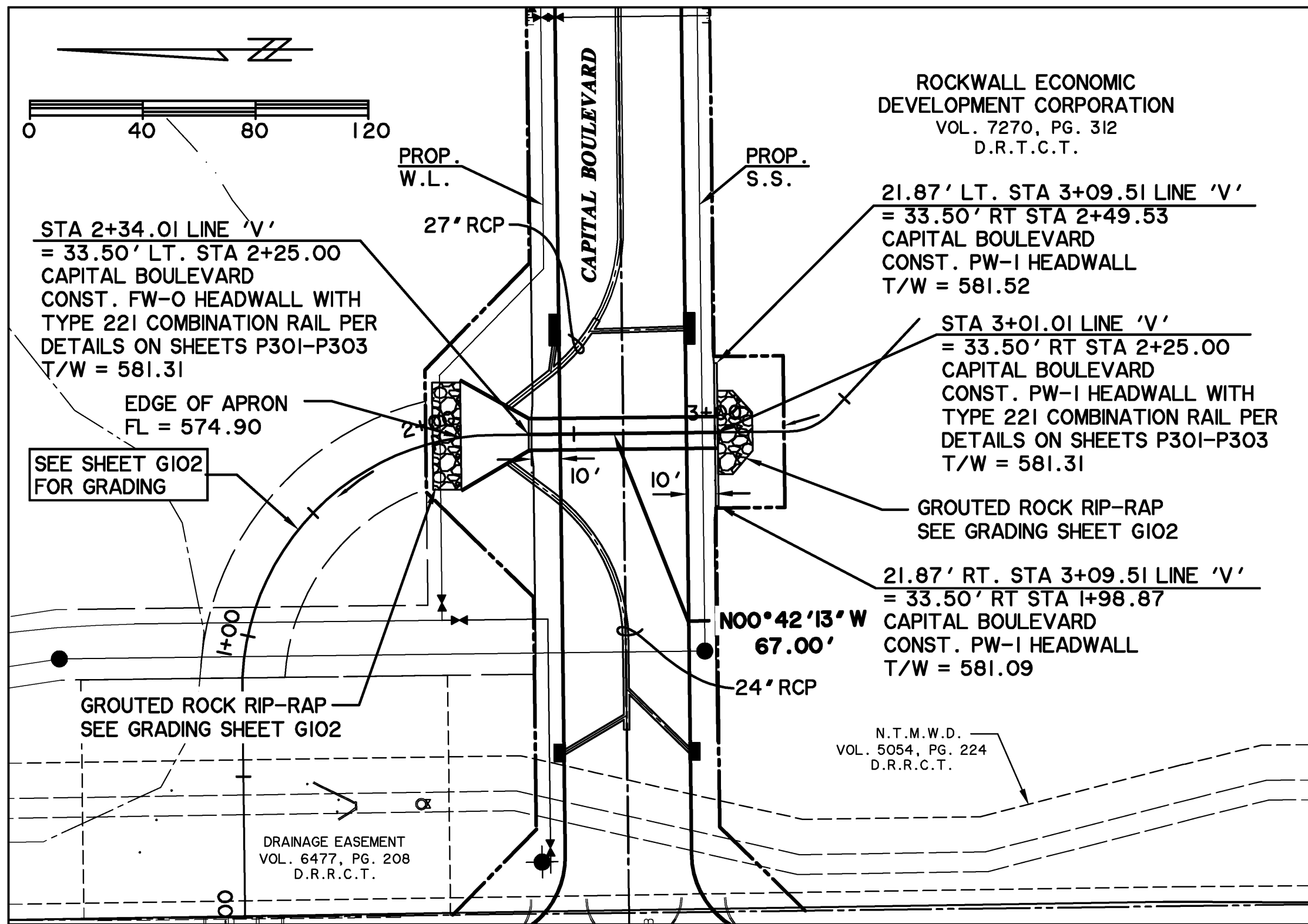
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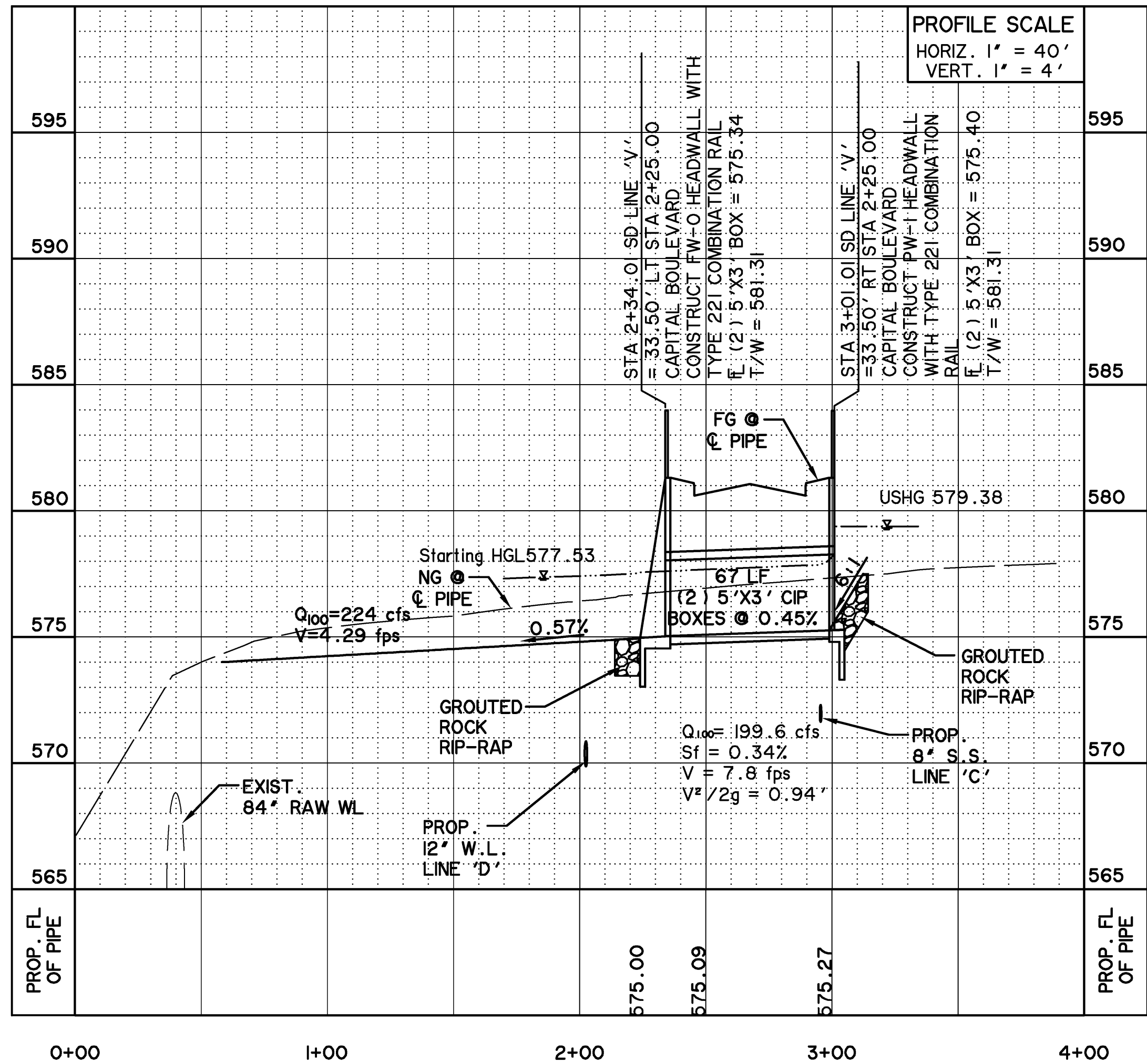
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CULVERT LINE 'V'

STORM DRAIN LINE 'X'



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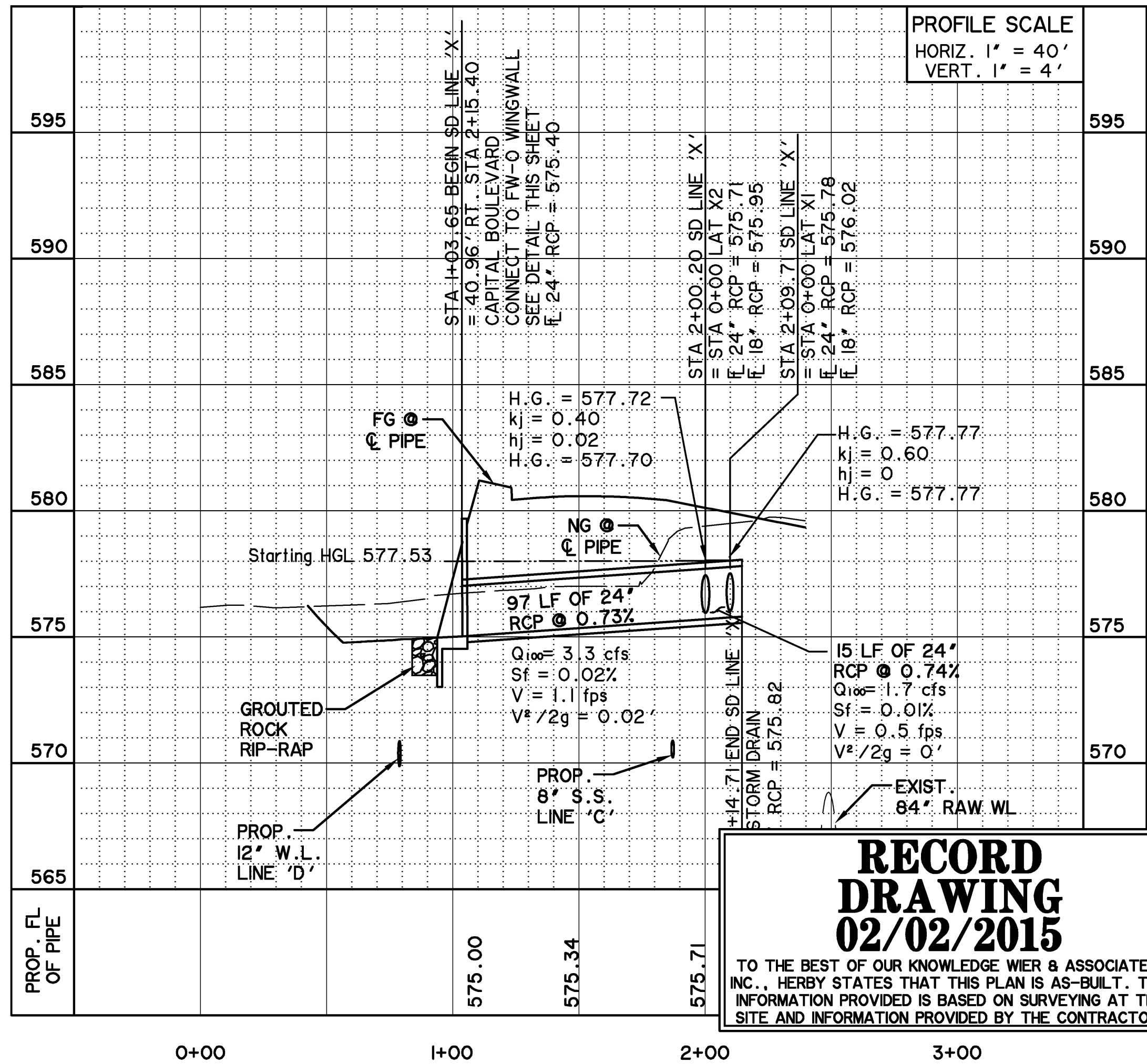
*** BENCH MARKS ***

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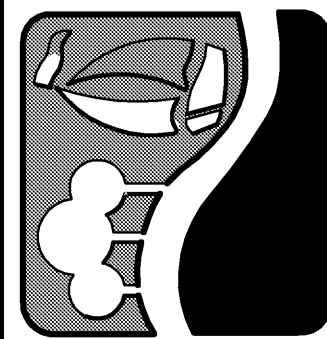
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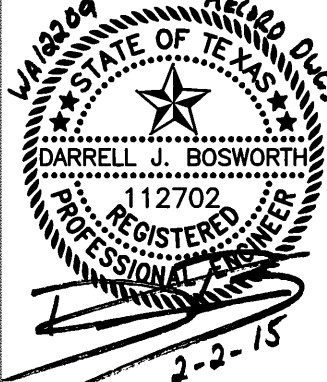
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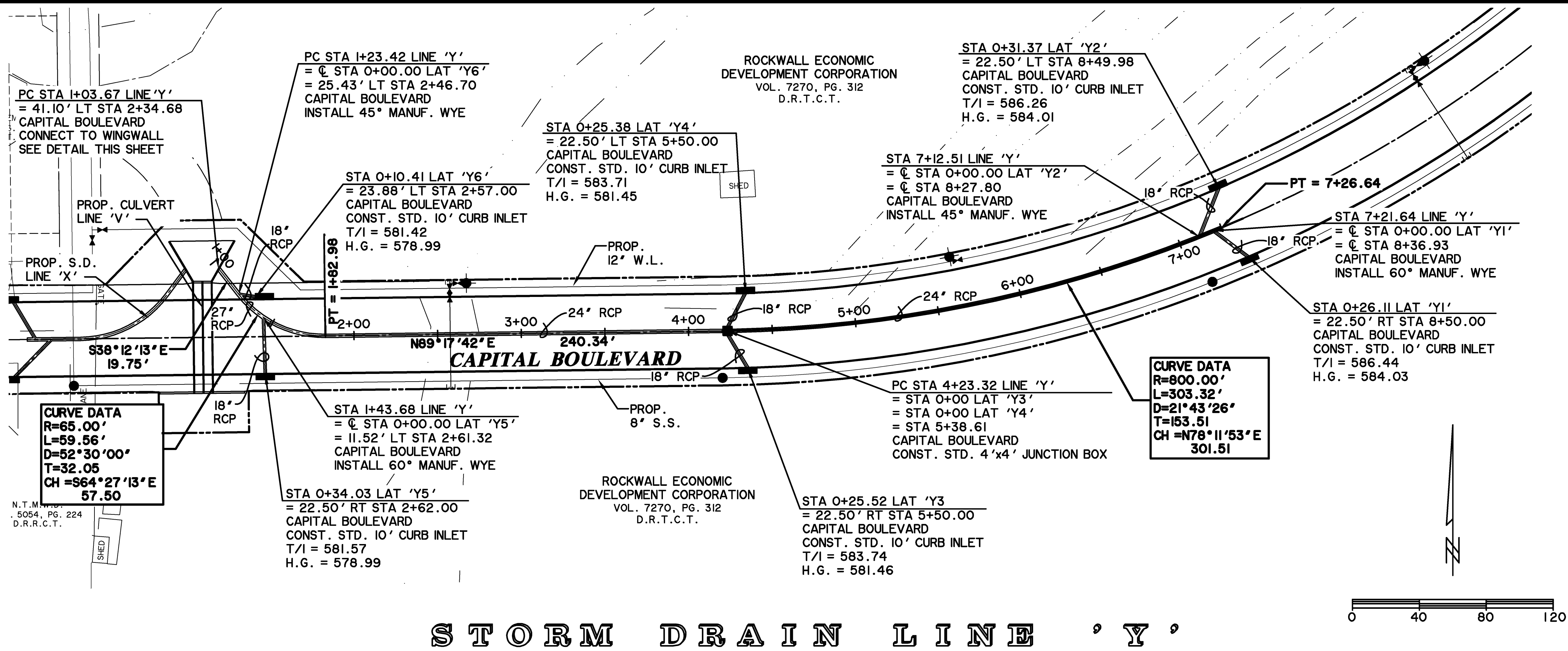


ROCKWALL TECHNOLOGY PARK
PHASE IV

STORM DRAIN PLAN AND PROFILE LINES 'V' & 'X'



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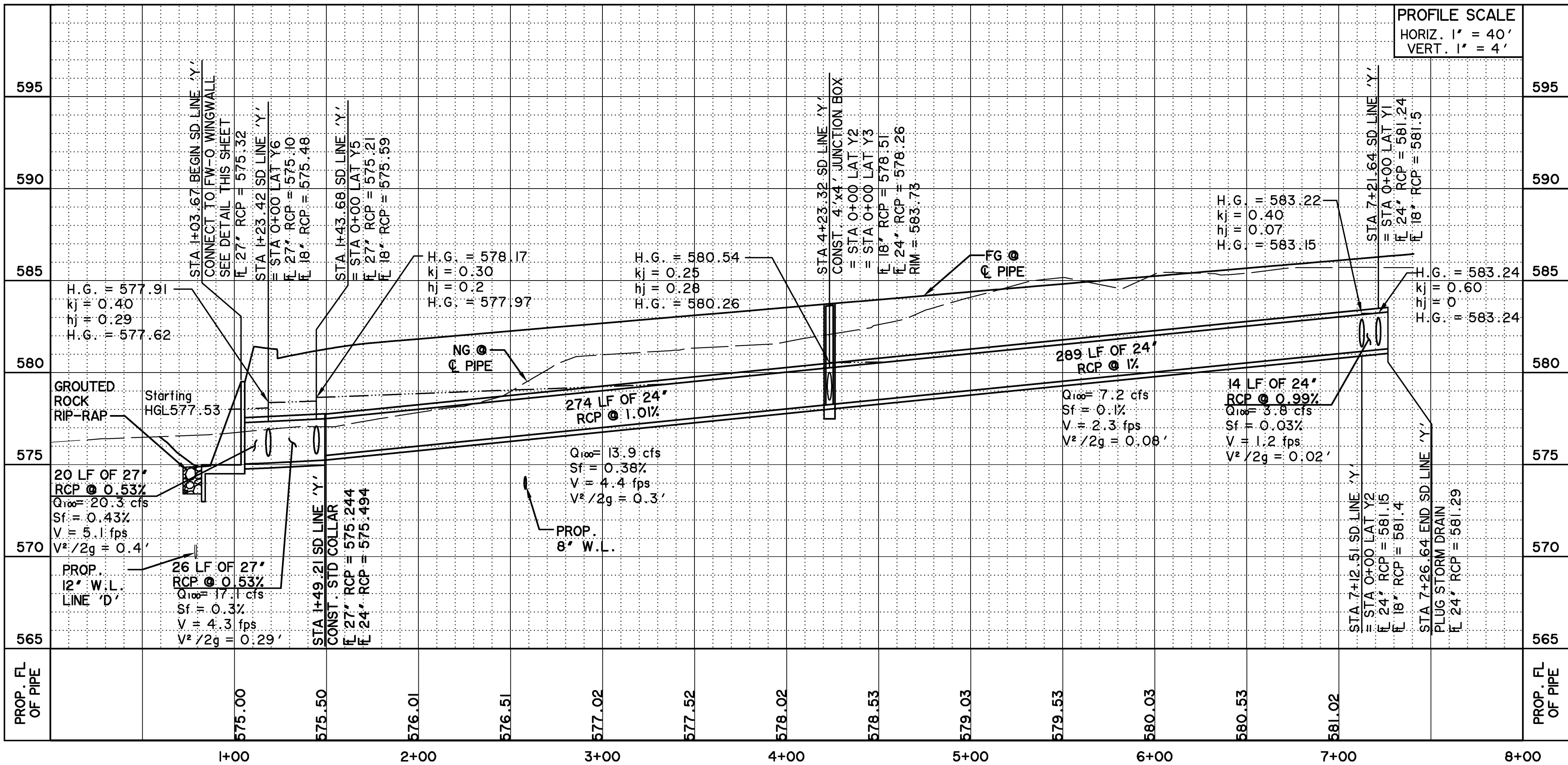
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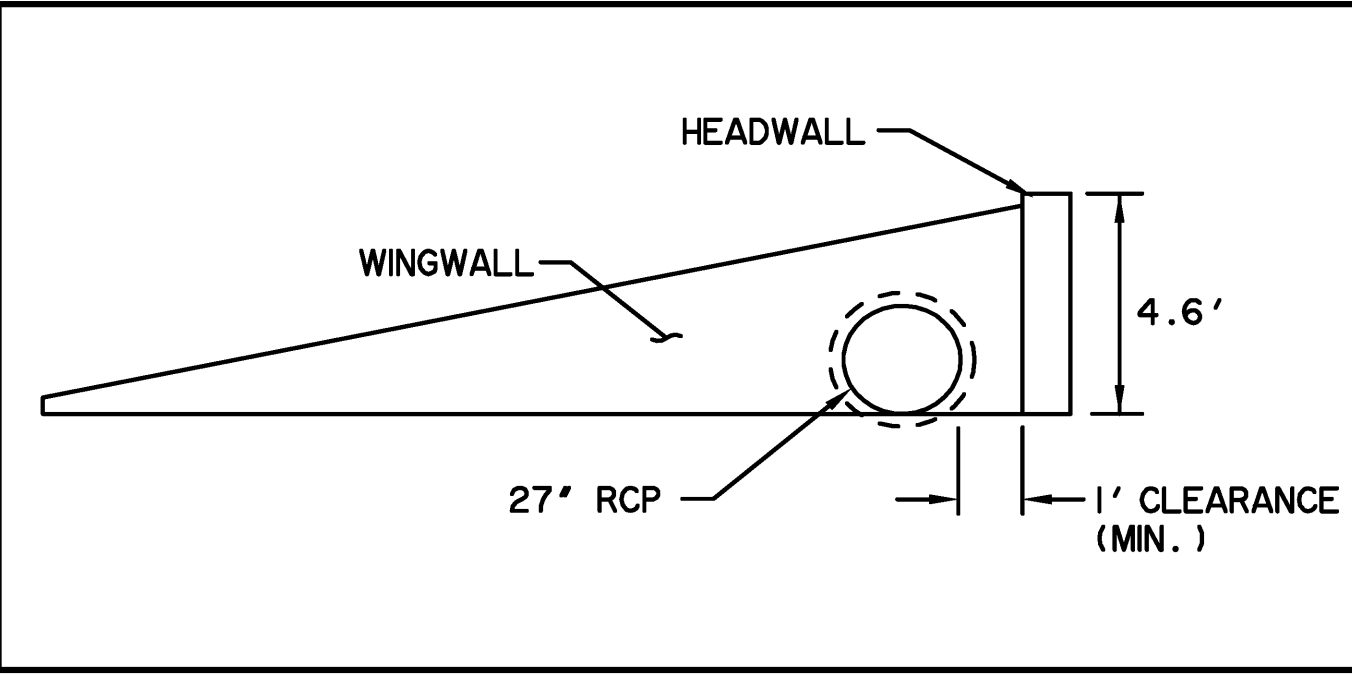
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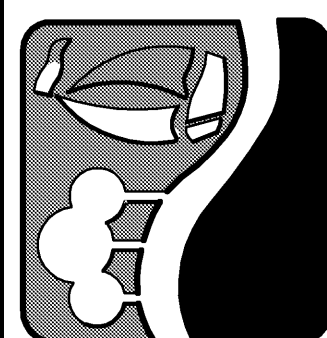


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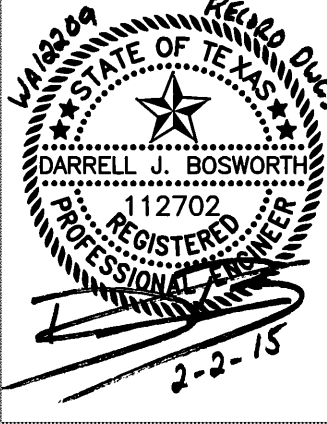
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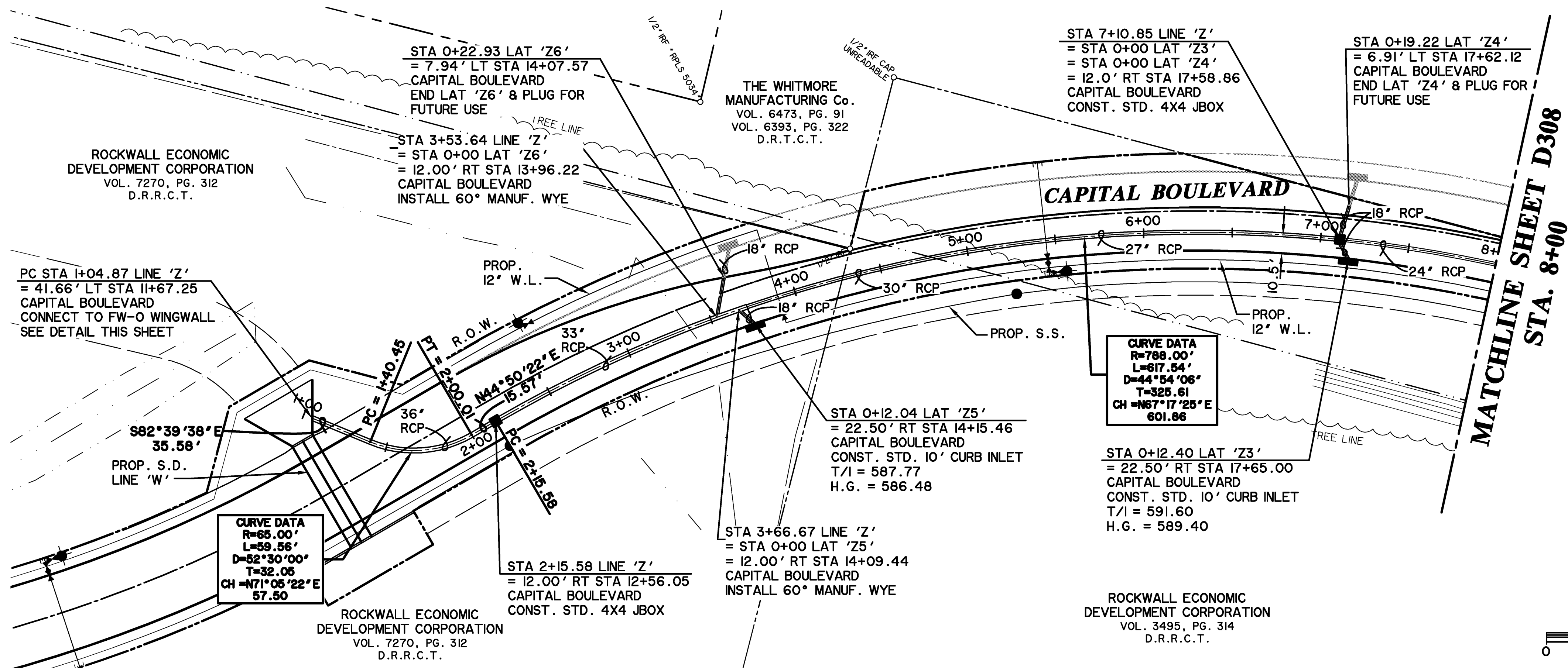


**ROCKWALL
TECHNOLOGY
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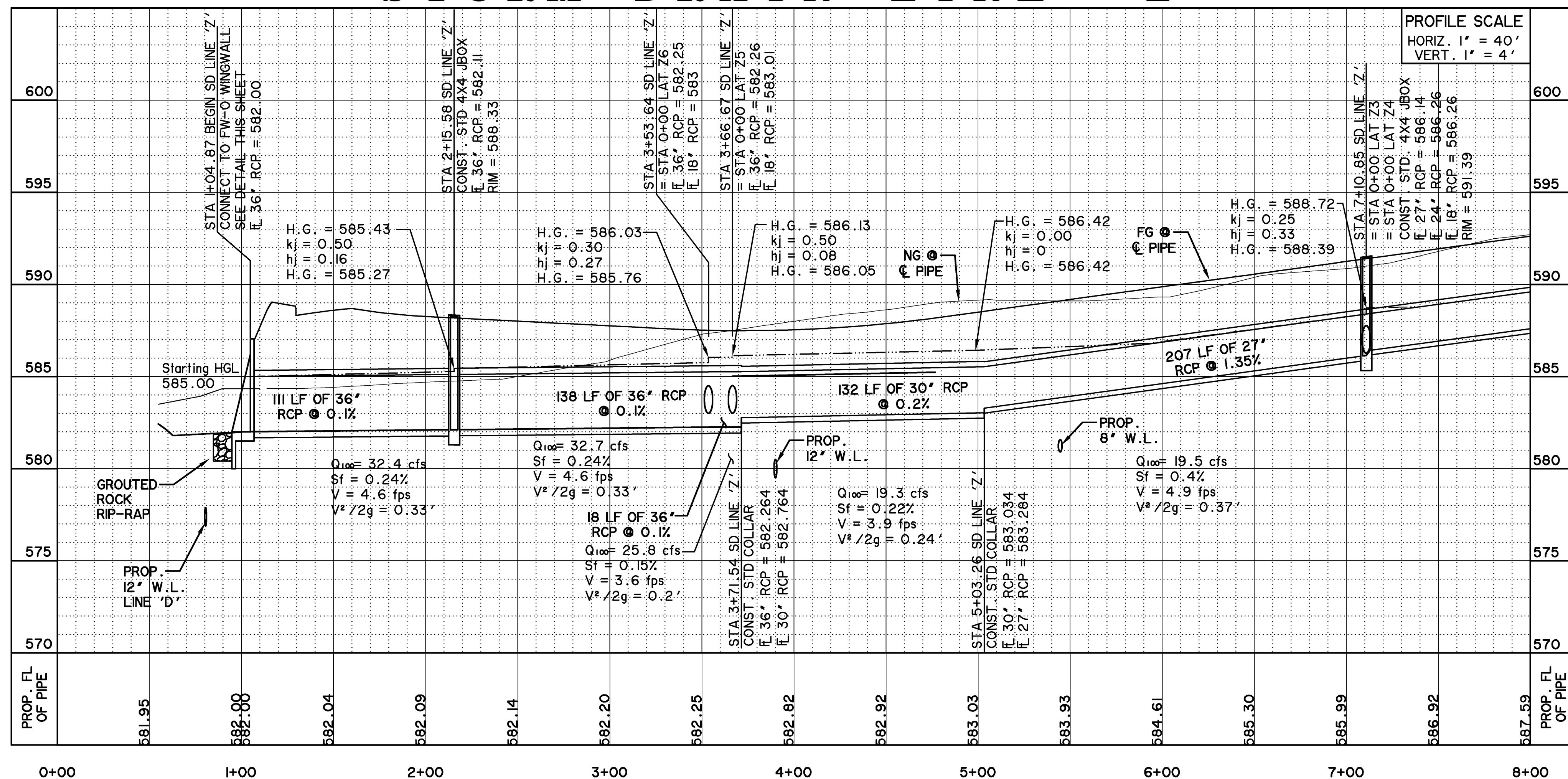
**STORM DRAIN
PLAN AND PROFILE
LINE 'Y'**



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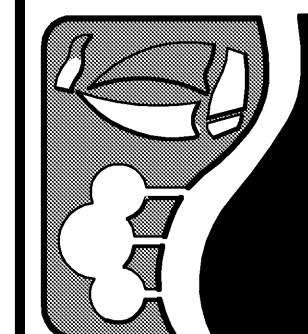


STORM DRAIN LINE 'Z'



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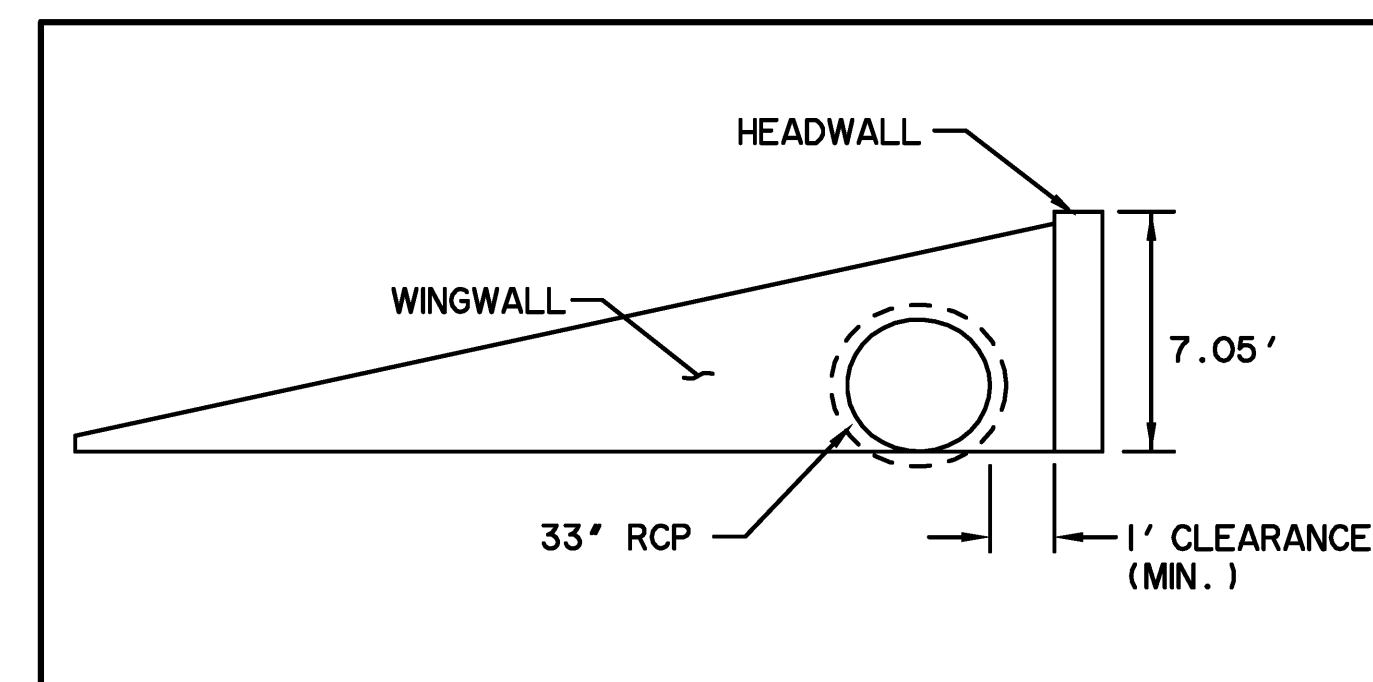
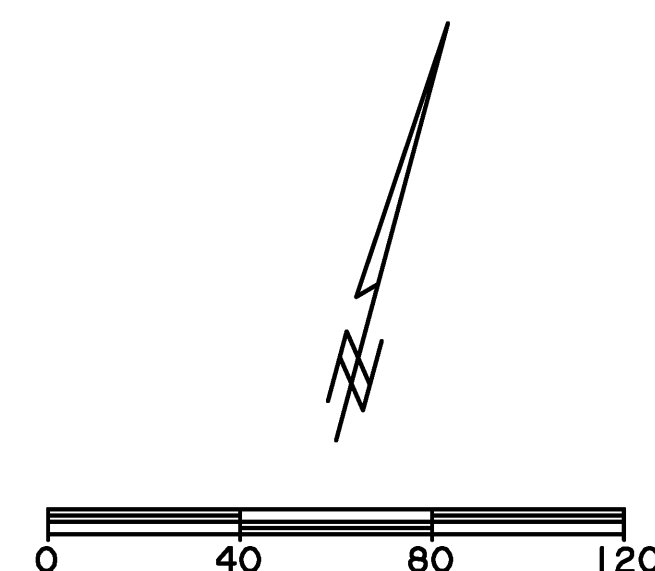


• BENCH MARKS •

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LOCATED AT THE NORTH RIGHT-OF-WAY
LINE OF DISCOVERY BOULEVARD $\pm 580'$ EAST
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BOULEVARD AND F.M. 549. 599.82 FT.

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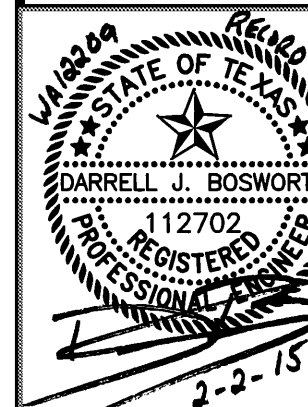


WINGWALL CONNECTION DETAIL
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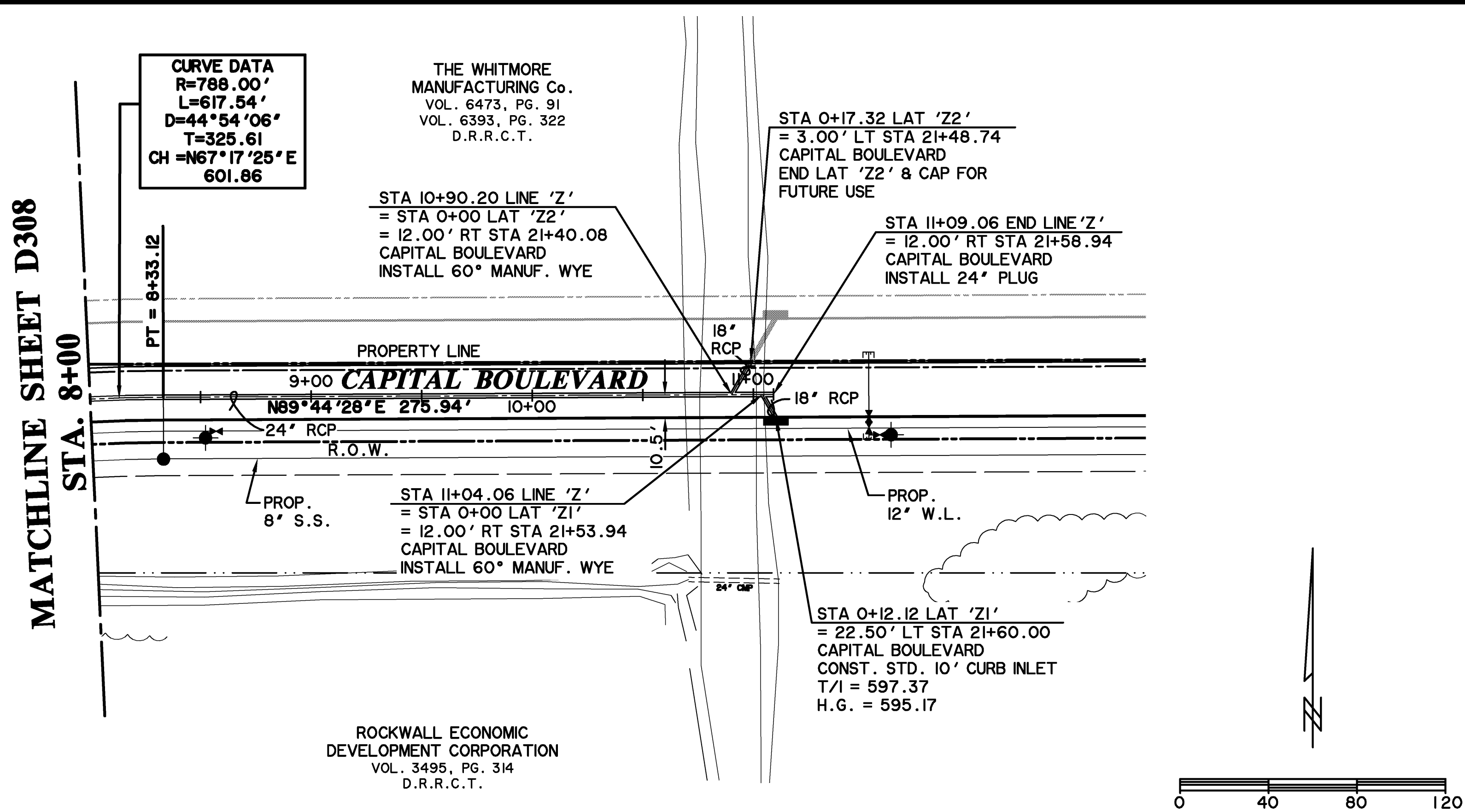
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DATE 01-19-2011
WA# 12209

SHEET NO
D307



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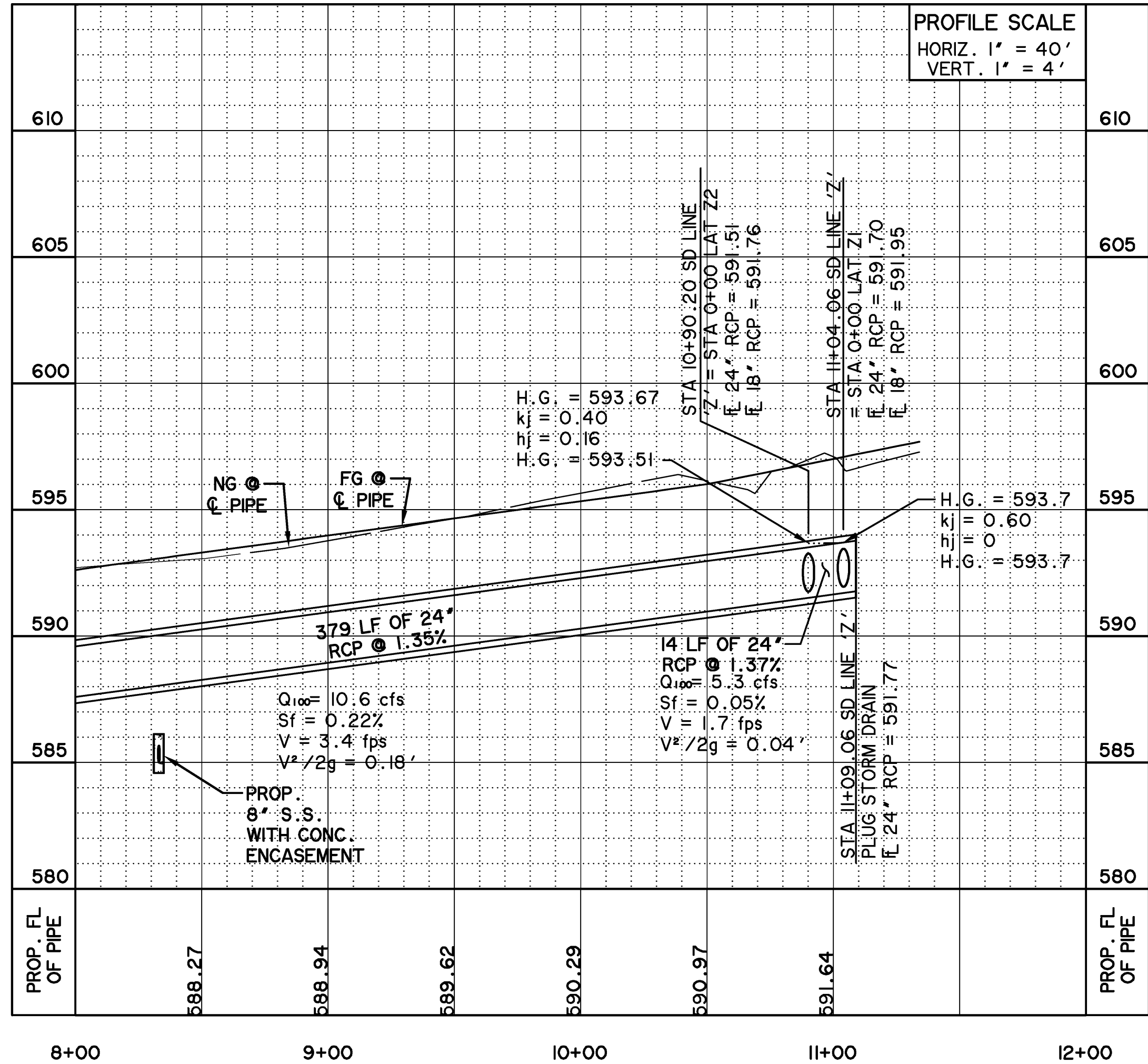
*** BENCH MARKS ***

BM A AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.

BM B AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.

BM C - AN '□' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

S T O R M D R A I N L I N E ' Z '

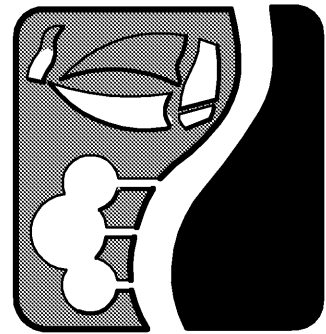


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02/02/2015**

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ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
www.WierAssociates.com
Texas Firm Registration No. F-2776



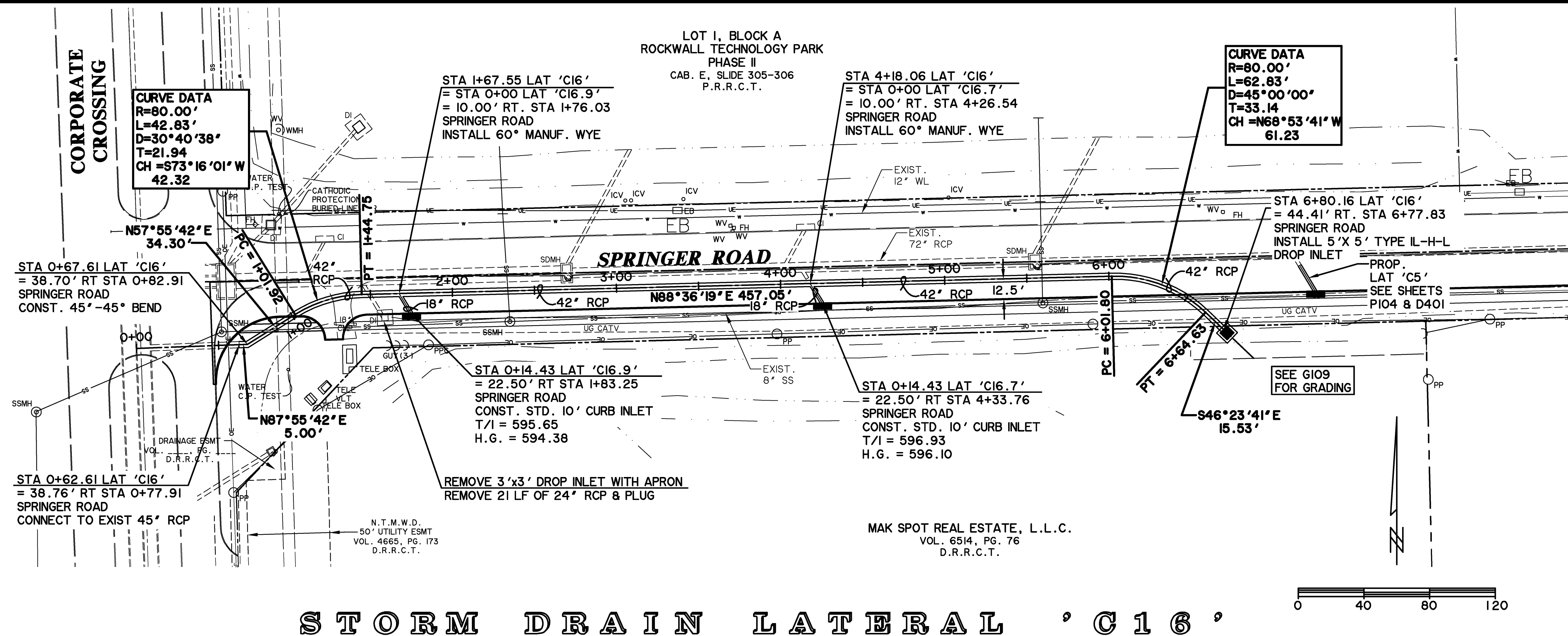
**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
PLAN AND PROFILE
LINE 'Z'**



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LAST SHEET OF 2
DATE: 02-02-2015
WA# 12209
**SHEET NO.
D308**

TIME 16:37 FILE: D309-SD-LAT-C16-12209.dwg



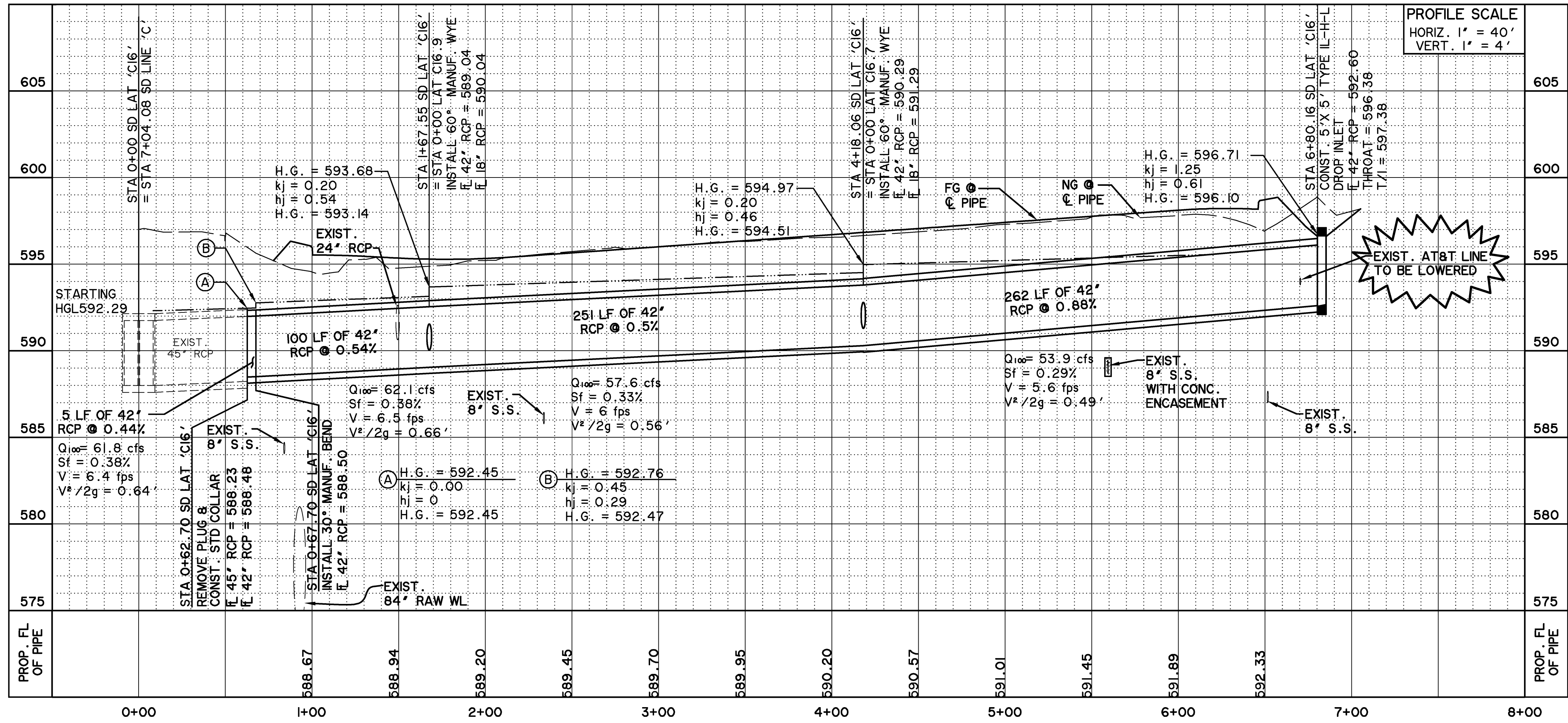
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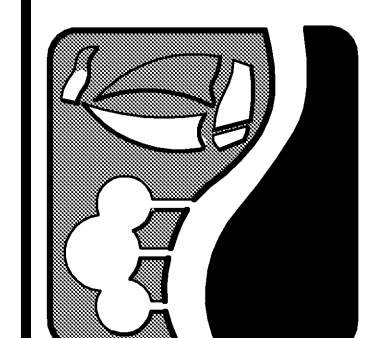


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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
PLAN AND PROFILE
LATERAL 'C16'**

STATE OF TEXAS
COUNTY OF ROCKWALL
DARRELL J. BOSWORTH
112702
REGISTERED PROFESSIONAL ENGINEER
2-2-15

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**SHEET NO.
D309**

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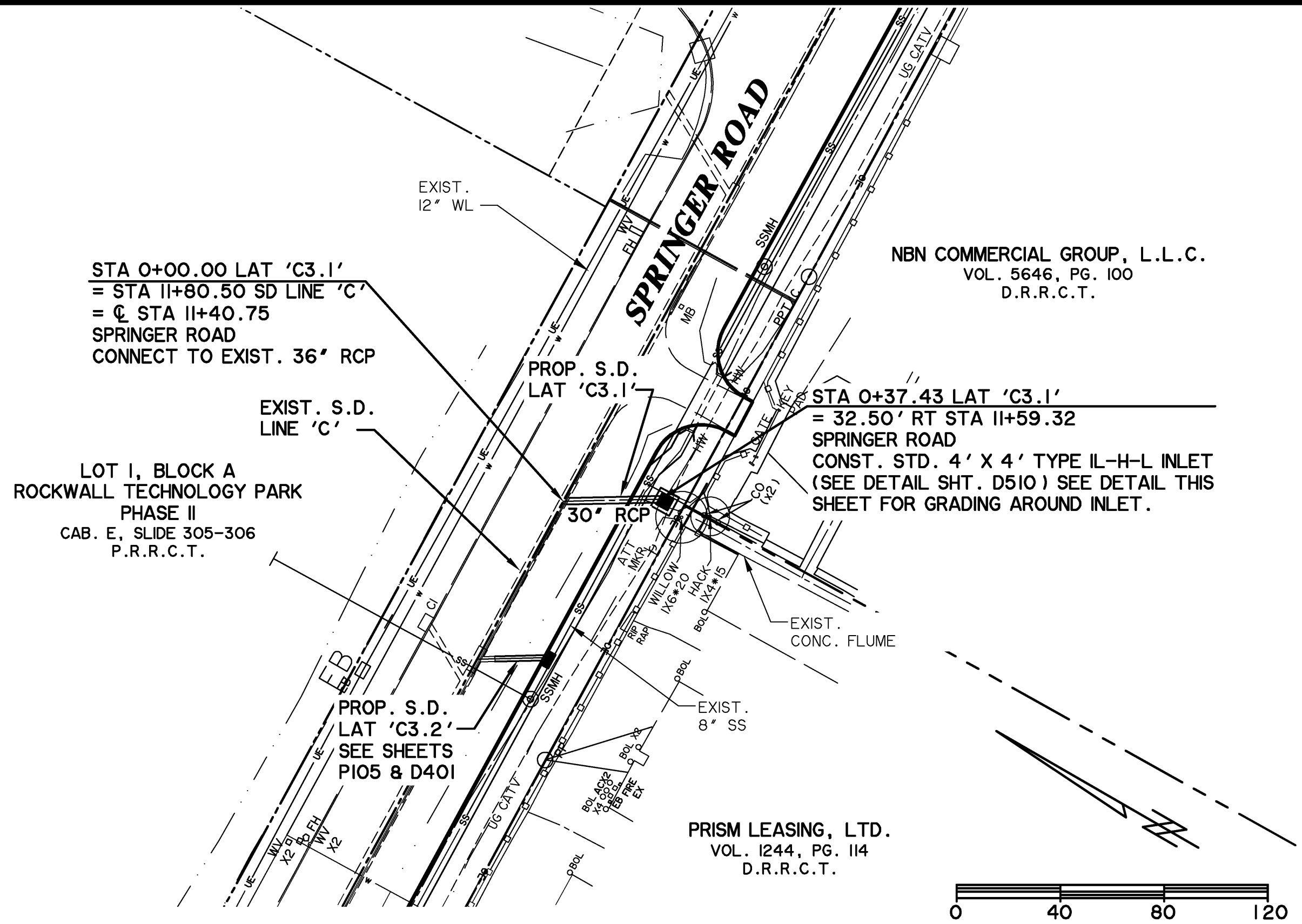
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• BENCH MARKS •

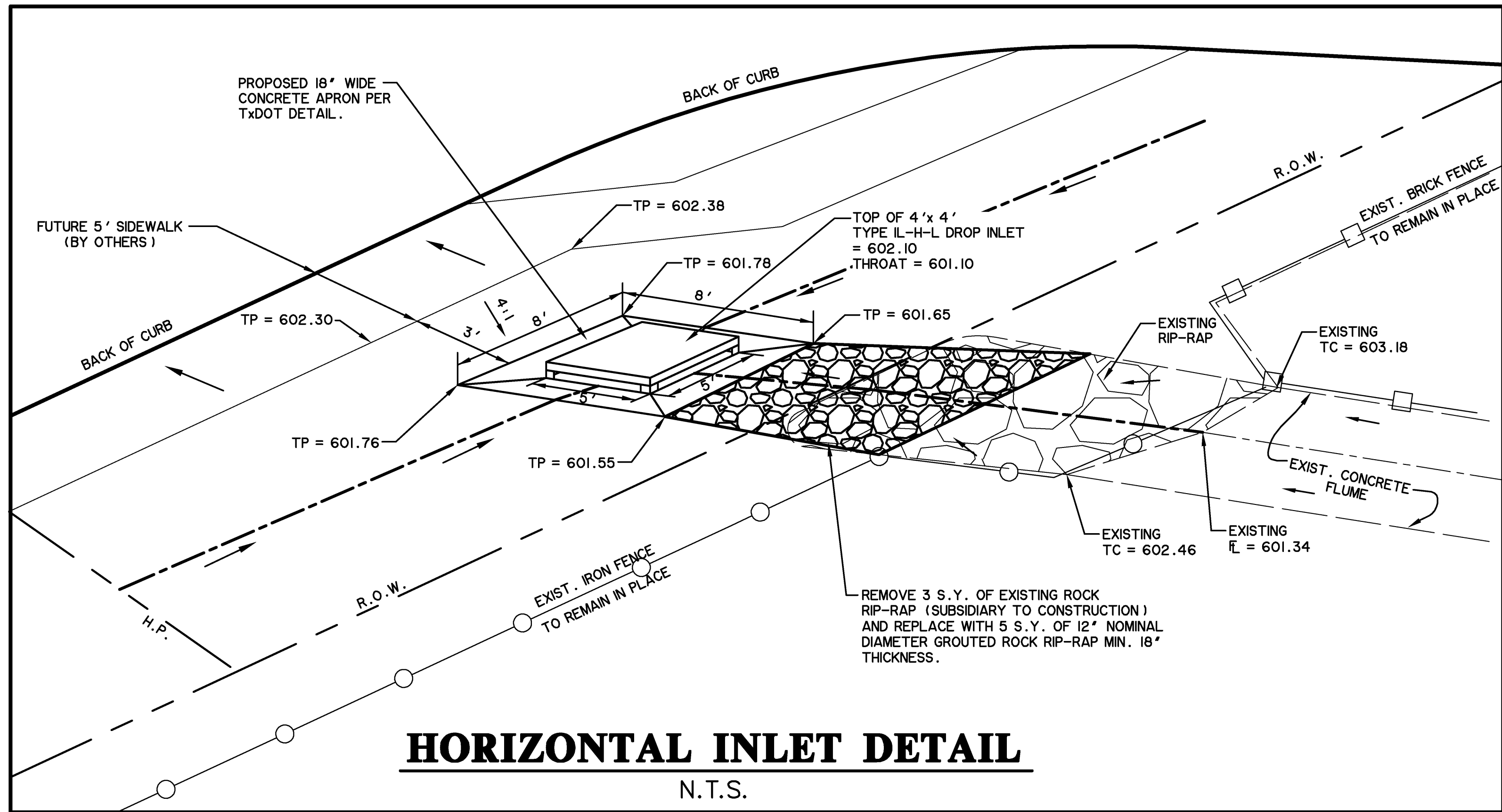
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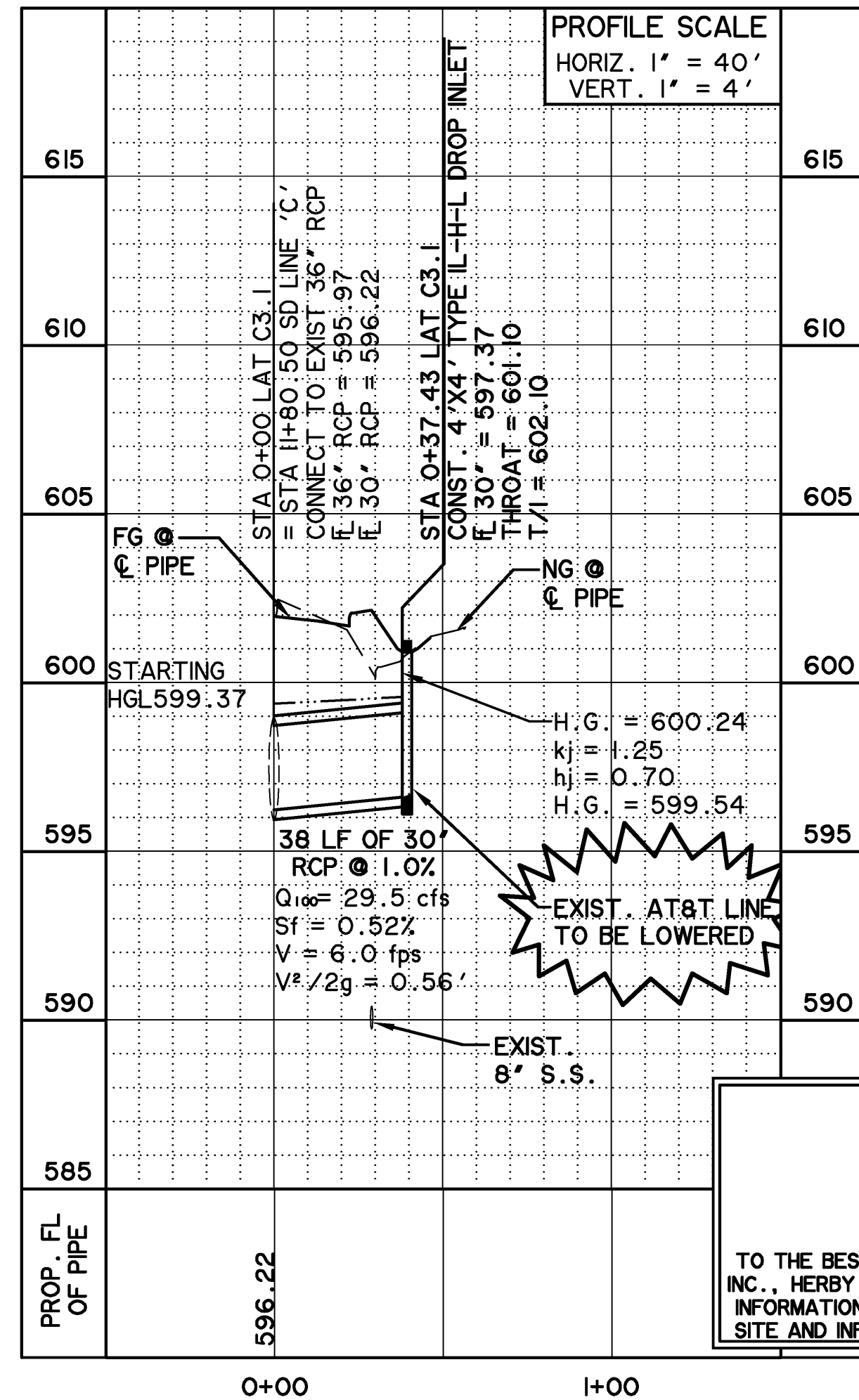


STORM DRAIN LATERAL 'C3.1'



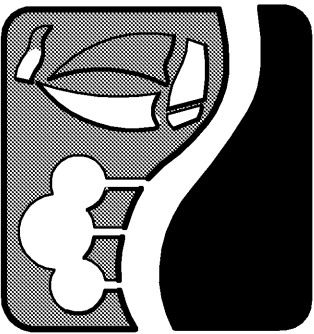
HORIZONTAL INLET DETAIL

N.T.S.



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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
PLAN AND PROFILE
LATERAL 'C3.1'**



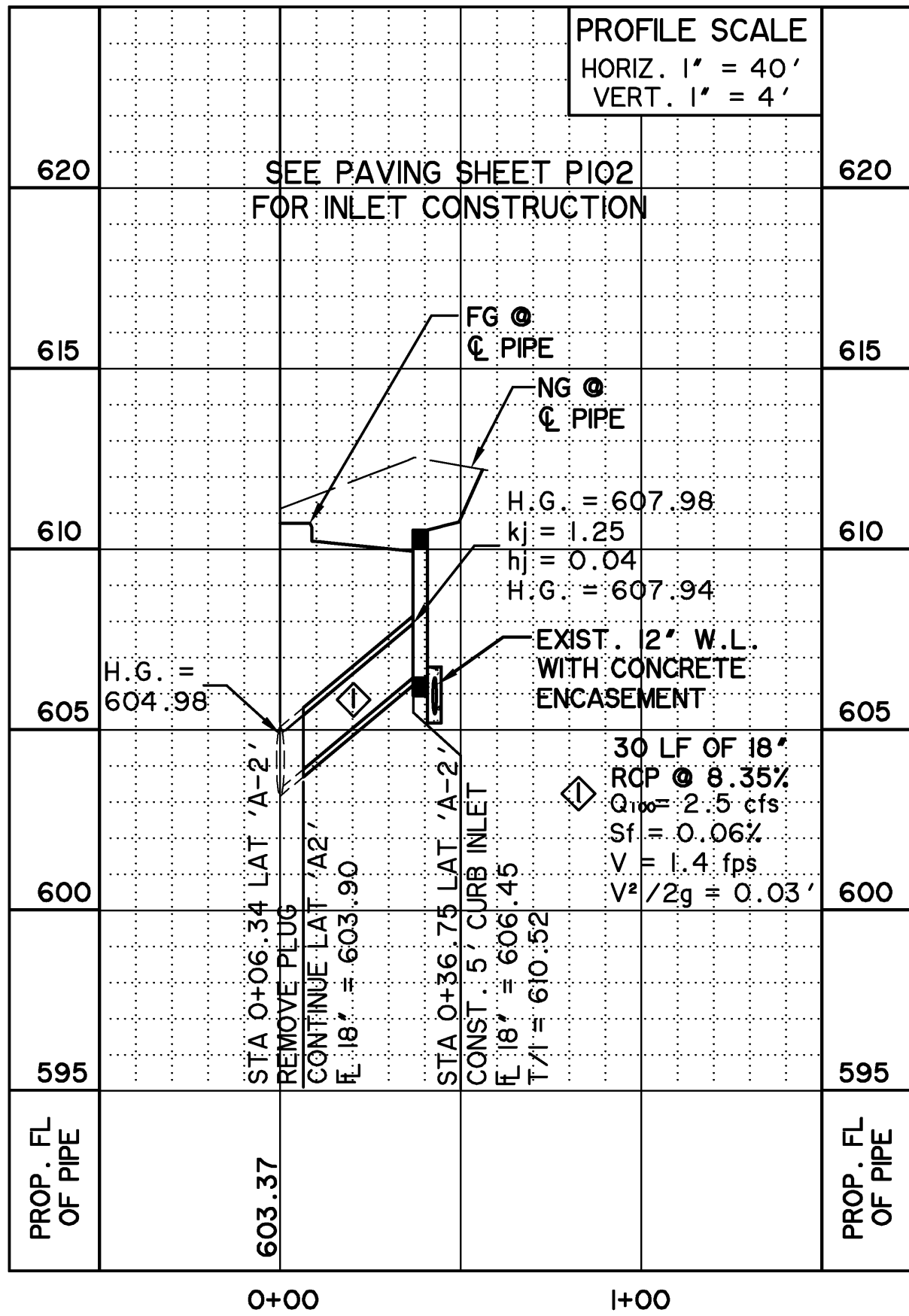
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**SHEET NO.
D310**

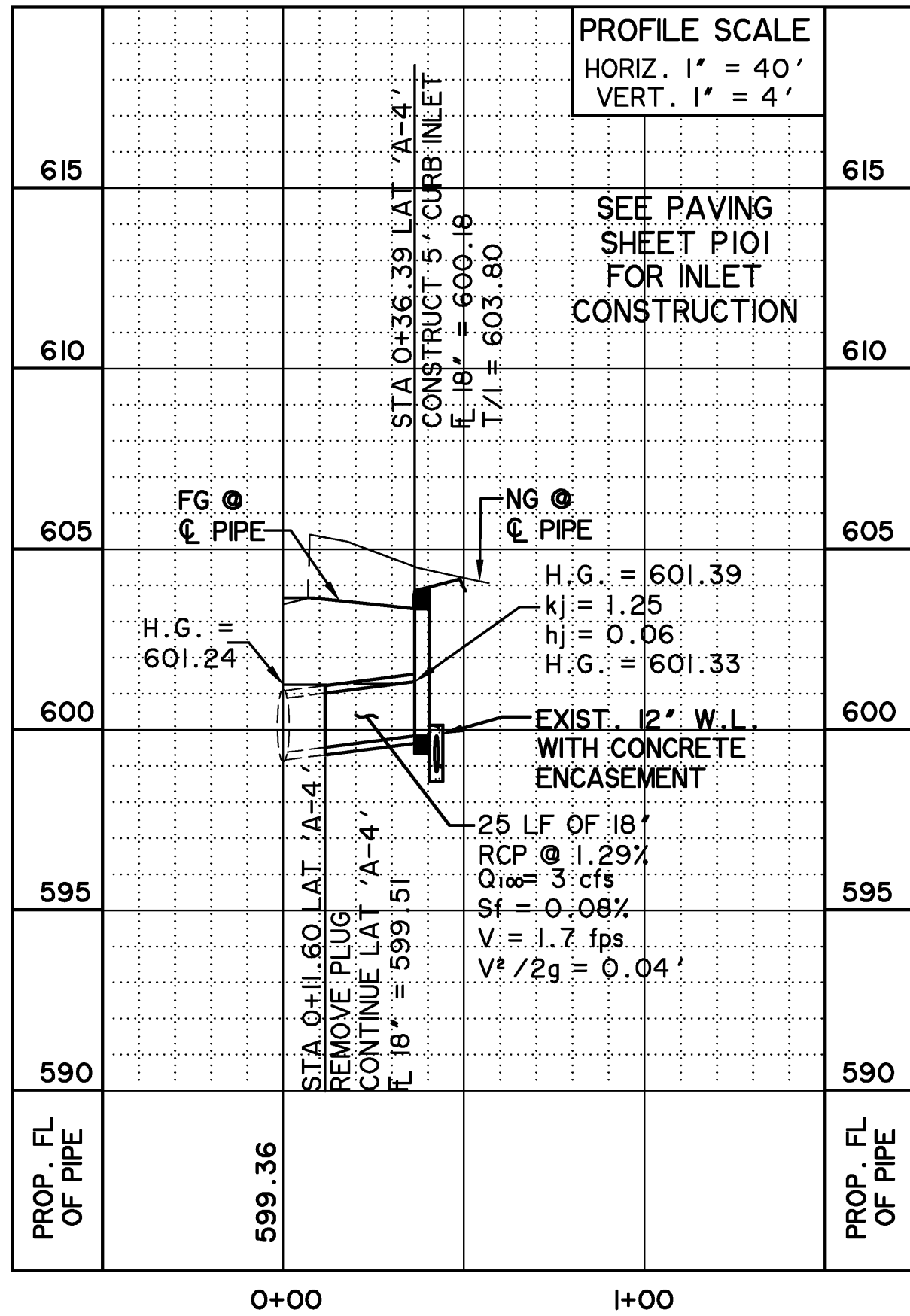
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701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
www.WierAssociates.com
Texas Firm Registration No. F-2776

TIME 10:31 FILE: D401-SD-LAT-12209.dwg

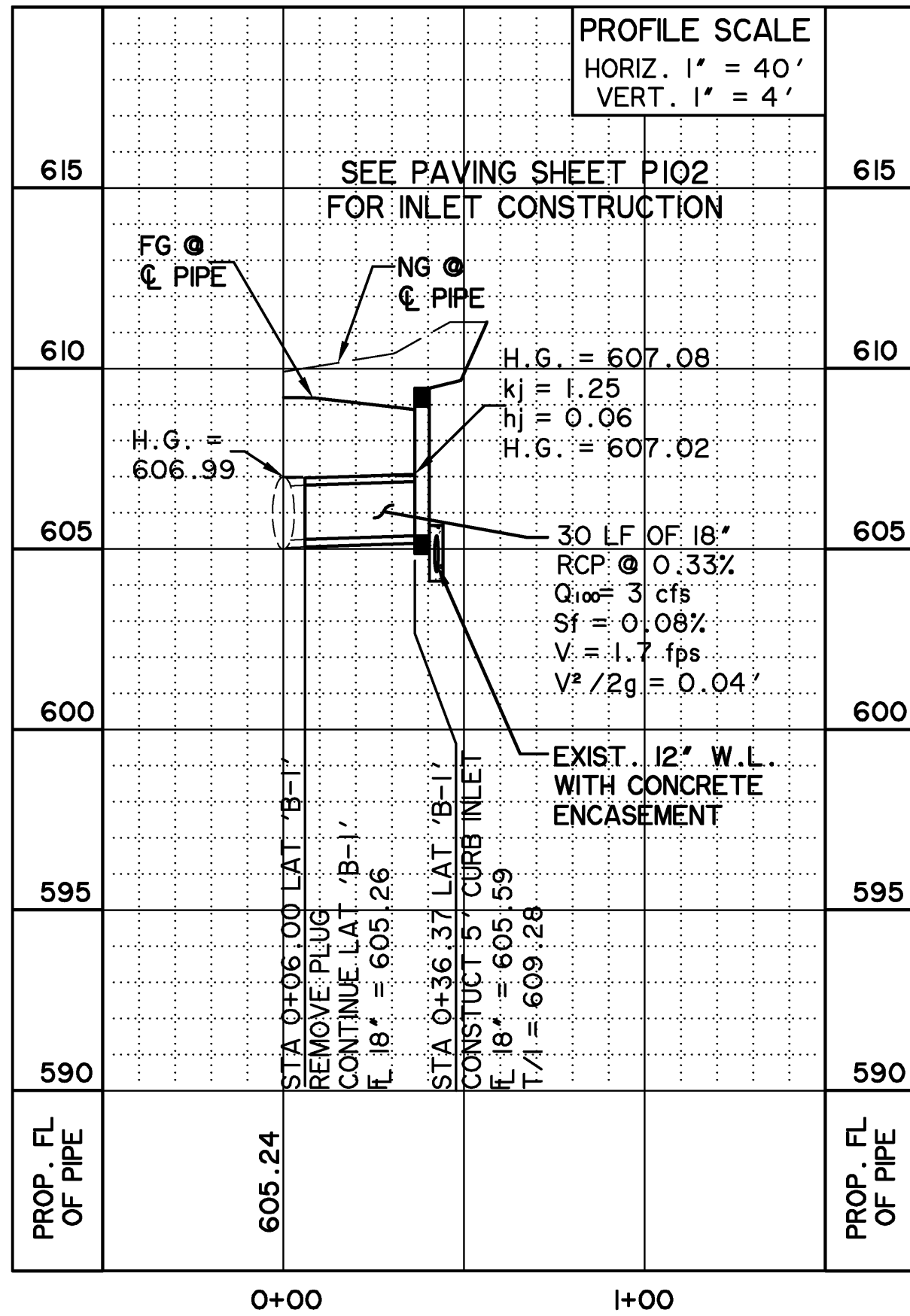
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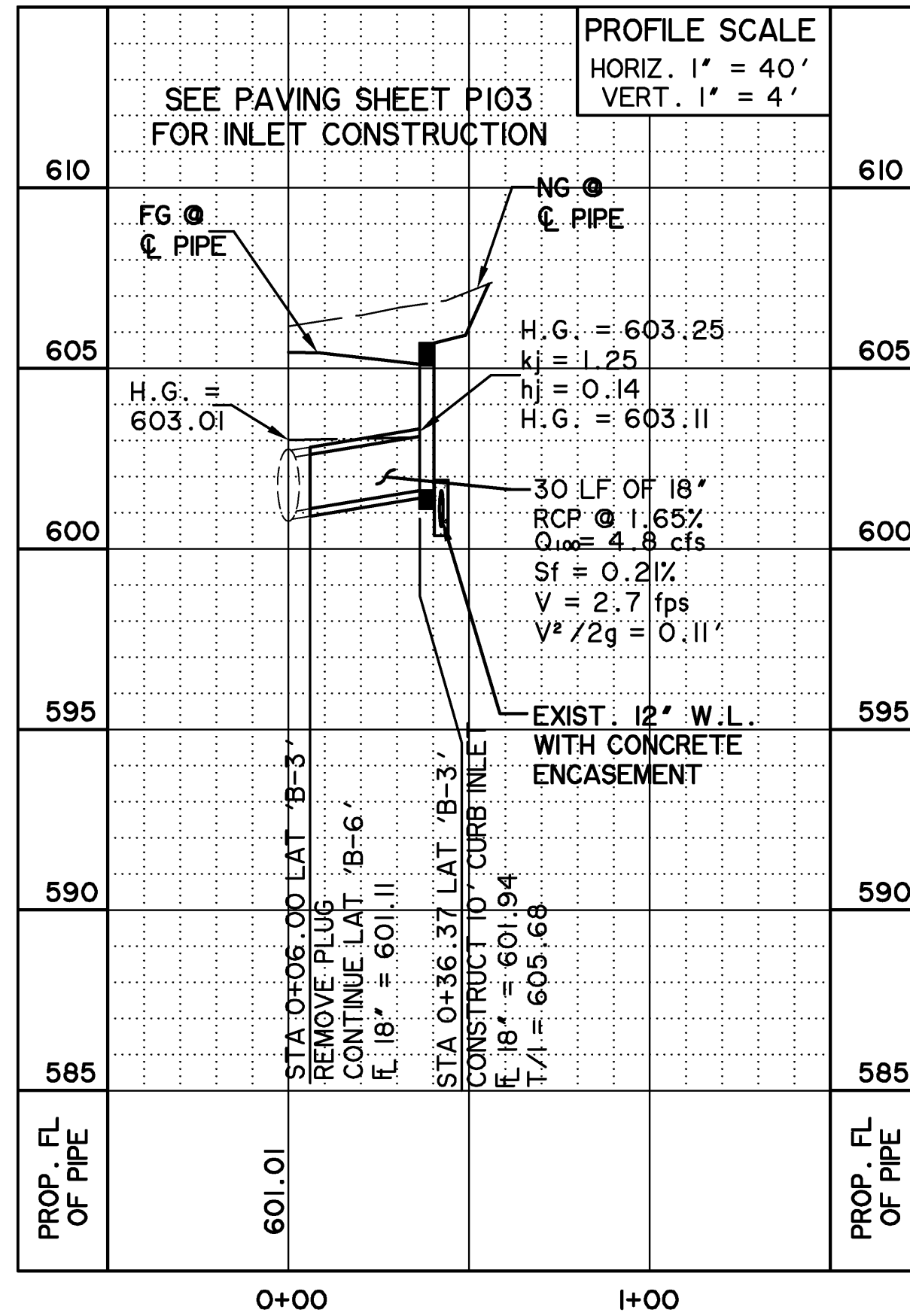
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L A T . ' B 1 '

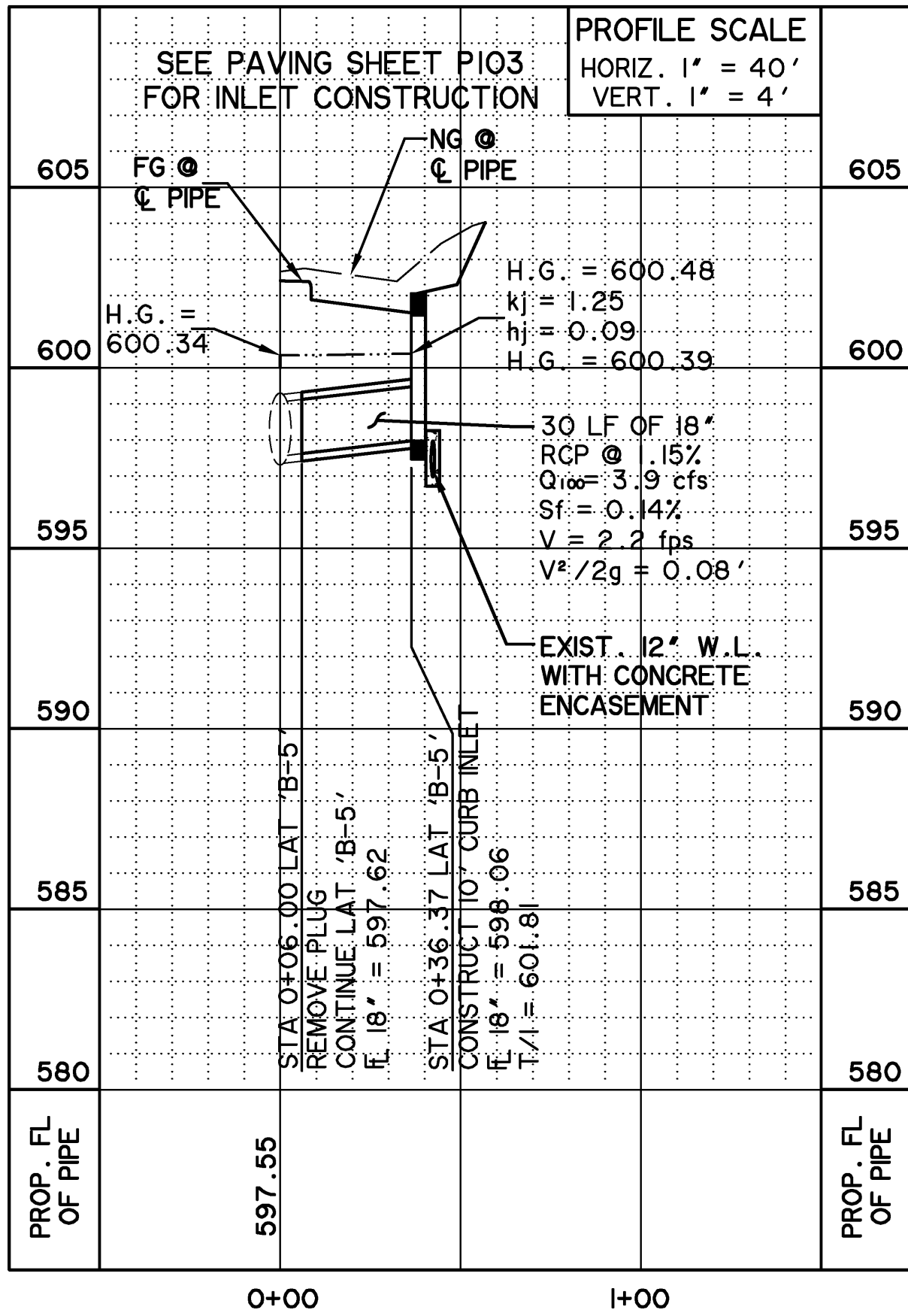


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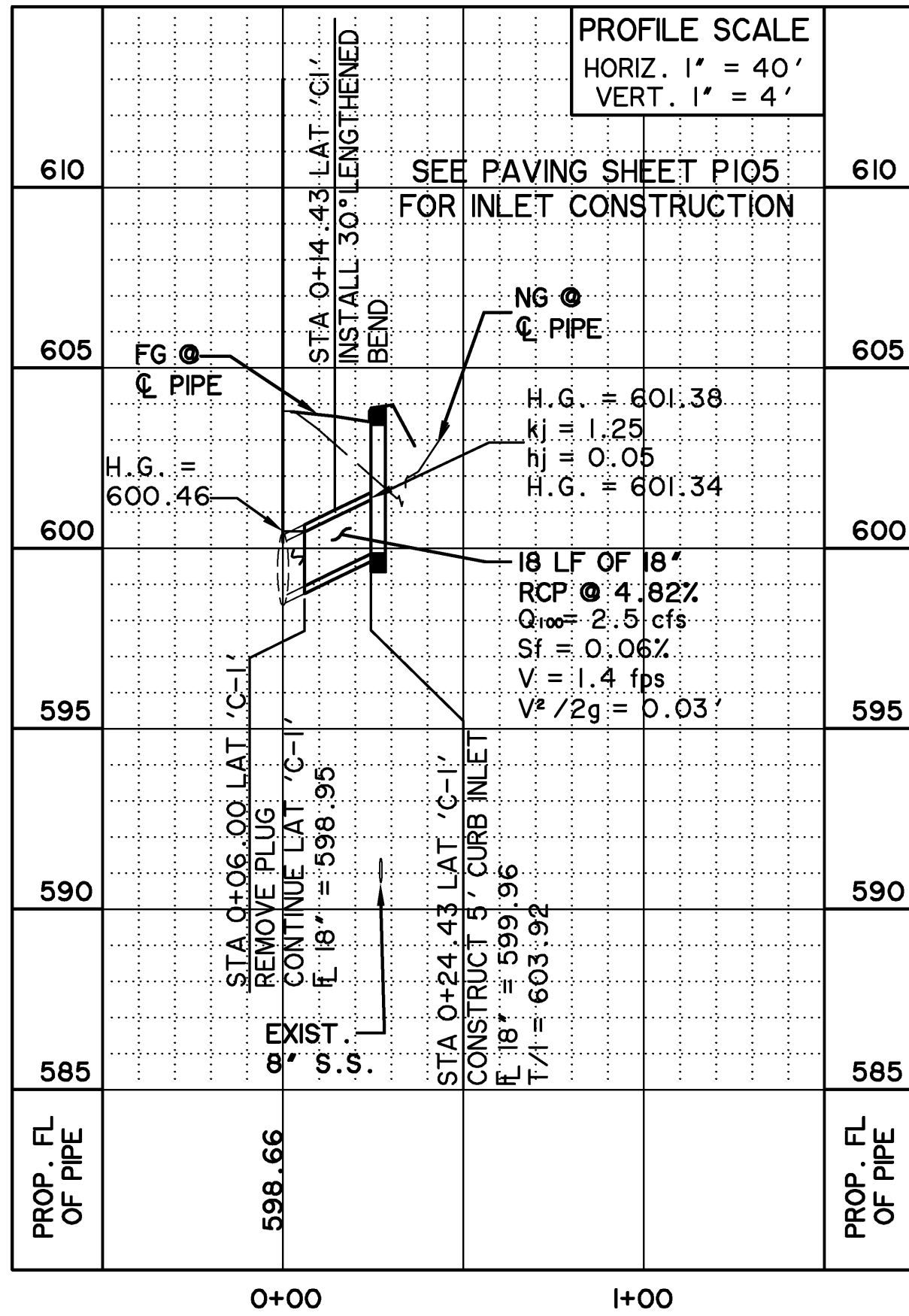


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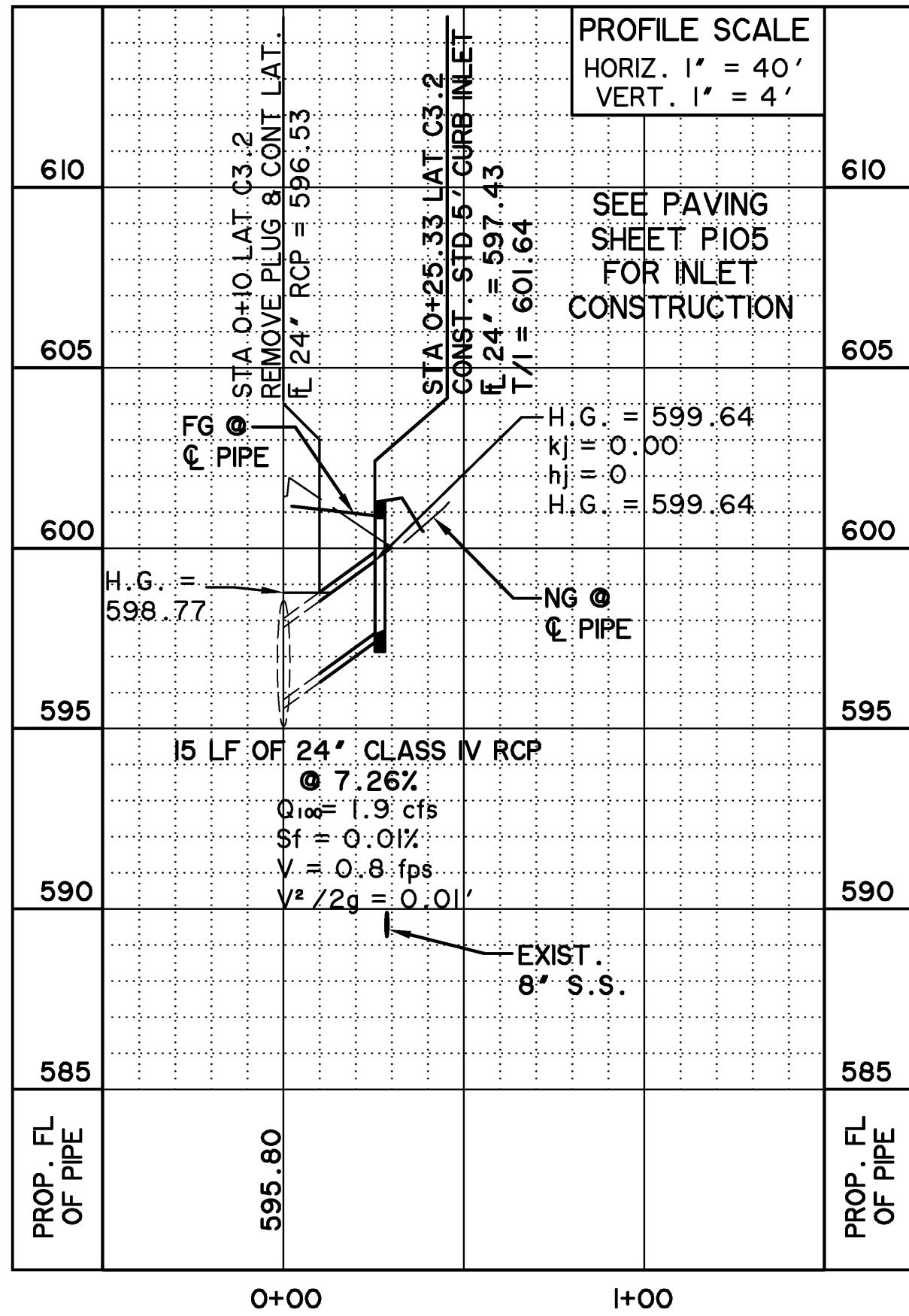
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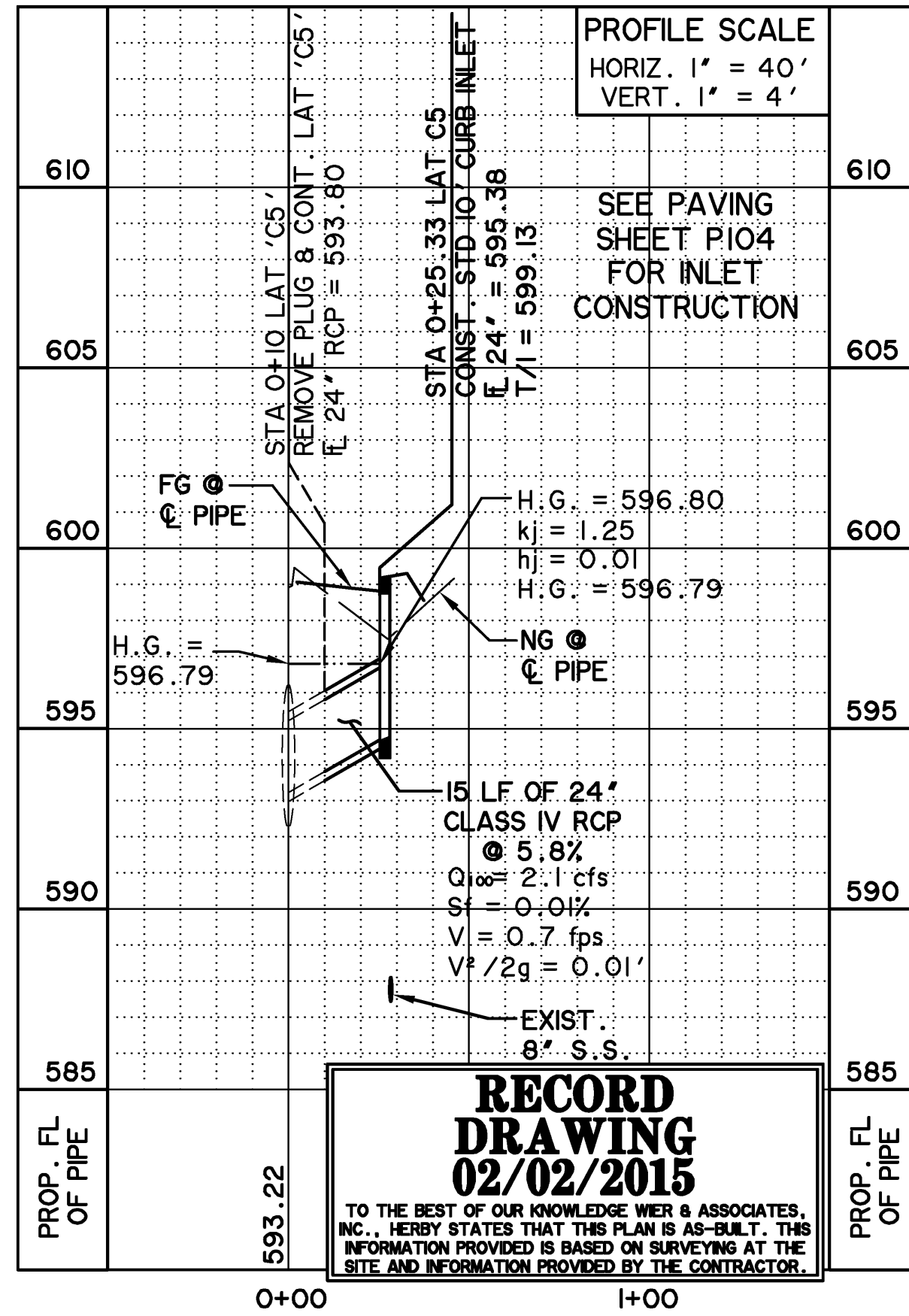
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L A T . ' C 3 . 2 '



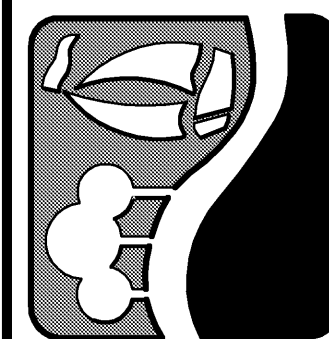
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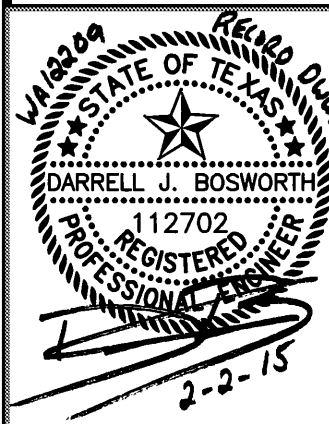
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
LATERALS
LINES 'A', 'B', & 'C'**



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**SHEET NO.
D401**

		PROFILE SCALE HORIZ. 1" = 40' VERT. 1" = 4'			
615					615
605					605
600					600
595					595
590					590
585					585
PROP. FL OF PIPE					PROP. FL OF PIPE

		PROFILE SCALE HORIZ. 1" = 40' VERT. 1" = 4'			
610					610
600					600
595					595
590					590
585					585
580					580
PROP. FL OF PIPE					PROP. FL OF PIPE

610				<p>PROFILE SCALE HORIZ. 1" = 40' VERT. 1" = 4'</p> <p>SEE PAVING SHEET P106 FOR INLET CONSTRUCTION</p>	610
605					605
600					600
595					595
590					590
585					585

610						<p>PROFILE SCALE HORIZ. 1" = 40' VERT. 1" = 4'</p> <p>SEE PAVING SHEET P106 FOR INLET CONSTRUCTION</p>	610
605							605
600							600
595							595
590							590
585							585
PROP. FL OF PIPE	597.50						PROP. FL OF PIPE

		PROFILE SCALE	
		HORIZ. 1' = 40' VERT. 1' = 4'	
615			615
605			605
600			600
595			595
590			590
585			585
PROP. FL OF PIPE		PROP. FL OF PIPE	

		PROFILE SCALE	
		HORIZ. 1" = 40' VERT. 1" = 4'	
615			615
610			610
600			600
595			595
590			590
585			585
PROP. FL OF PIPE		PROP. FL OF PIPE	

		PROFILE SCALE HORIZ. 1" = 40' VERT. 1" = 4'			
610					610
605					605
600					600
595					595
590					590
585					585
PROP. FL. OF PIPE					PROP. FL. OF PIPE

STA 0+00 LAT. M3
= STA 3+76.79 SD LINE 'M'
FL 18" RCP = 593.3
FL 27" RCP = 592.75

STA 0+25.67 LAT. M3
CONST. STD. 10" CURB INLET
FL 18" = 593.55
T/I = 597.46

FG ●
C PIPE
H.G. = 596.01

NG ●
C PIPE
H.G. = 596.39
kj = 1.25
hj = 0.28
H.G. = 596.11

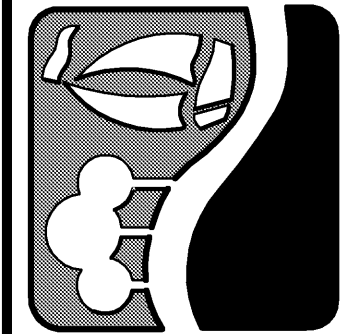
24 LF OF 18" RCP ● 0.85%
Q₁₀₀ = 6.7 cfs
Sf = 0.41%
V = 3.8 fps
V²/2g = 0.22'

PROP. 12" W.L. CONCRETE ENCASE

533.30

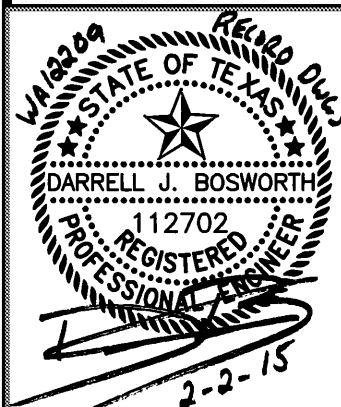
		PROFILE SCALE HORIZ. 1' = 40' VERT. 1' = 4'	
610			610
605		STA 0+00 LAT M4 = STA 3+76.79 SD LINE "M" FL 18" RCP = 593.3 TL 27" RCP = 532.75	605
600		STA 0+25.67 LAT M4 CONST. STD 10" CURB INLET FL 18" = 593.39 T/I = 597.47	600
595		FG @ C PIPE H.G. = 596.01	595
590		H.G. = 596.39 k _j = 1.25 h _f = 0.28 H.G. = 596.11	590
585		24 LF OF 18" RCP @ 0.85% Q _{max} = 6.7 cfs Sf = 0.41% V = 3.8 fps. V² / 2g = 0.22'	585
PROP. FL OF PIPE	593.30	<h1 style="margin: 0;">RECORD DRAWING</h1> <h2 style="margin: 0;">02/02/2015</h2> <p style="font-size: small; margin-top: 5px;">TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HEREBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.</p>	PROP. FL OF PIPE

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ROCKWALL TECHNOLOGY PARK PHASE IV

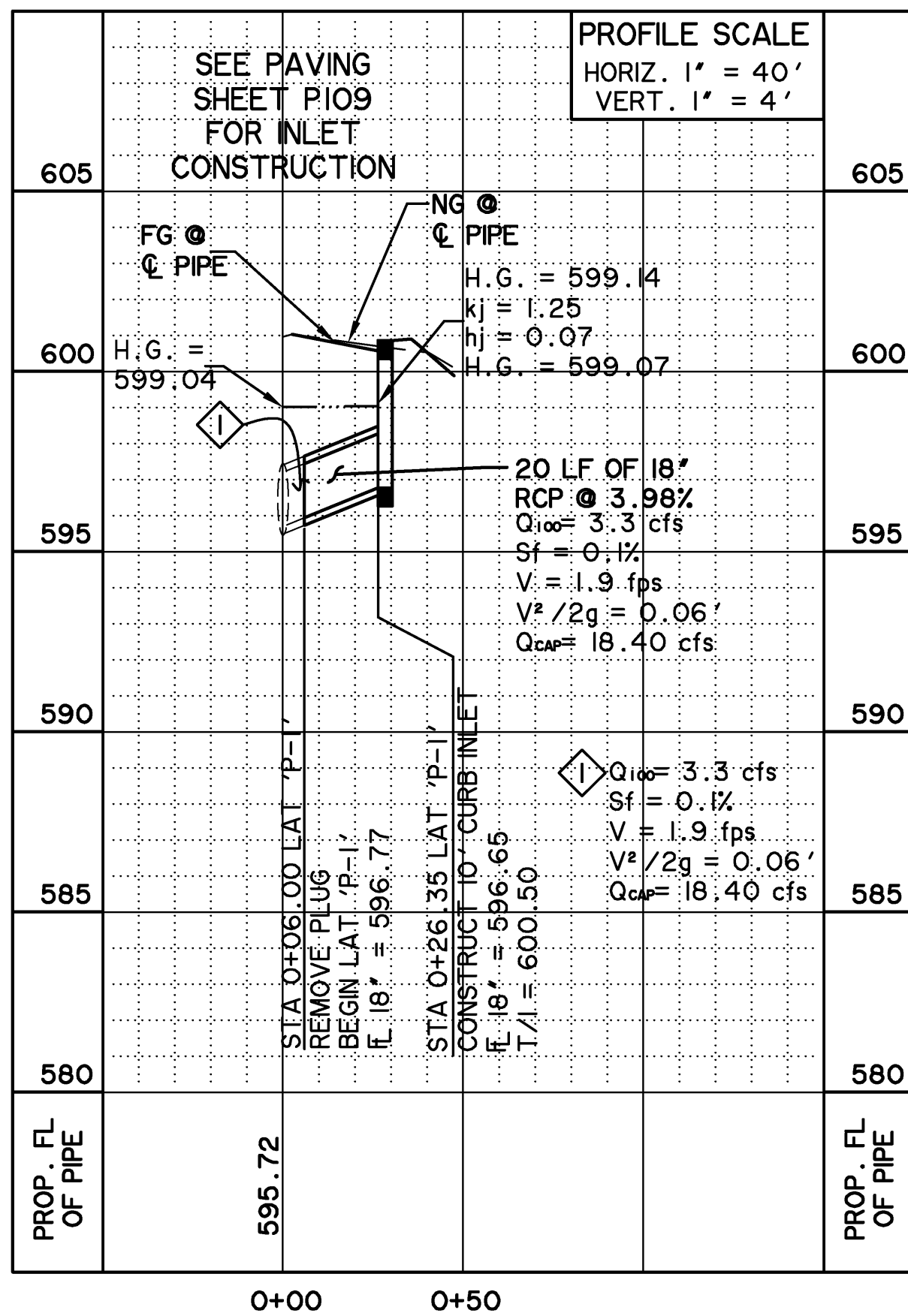
STORM DRAIN LATERALS LINES 'C', 'D', & 'M'



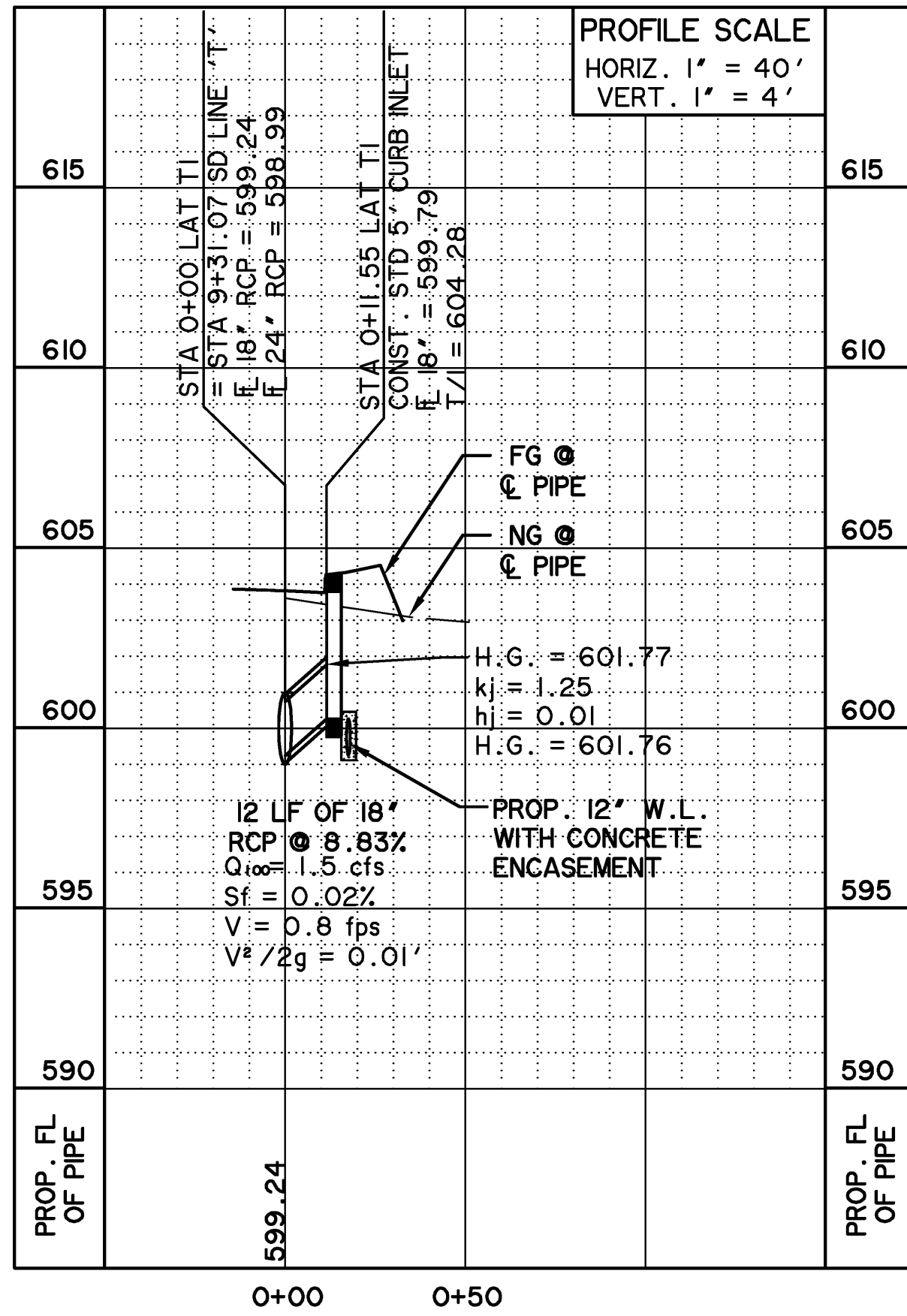
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SHEET NO.
D402

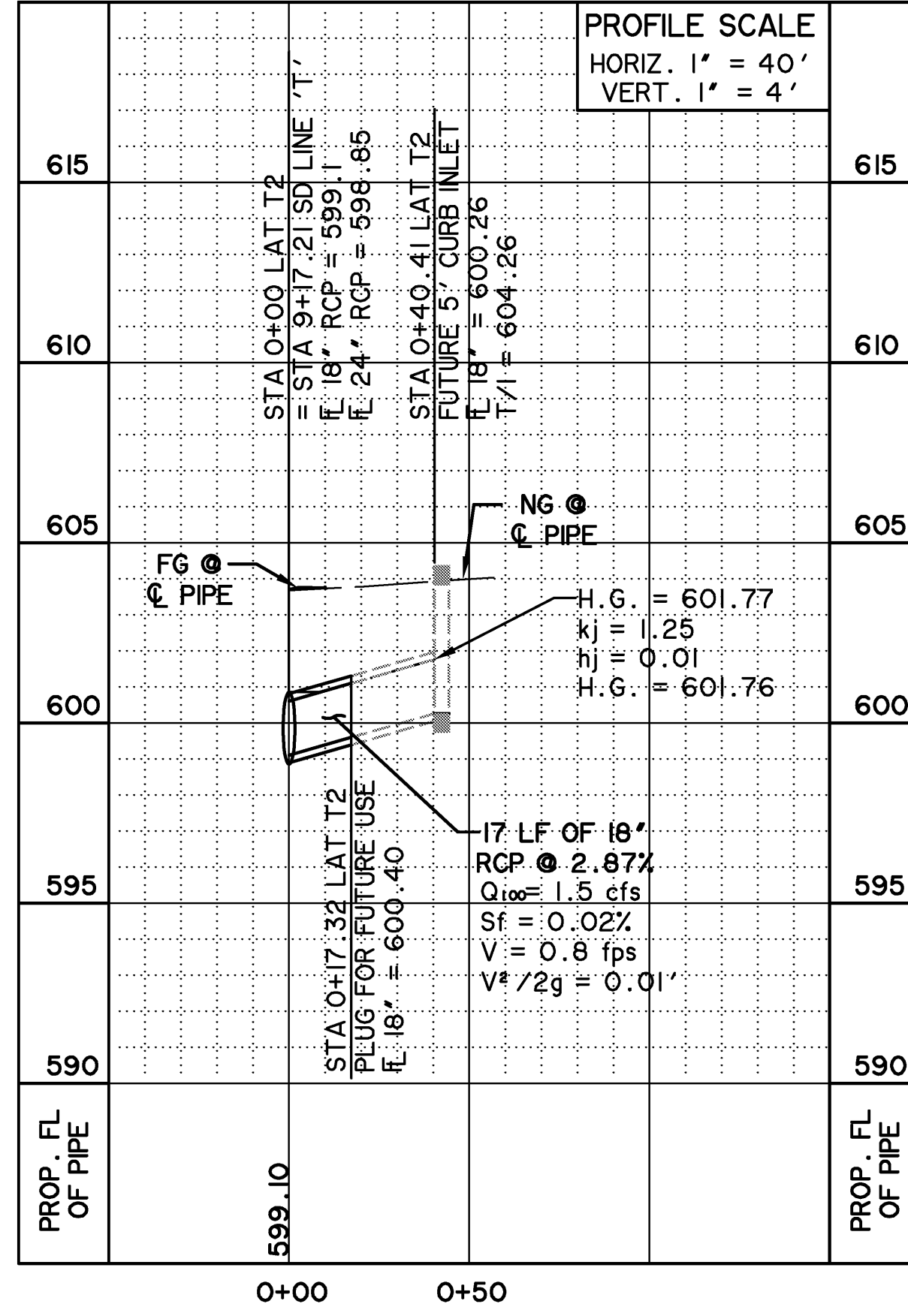
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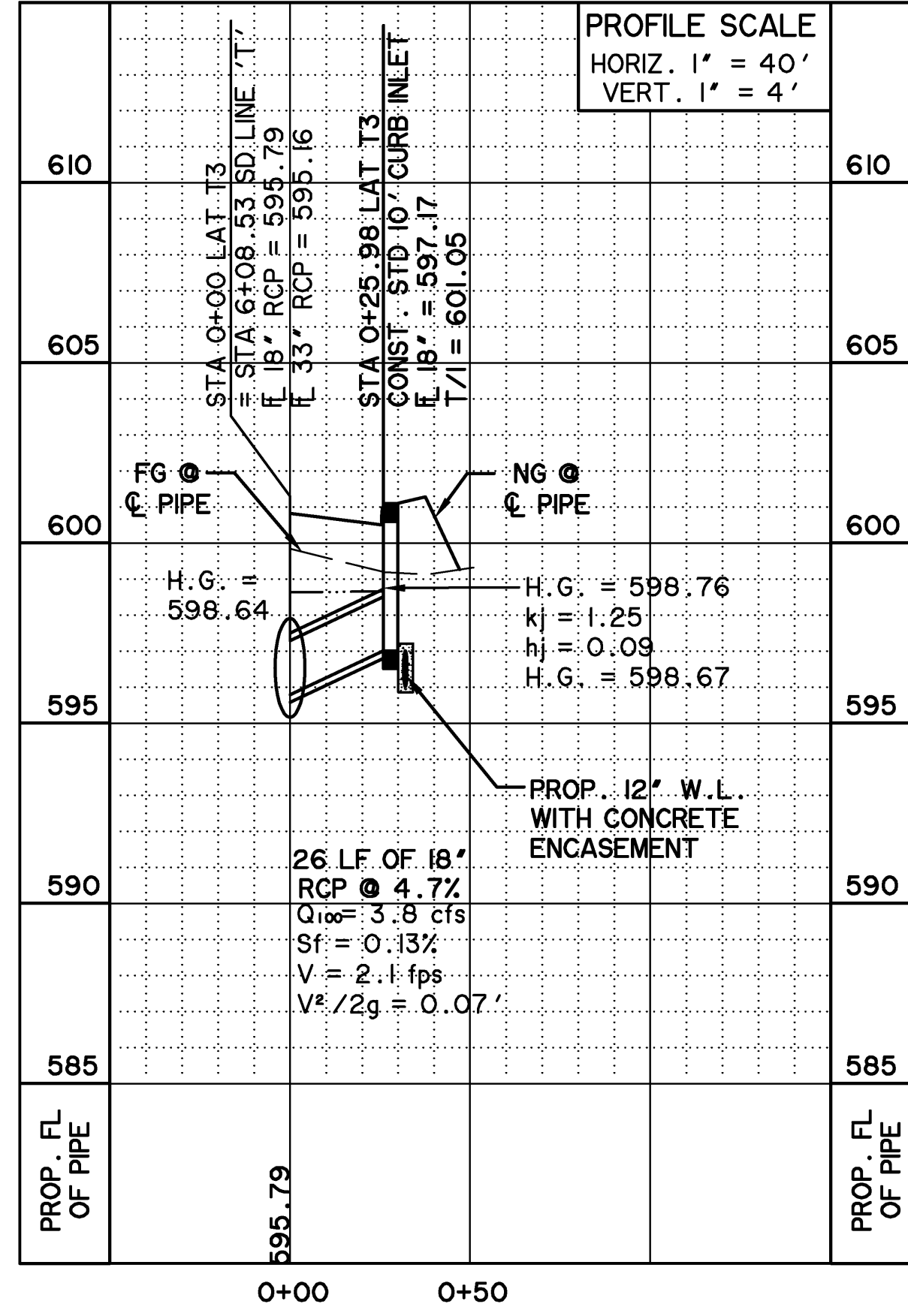
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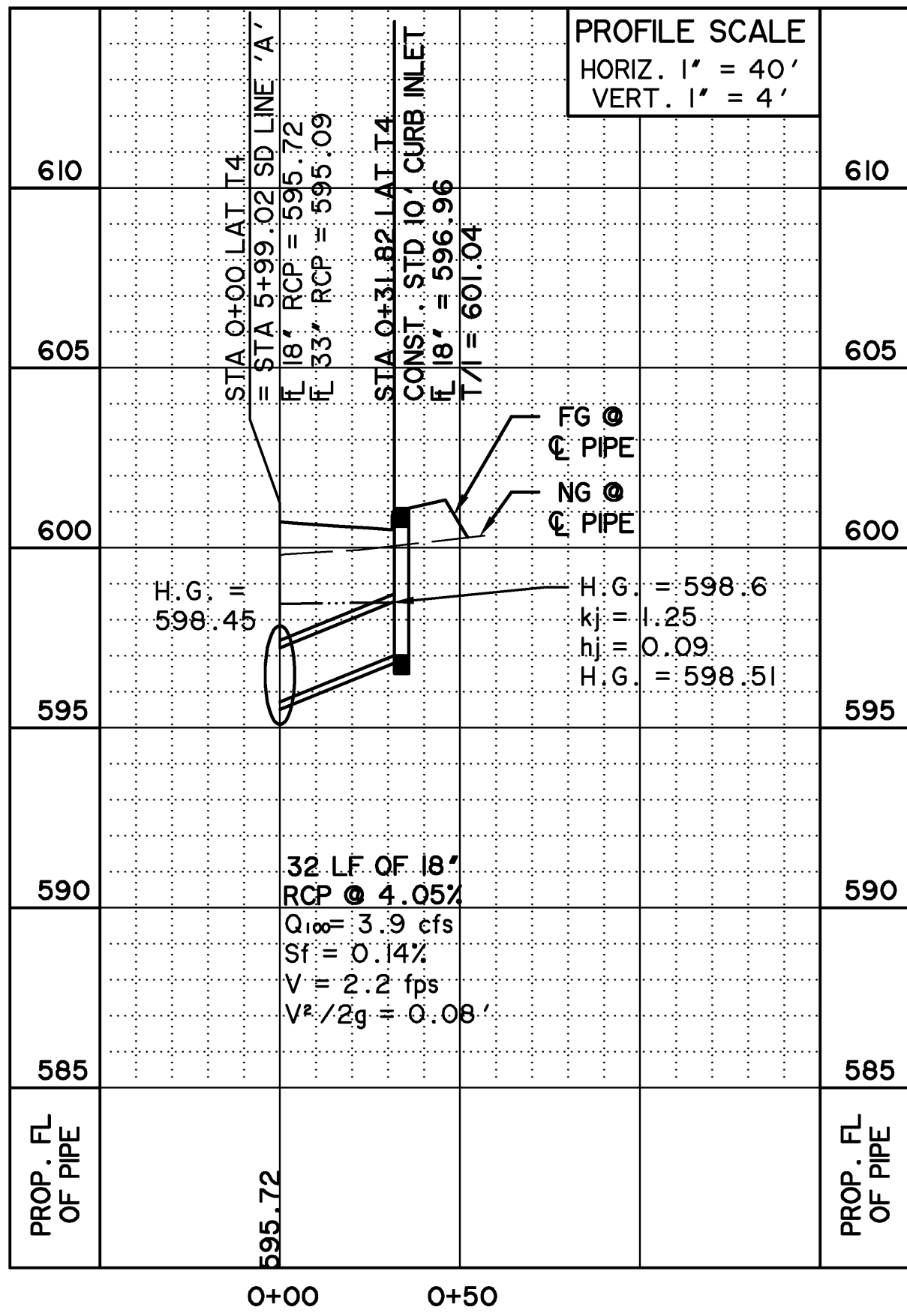
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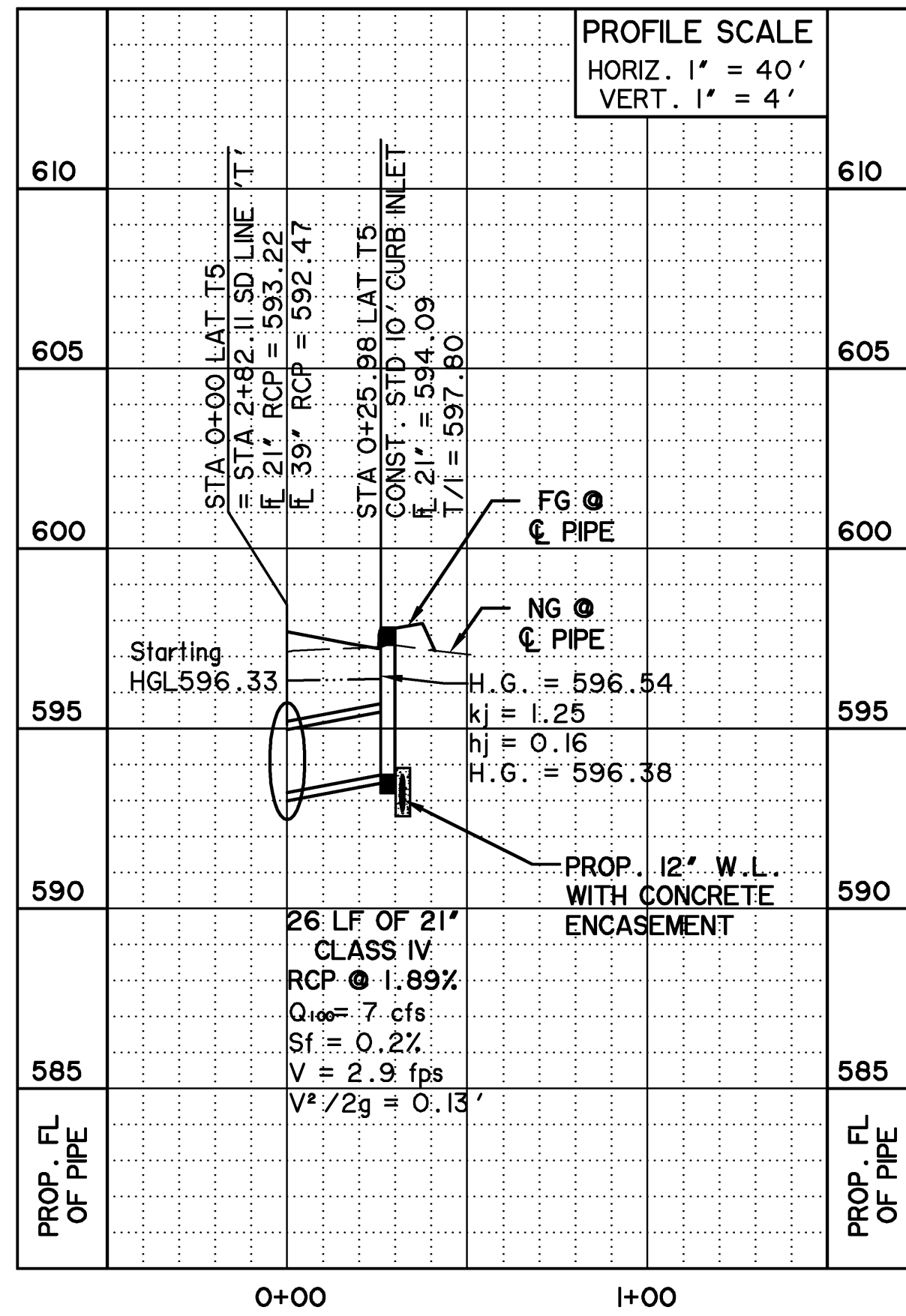
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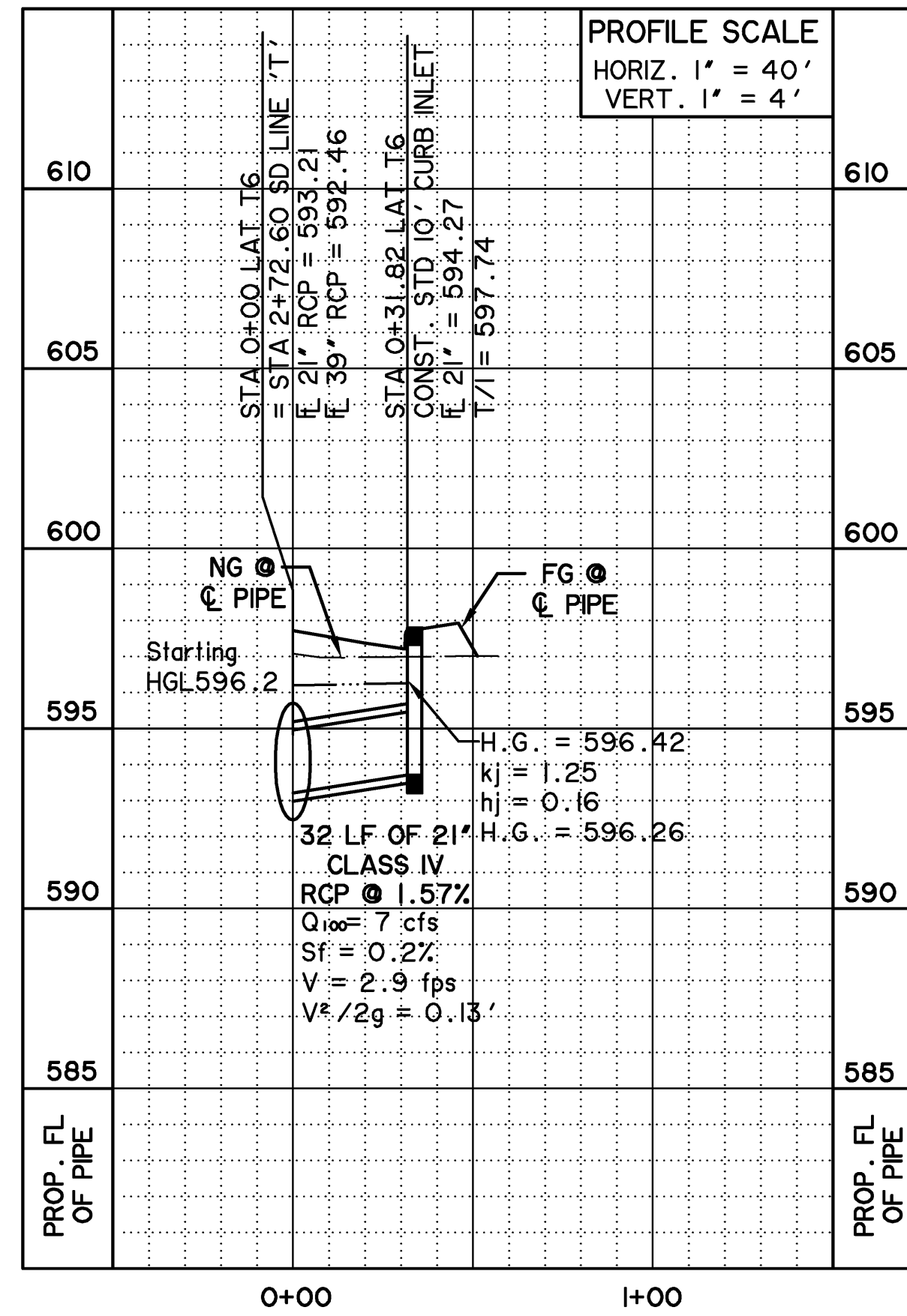
LAT. 'T4'



LAT. 'T5'



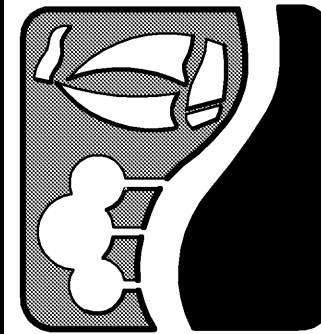
LAT. 'T6'



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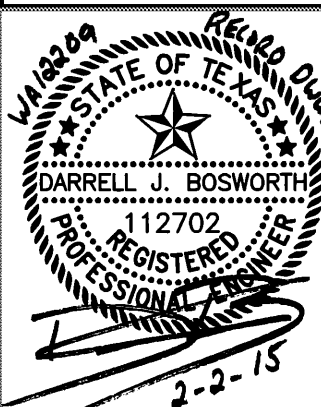
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
LATERALS
LINES 'P' & 'T'**

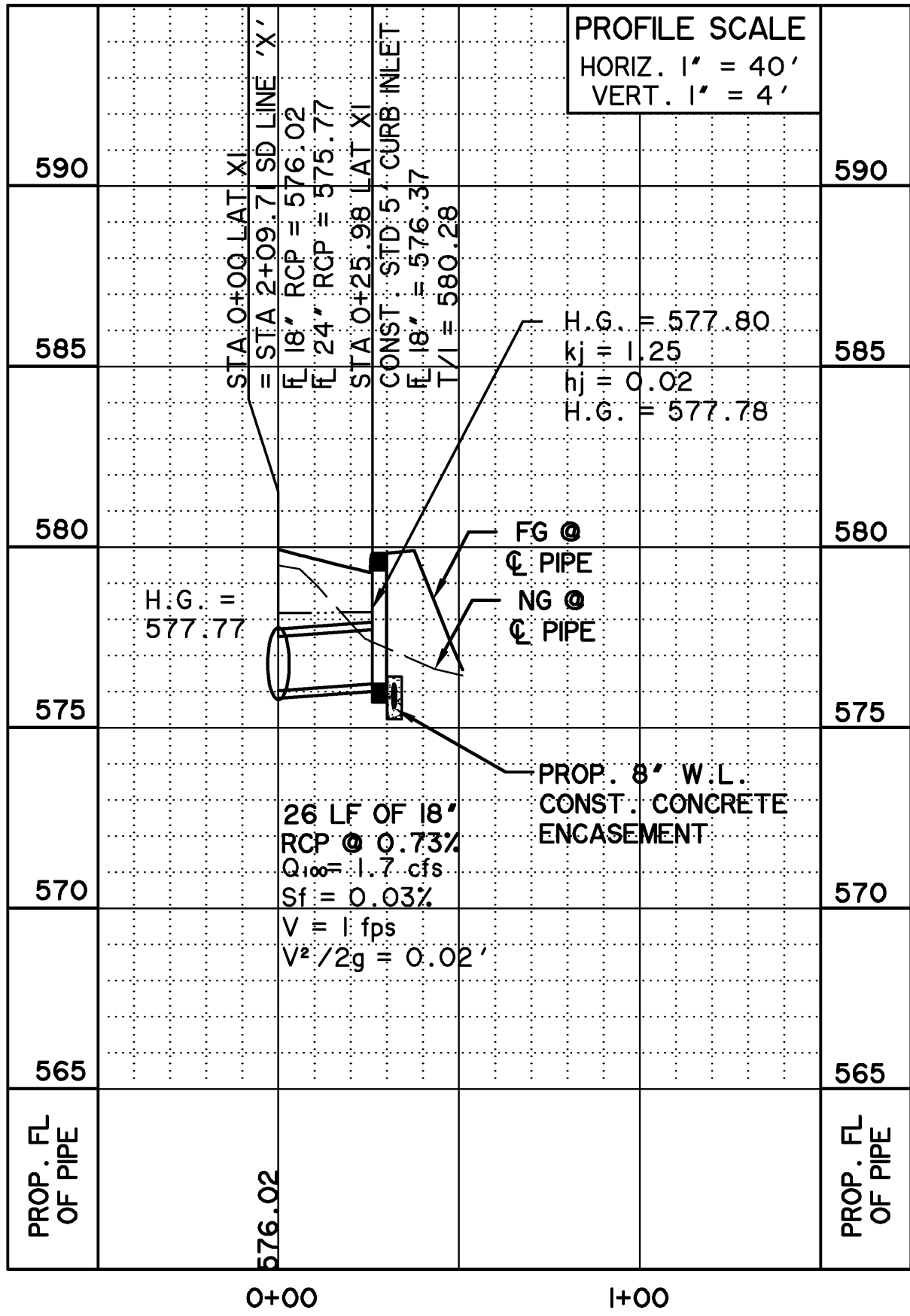


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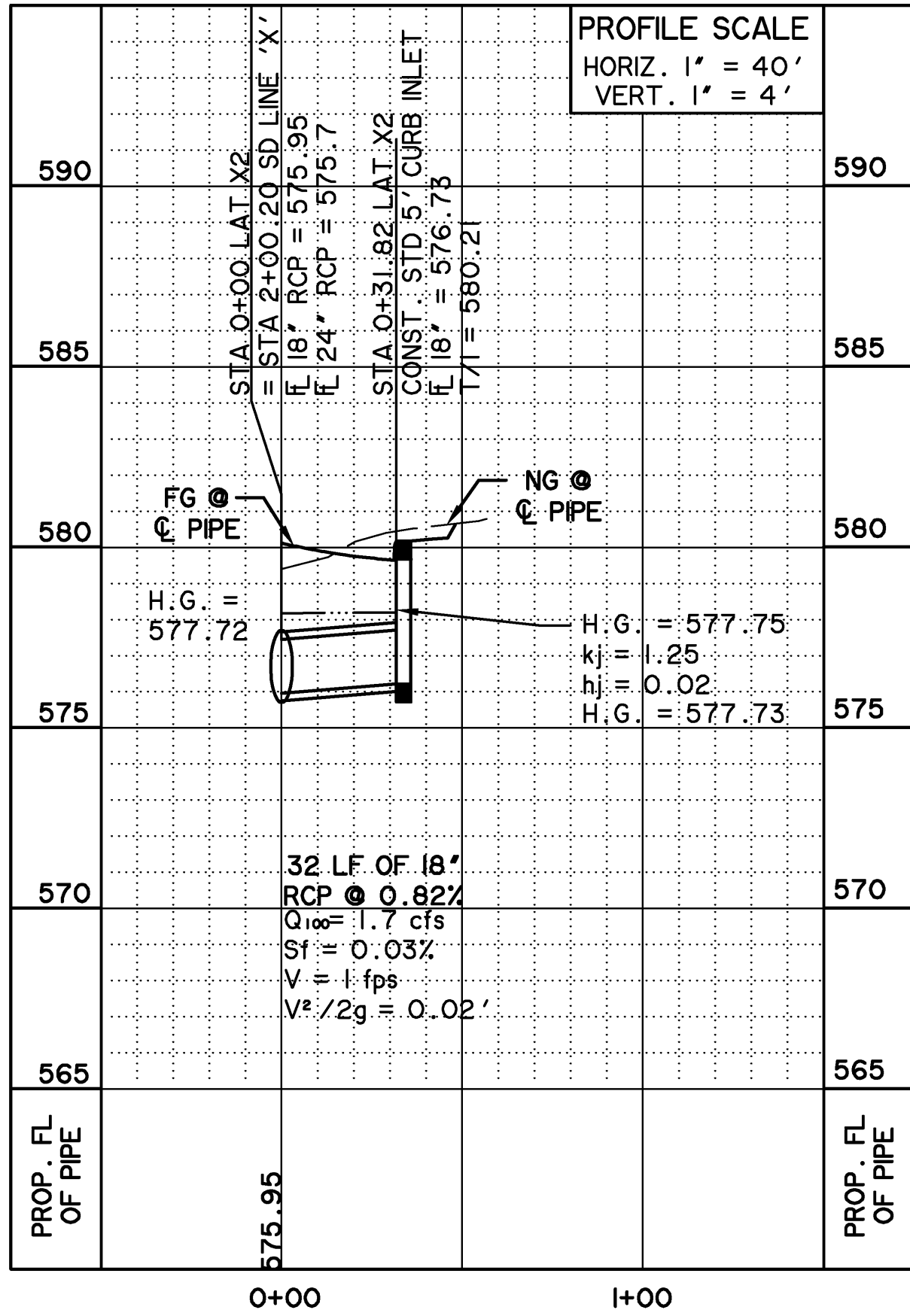
**SHEET NO.
D403**

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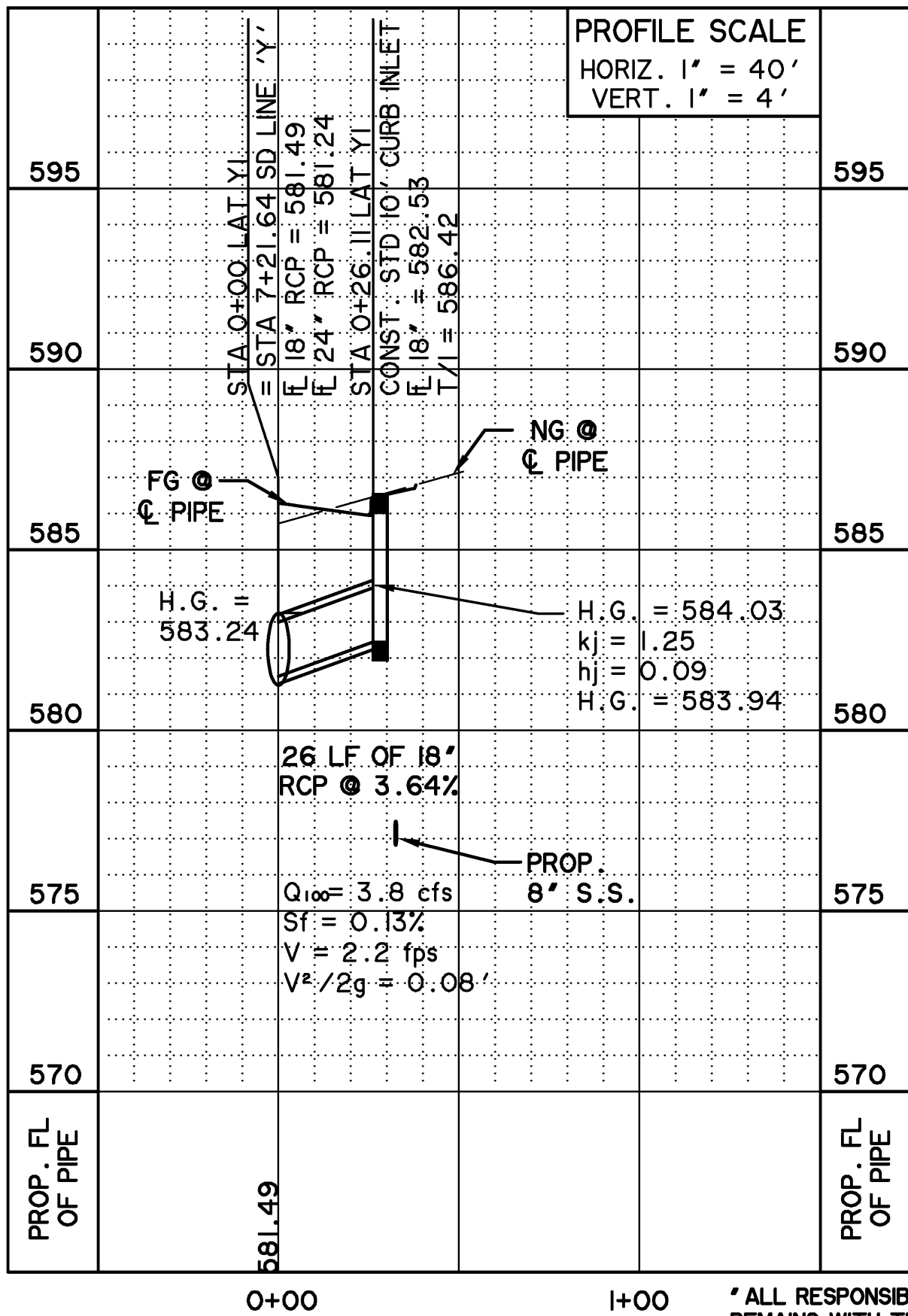
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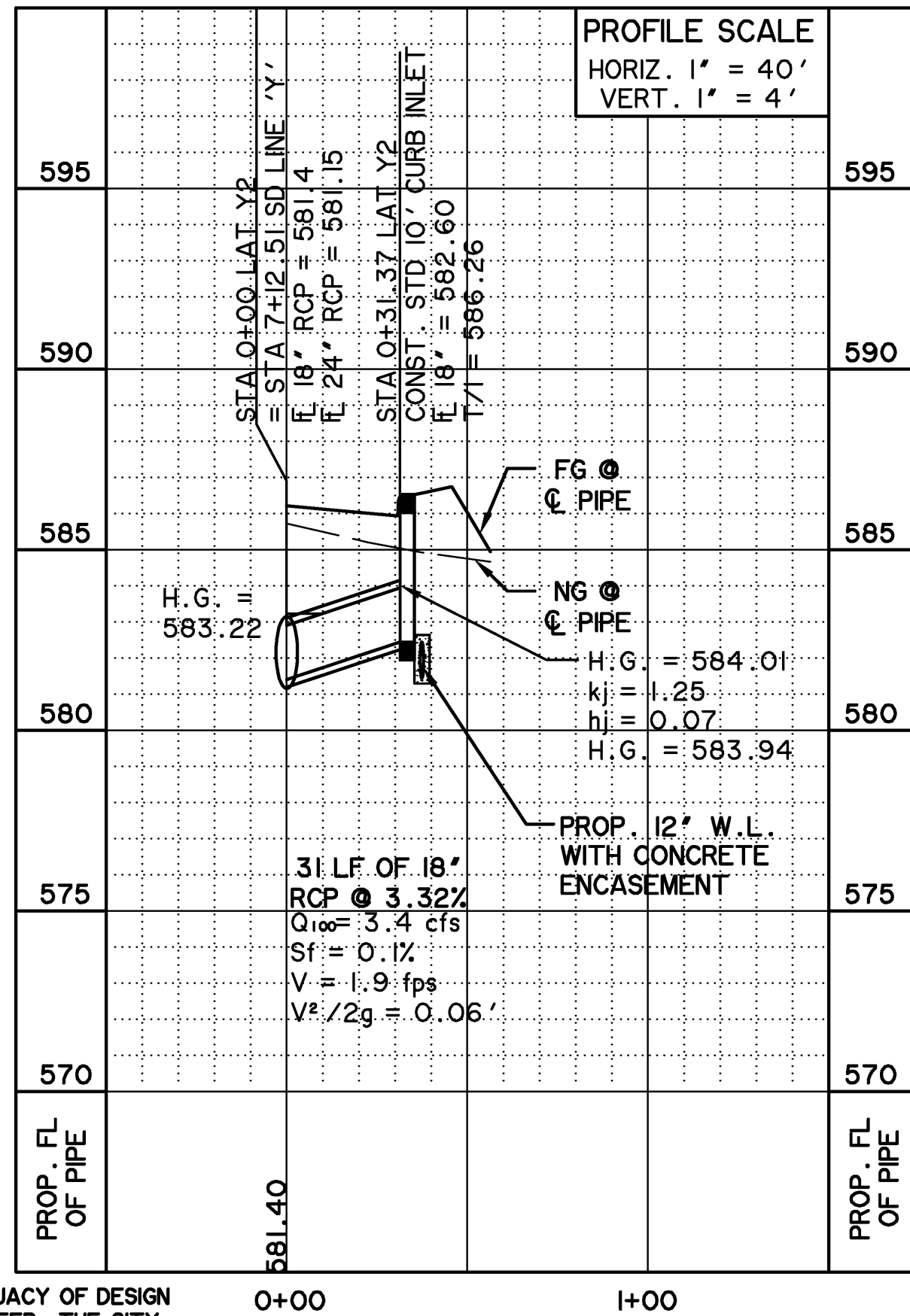
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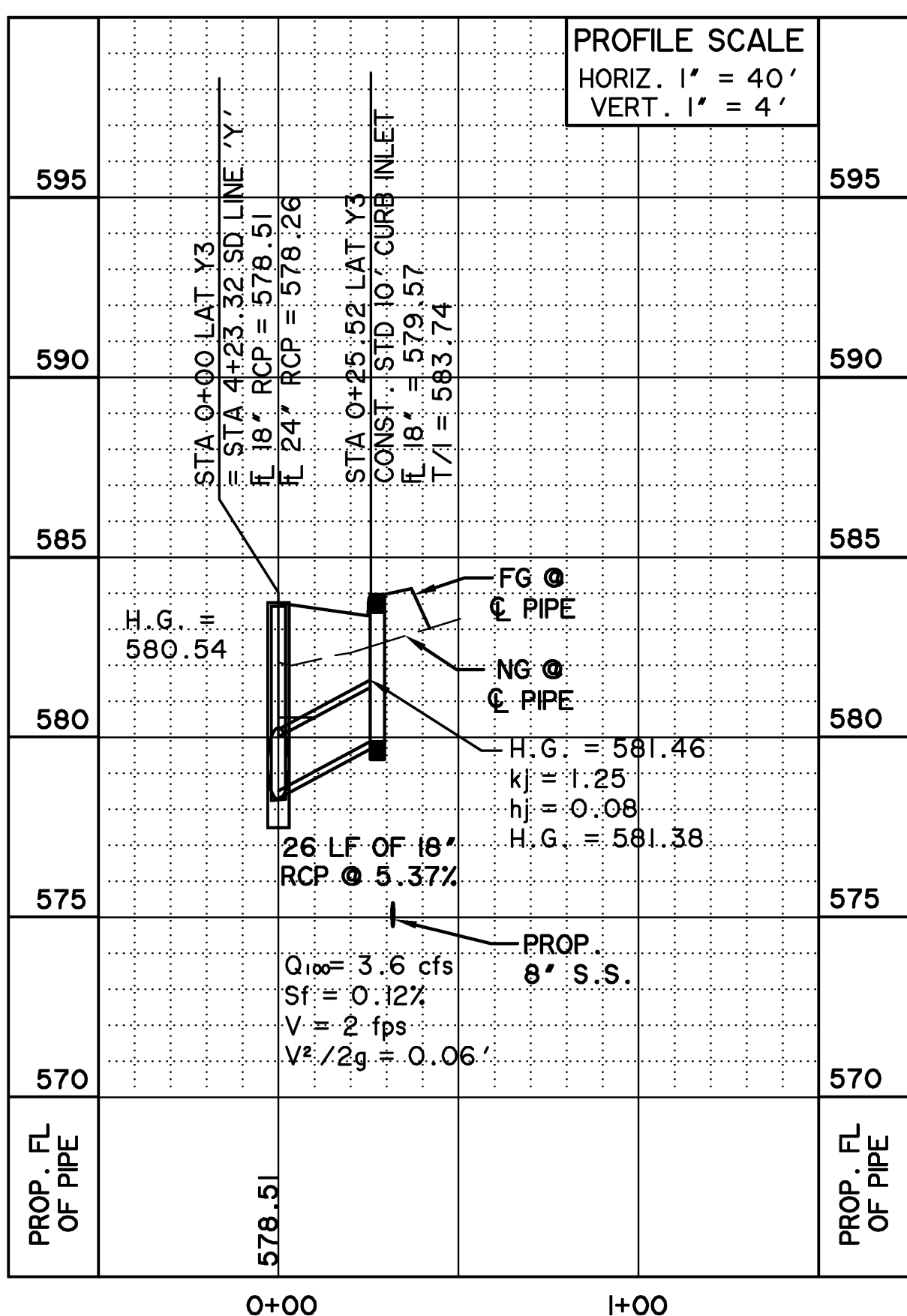
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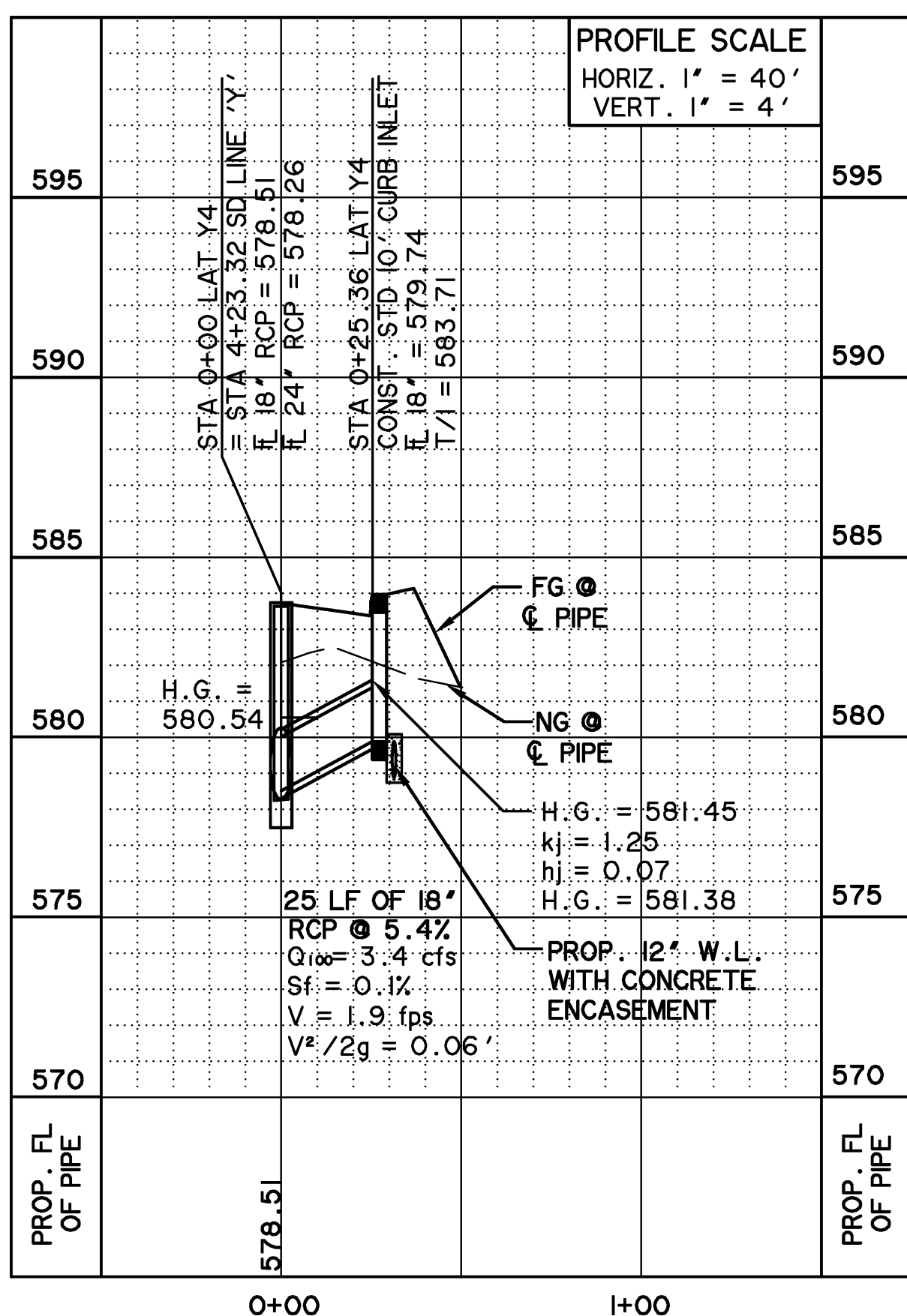
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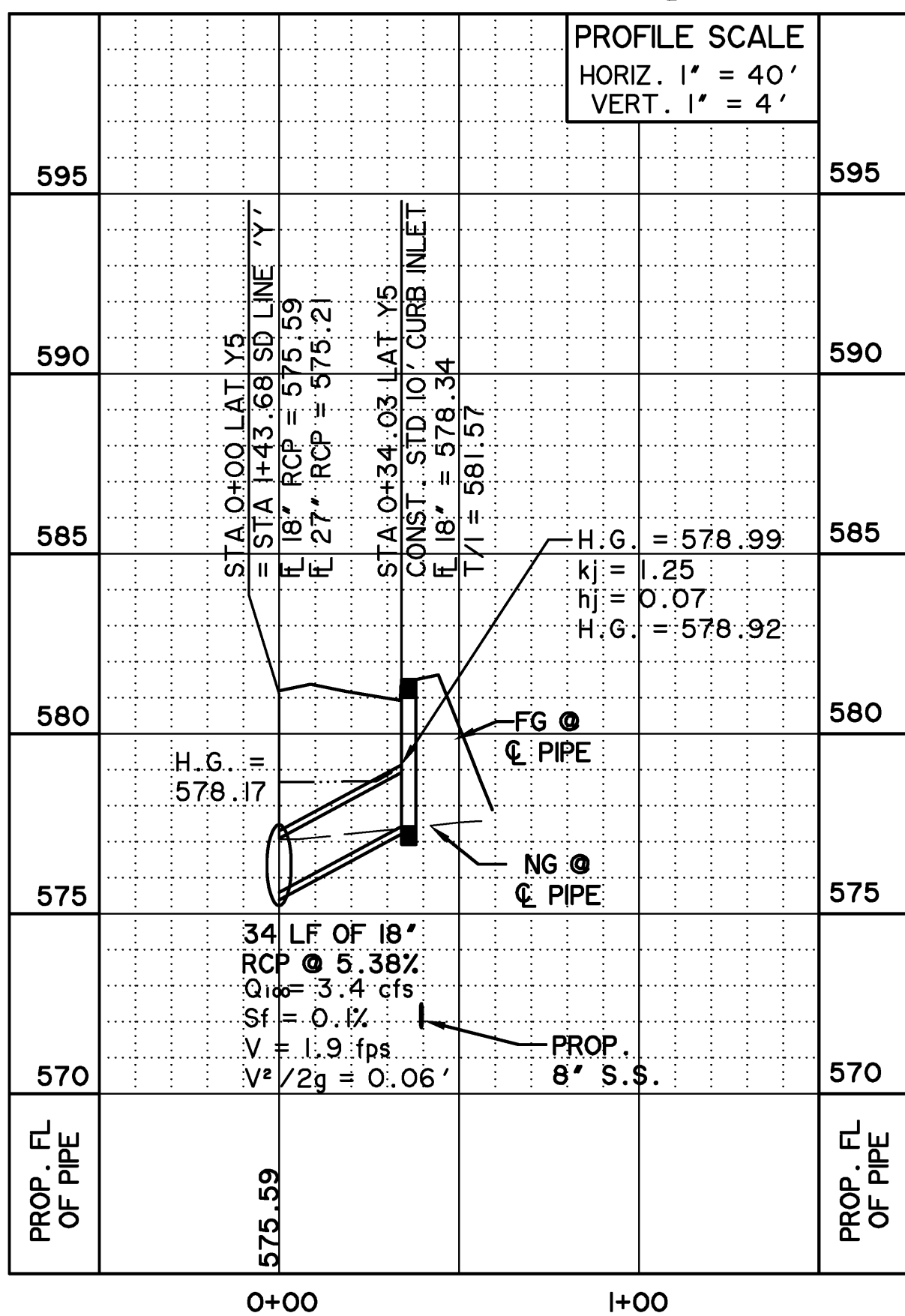
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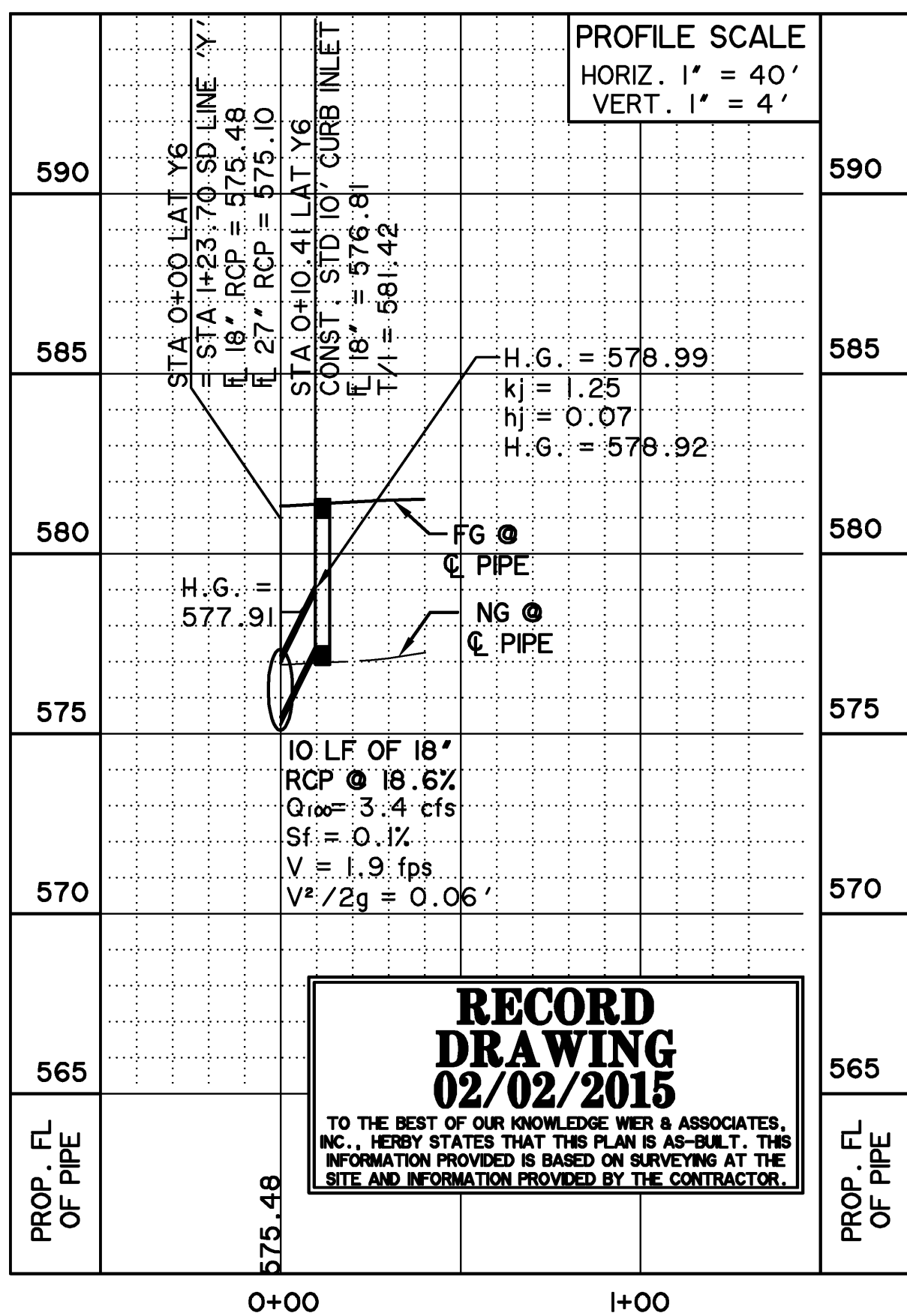
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L A T . ' Y 5 '

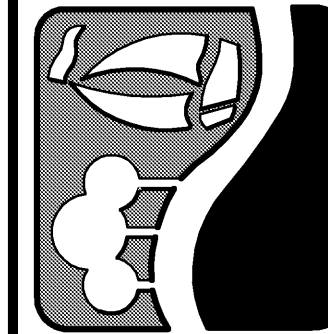


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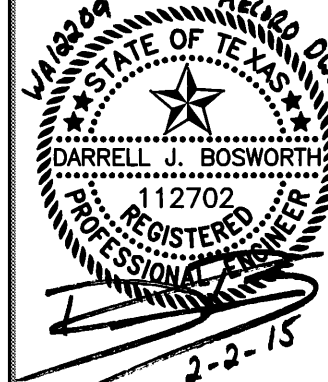
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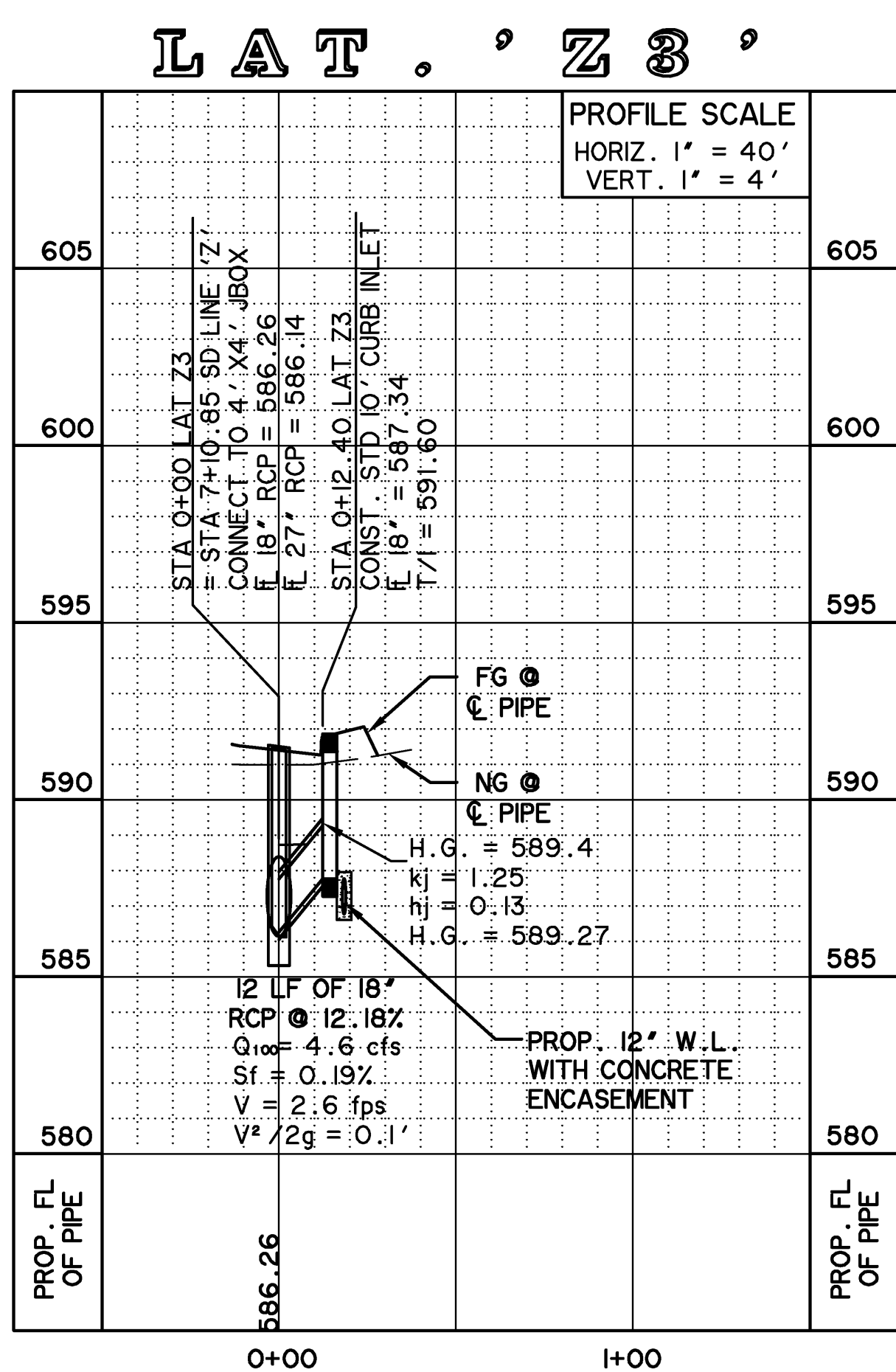
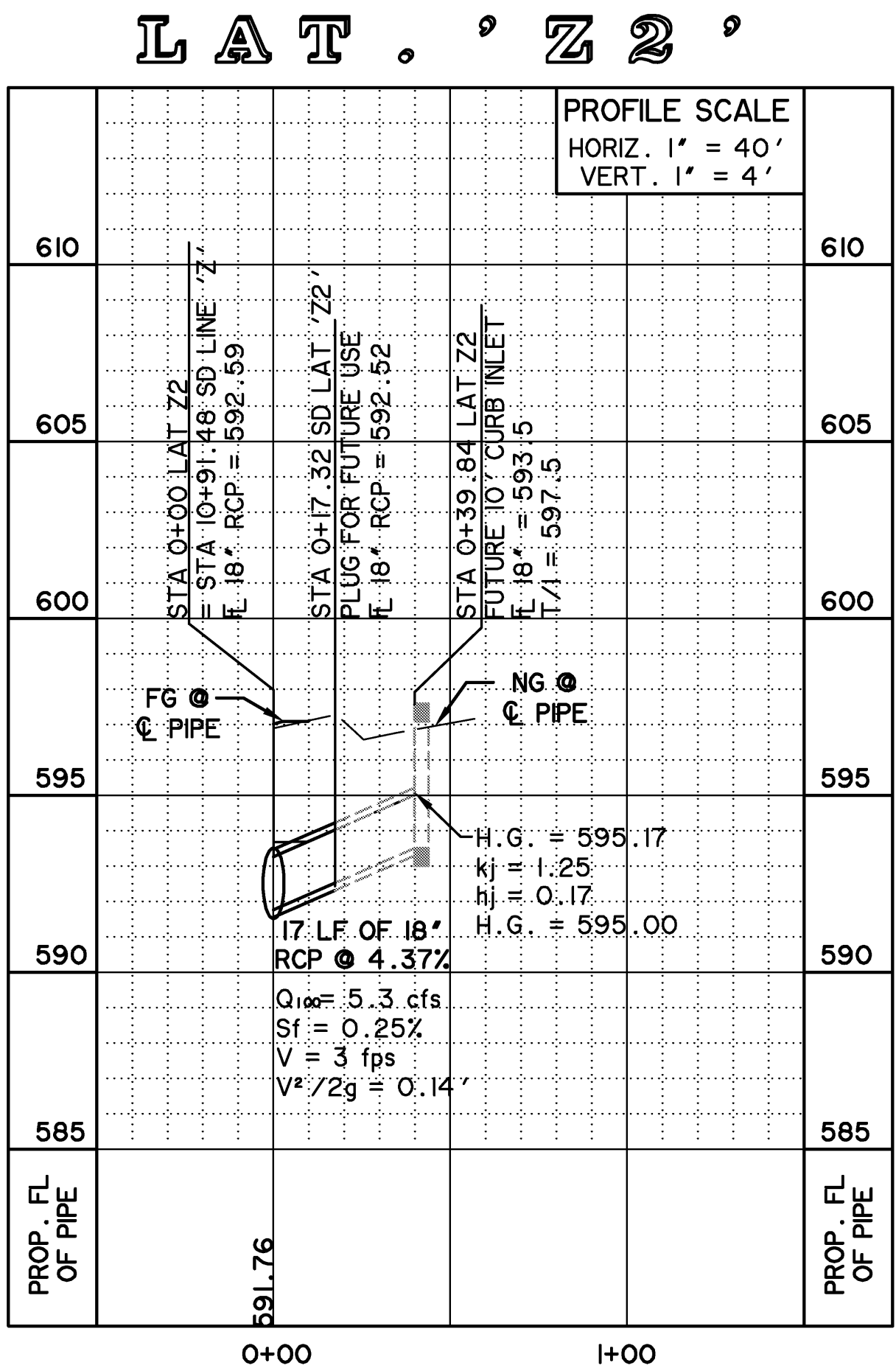
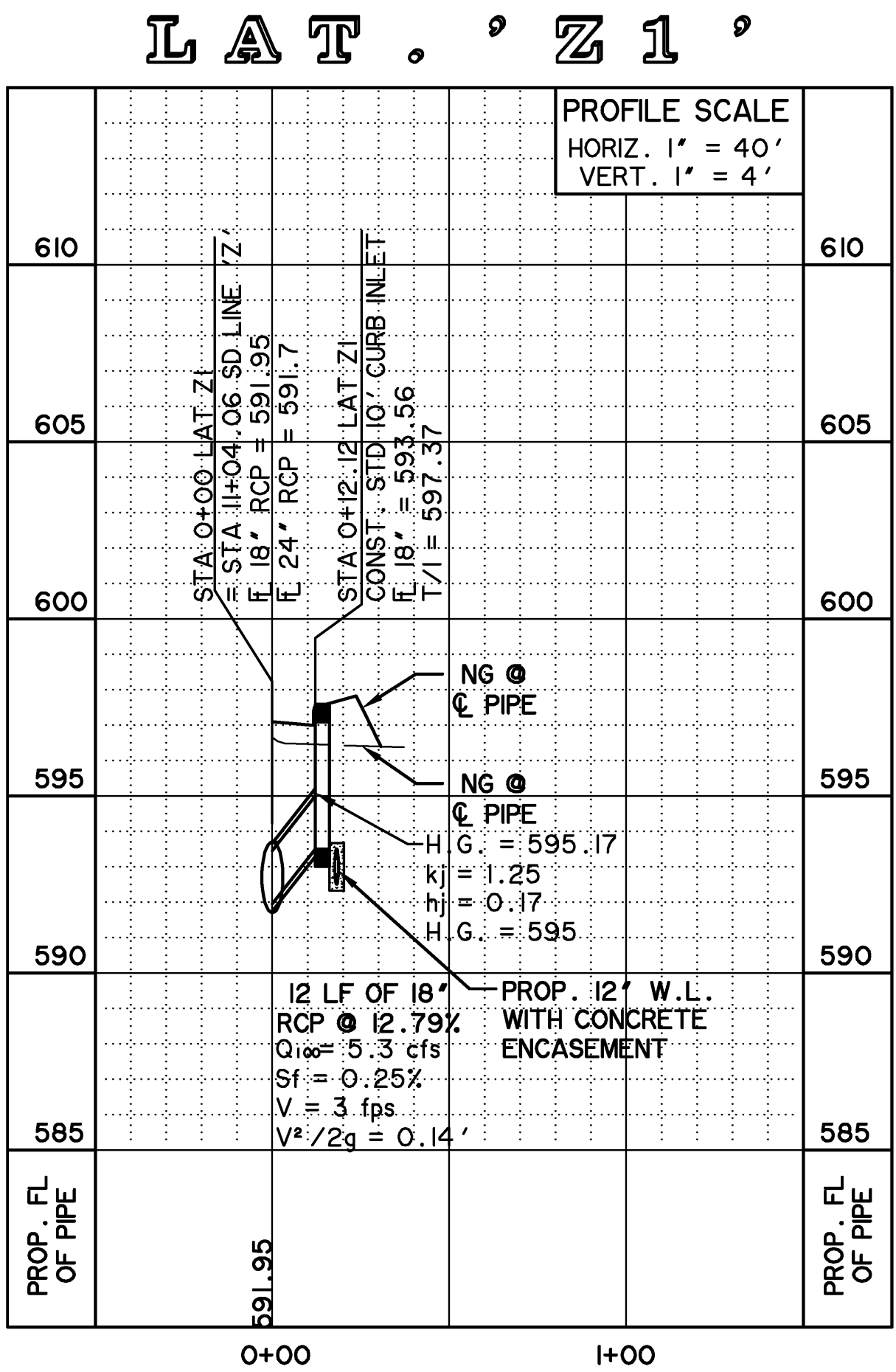
**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
LATERALS
LINES 'X' & 'Y'**

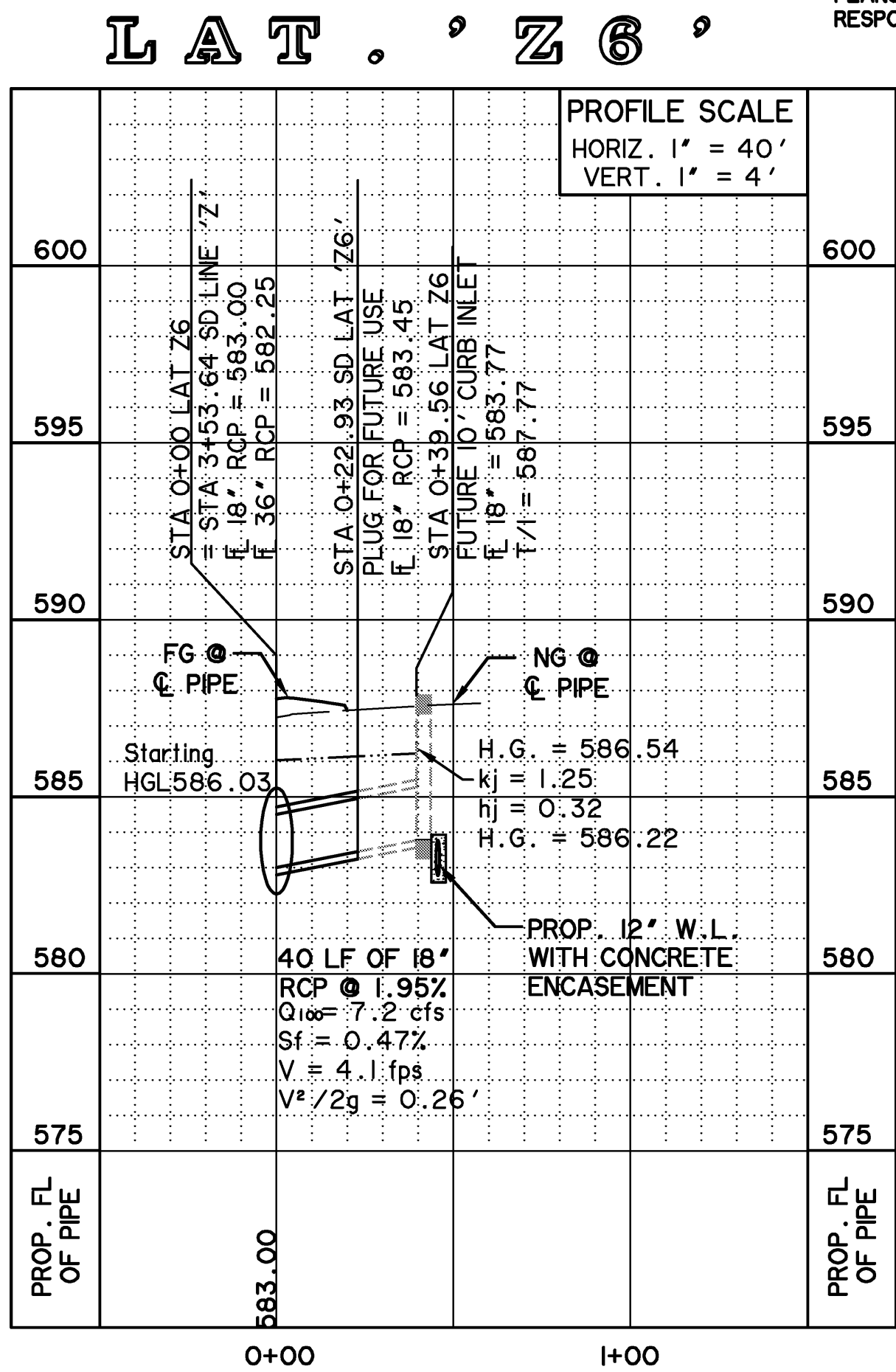
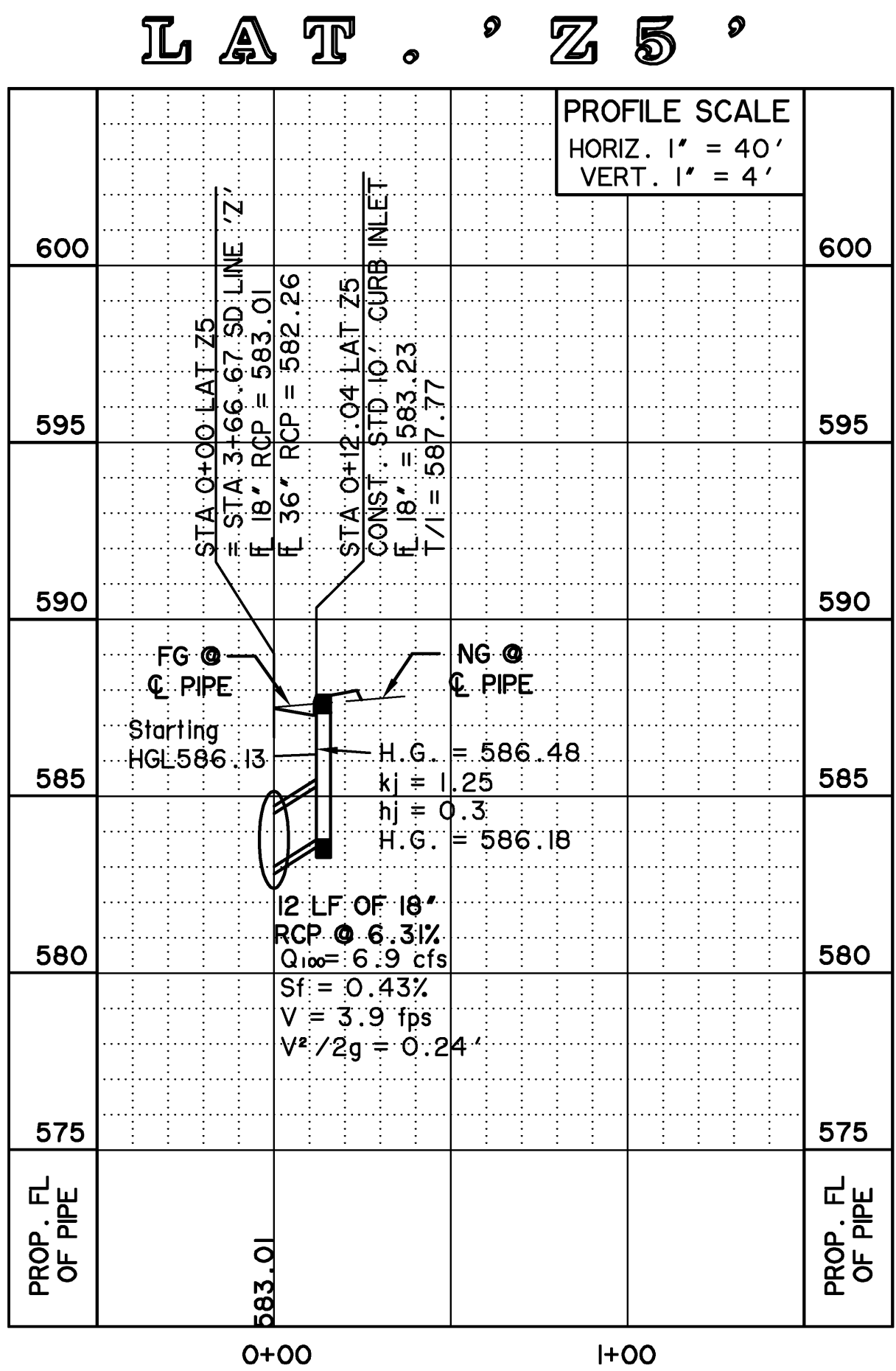
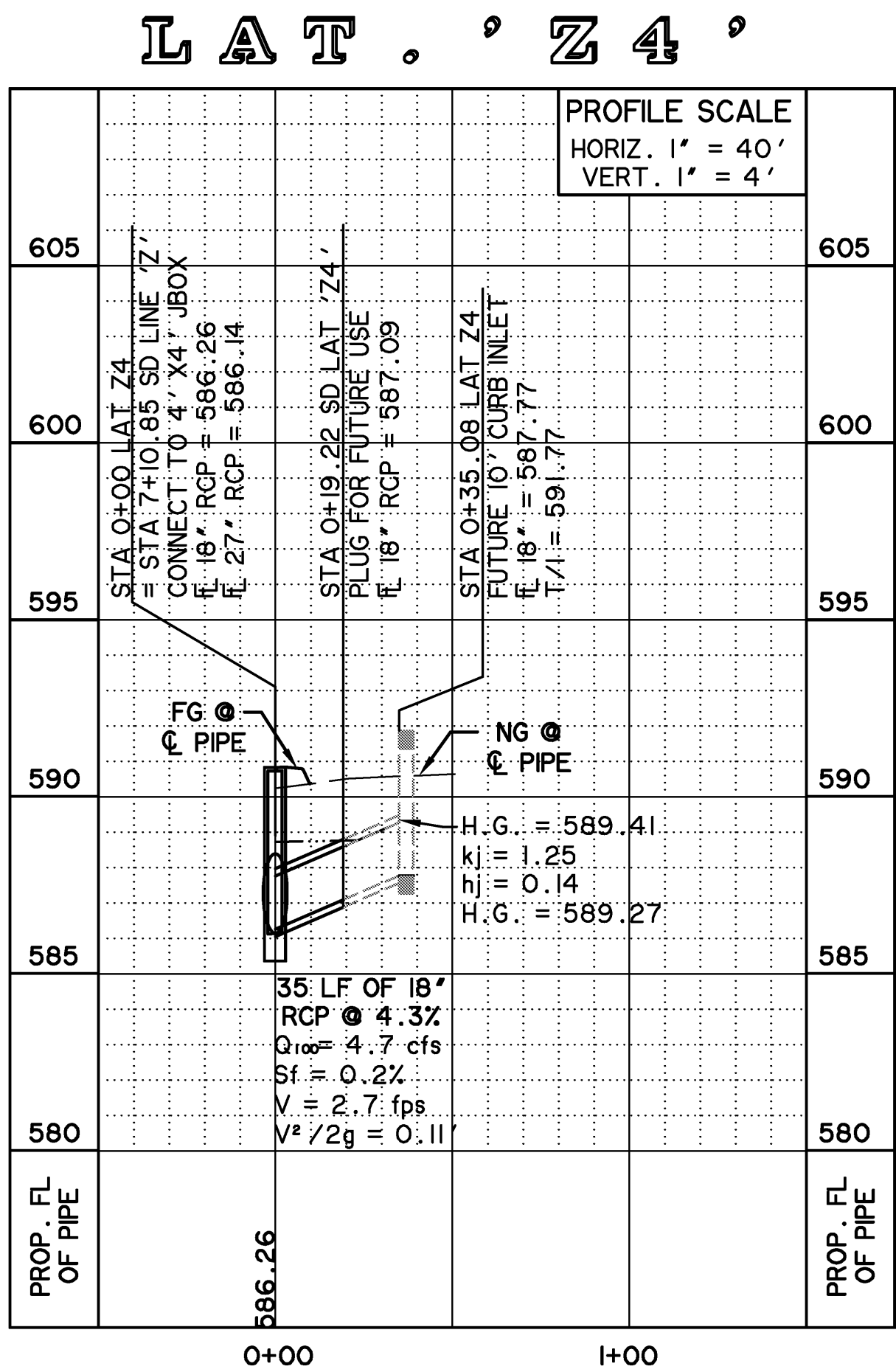


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DATE: 02-21-2015
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**SHEET NO.
D404**

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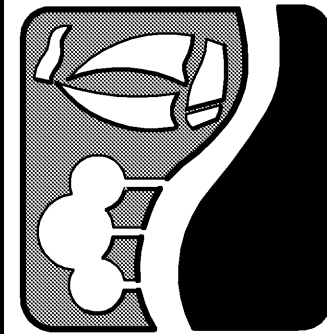


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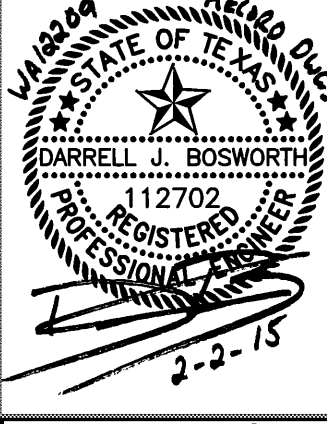
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**STORM DRAIN
LATERALS
LINE 'Z'**



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COMMENTS:

LEVELS DISPLAYED

TABLE OF DIMENSIONS & REINFORCING STEEL (Wings for One Structure End)									
Dimensions					Variable Reinforcing				Estimated Quantities per ft of wing length (2-Wings)
Maximum Wingwall Height Hw	W	X	Y	Z	Bars J1		Bars J2		
					Size	Spa	Size	Spa	Reinf (Lb/Ft) Conc (CY/Ft)
2'-6"	2'-5"	1'-0"	9"	7"	#4	1'-0"	#4	1'-0"	33.73 0.248
3'-0"	2'-5"	1'-0"	9"	7"	#4	1'-0"	#4	1'-0"	37.07 0.261
3'-6"	2'-5"	1'-0"	9"	7"	#4	1'-0"	#4	1'-0"	37.74 0.273
4'-0"	2'-5"	1'-0"	9"	7"	#4	1'-0"	#4	1'-0"	38.41 0.285
4'-6"	3'-2"	1'-6"	1'-0"	7"	#4	1'-0"	#4	1'-0"	41.75 0.330
5'-0"	3'-2"	1'-6"	1'-0"	7"	#4	1'-0"	#4	1'-0"	45.09 0.343
5'-6"	3'-2"	1'-6"	1'-0"	7"	#4	1'-0"	#4	1'-0"	45.75 0.355
6'-0"	3'-2"	1'-6"	1'-0"	7"	#4	1'-0"	#4	1'-0"	46.42 0.367
7'-0"	3'-8"	1'-9"	1'-3"	7"	#4	1'-0"	#4	1'-0"	52.77 0.414
8'-0"	4'-2"	2'-0"	1'-6"	8"	#5	1'-0"	#4	1'-0"	60.19 0.486
9'-0"	4'-8"	2'-3"	1'-9"	8"	#4	6"	#4	6"	81.49 0.535
10'-0"	5'-2"	2'-6"	2'-0"	8"	#5	6"	#4	6"	97.25 0.584
11'-0"	5'-8"	2'-9"	2'-3"	8"	#6	6"	#5	6"	133.65 0.634
12'-0"	6'-2"	3'-0"	2'-6"	9"	#7	6"	#5	6"	162.29 0.721
13'-0"	6'-8"	3'-3"	2'-9"	11"	#7	6"	#5	6"	178.80 0.856
14'-0"	7'-2"	3'-6"	3'-0"	1'-0"	#8	6"	#5	6"	216.78 0.959
15'-0"	7'-8"	4'-0"	3'-0"	1'-1"	#9	6"	#6	6"	283.06 1.068
16'-0"	8'-2"	4'-6"	3'-0"	1'-3"	#9	6"	#6	6"	297.02 1.234

TABLE OF WINGWALL REINFORCING (2-Wings)

Bar	Size	No.	Spa
D	#5	~	1'-0"
E	#4	~	1'-0"
F	#4	~	1'-0"
G	#6	4	~
M	#4	4	~
P	#4	~	1'-0"
R	#5	6	~
V	#4	~	1'-0"

TABLE OF ESTIMATED CULVERT TOEWALL QUANTITIES

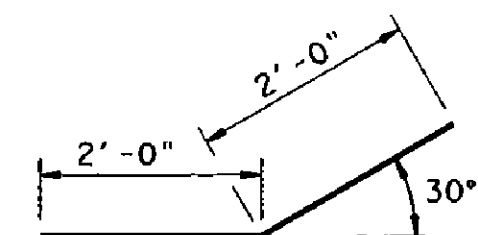
Bar	Size	No.	Spa
L	#4	~	1'-6"
Q	#4	1	~
Reinf (Lb/Ft)			2.45
Conc (CY/Ft)			0.037

WING DIMENSION CALCULATIONS:

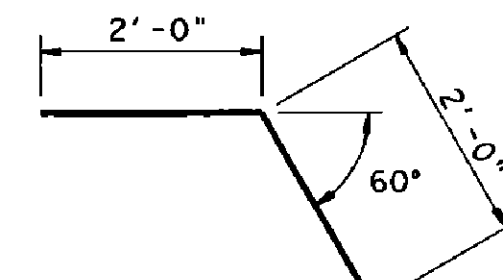
Formulas: (All values are in Feet)
 $Hw = H + T + C - 0.250'$
 $A = (Hw - 0.333') (SL)$
 $B = (A) \text{ Tangent } (30^\circ)$
 $Lw = (A) \div \text{Cosine } (30^\circ)$
 For Cast-in-place culverts:
 $Ltw = (N) (S) + (N+1) (U)$
 For Precast culverts:
 $Ltw = (N) (2U + S) + (N-1) (0.500')$
 Total Wingwall Area (Two Wings ~ S.F.) = $(Hw + 0.333') (Lw)$

Hw = Height of Wingwall
 SL:1 = Side Slope Ratio (Horizontal:1 Vertical)
 Lw = Length of Wingwall
 Ltw = Culvert Toewall Length
 N = Number of Culvert Spans

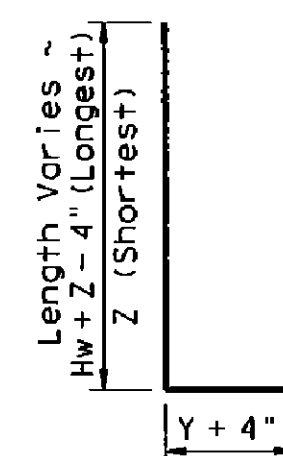
See applicable box culvert standard for H, S, T, and U values.



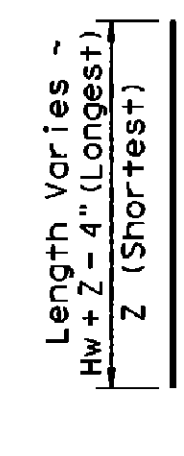
BARS D



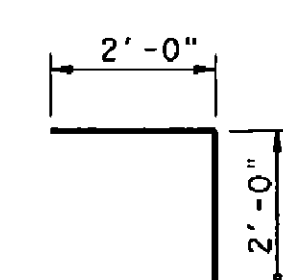
BARS R



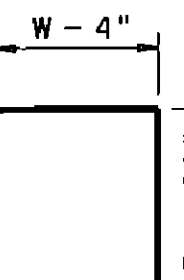
BARS J1



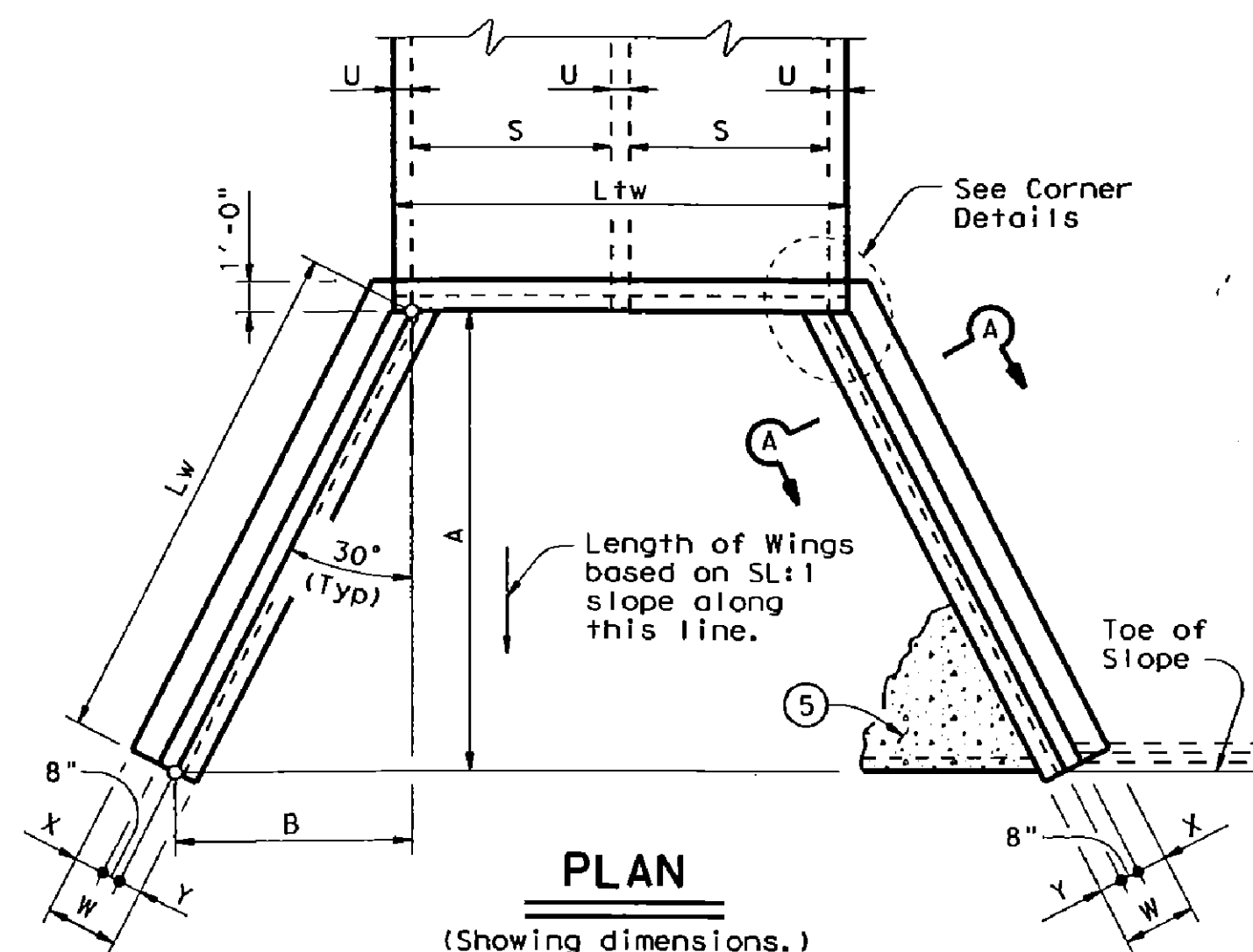
BARS V



BARS L

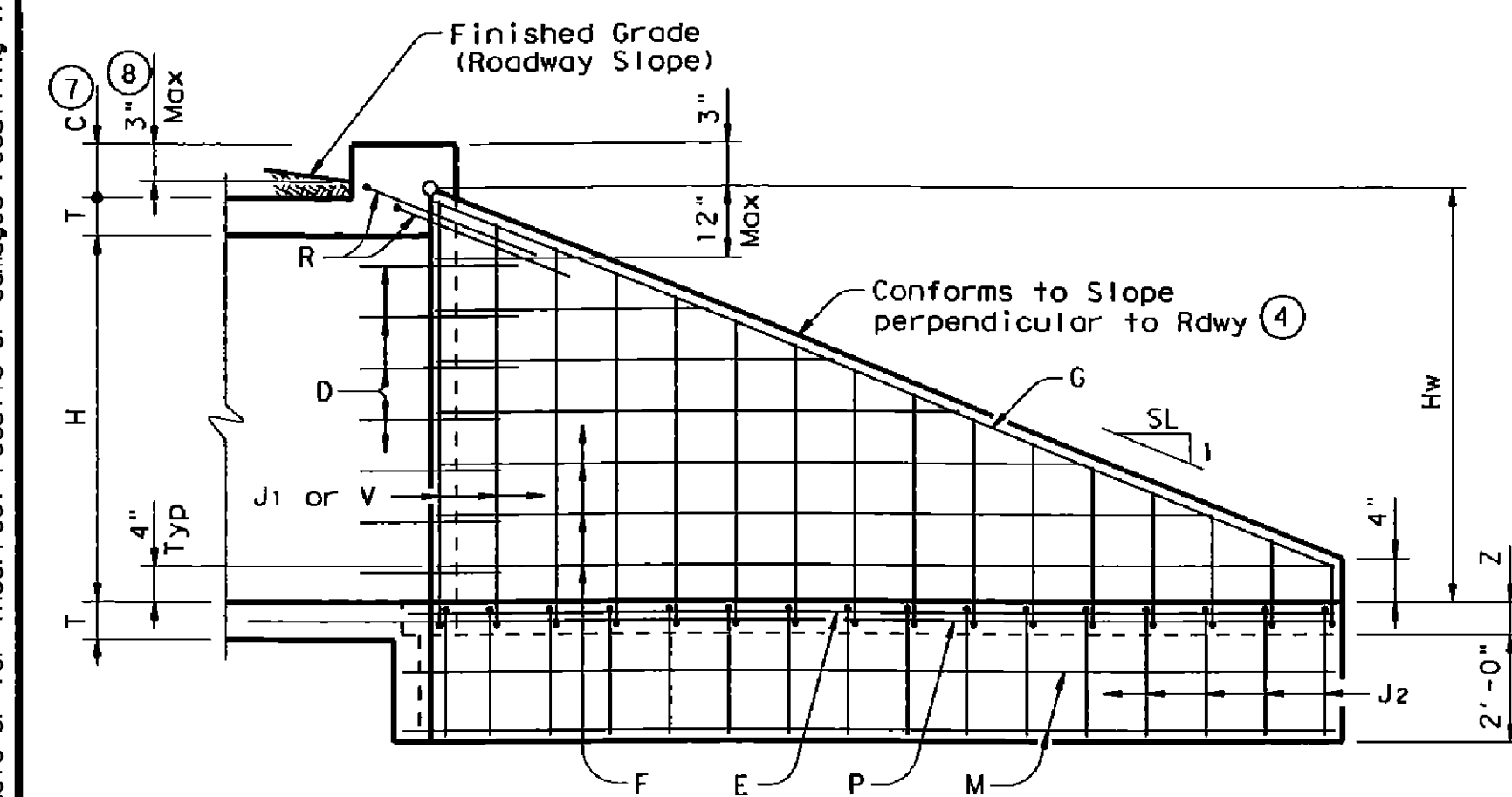


BARS J2



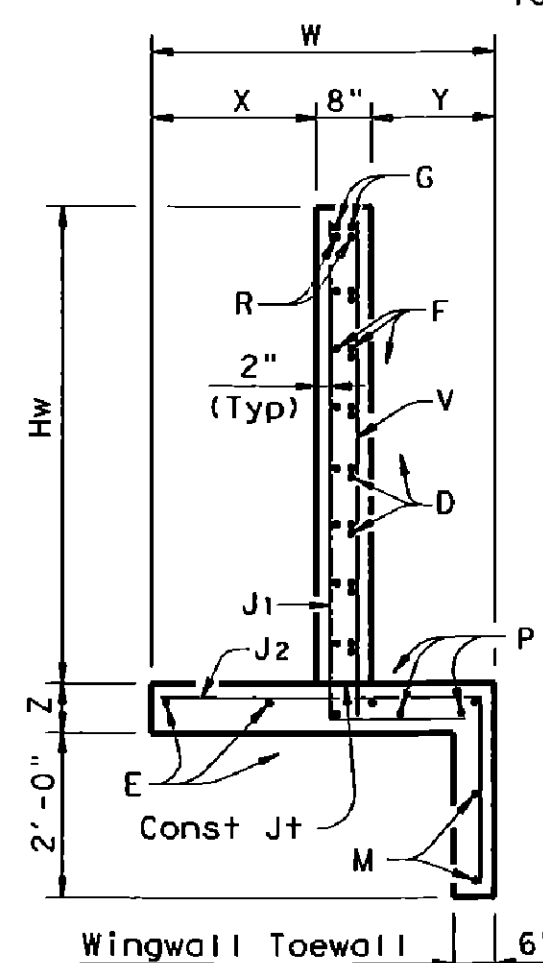
PLAN

(Showing dimensions.)

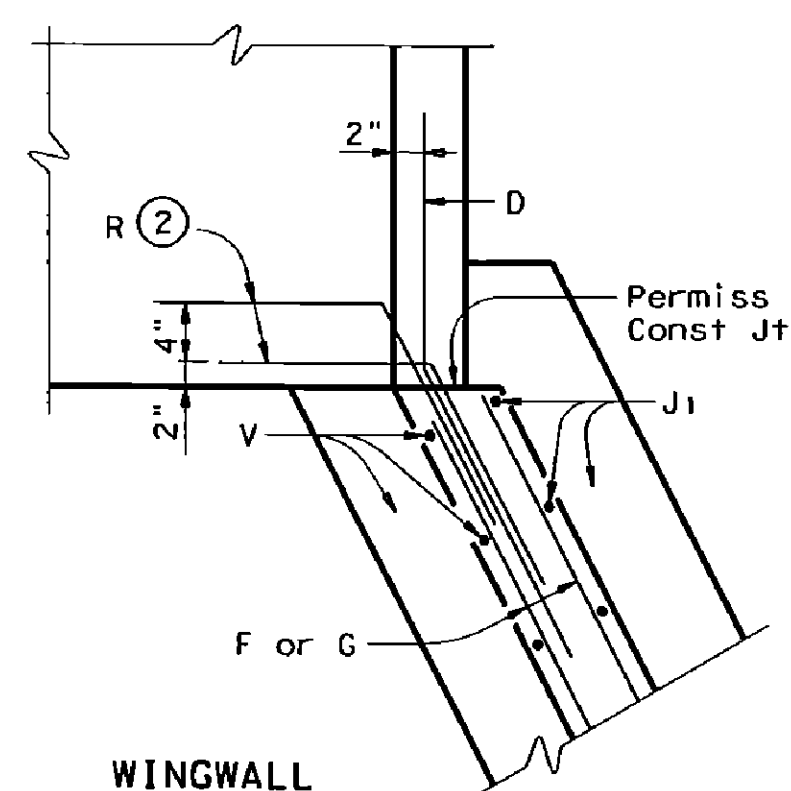


INSIDE ELEVATION

(Showing reinforcing. Culvert and Culvert Toewall reinforcing not shown for clarity.)



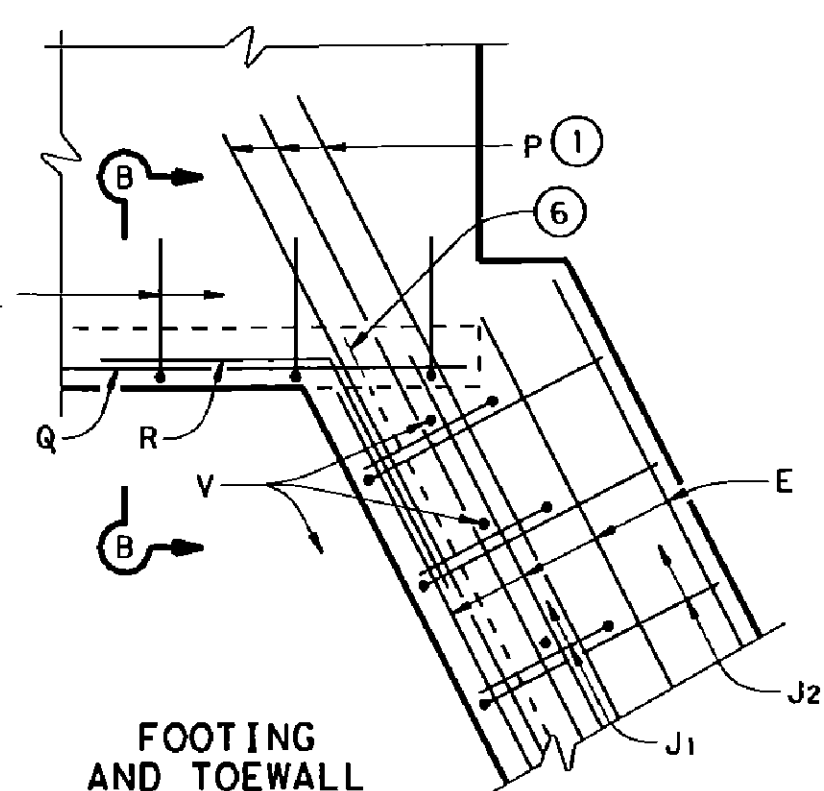
SECTION A-A



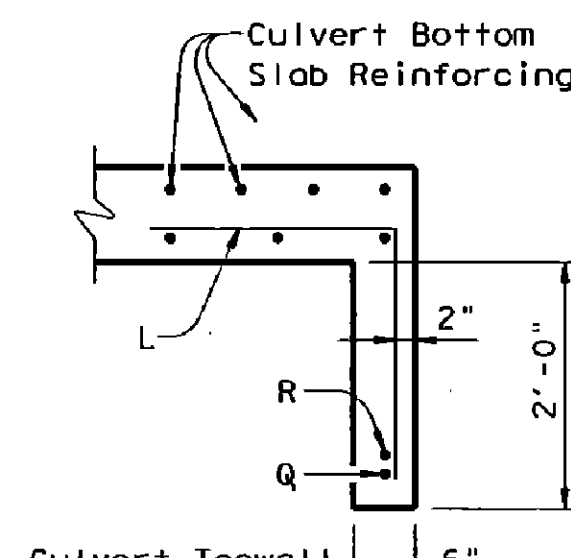
WINGWALL

CORNER DETAILS

(Culvert and Culvert Toewall reinforcing not shown for clarity.)



FOOTINGS AND TOEWALL



SECTION B-B

- Extend Bars P 3'-0" minimum into bottom slab of Box Culvert.
- Adjust to fit as necessary to maintain 1 1/4" clear cover and 4" minimum between bars.
- Quantities shown are based on an average wing height for two wings (one structure end). To determine total quantities for two wings multiply the tabulated values by Lw.
- Recommended values of Slope are: 2:1, 3:1, 4:1, & 6:1.
- When shown elsewhere on the plans, a 5" deep concrete riprap shall be constructed. Payment for riprap shall be as required by Item 432, "Riprap". Unless otherwise shown on the plans or directed by the Engineer, the riprap shall have a 6" wide by 1'-6" deep reinforced concrete toewall along all edges adjacent to natural ground; the toewall shall be reinforced by extending typical riprap reinforcing into the toewall; construction joints or grooved joints, oriented in the direction of flow, shall extend across the full distance of the riprap, at intervals of approximately 20'. When such riprap is provided, the culvert toewall shown in SECTION B-B will not be required.
- At Contractor's option, Culvert Toewall may be ended flush with Wingwall Toewall. Adjust reinforcing from that shown as necessary.
- 0" min to 5'-0" max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail, bicycle rail or curbs taller than 1'-0", refer to ECD standard. For structures with 16 bridge rail, refer to 16-CM standard. For structures with traffic rail, other than 16, refer to RAC standard.
- For vehicle safety, curb heights and wall heights shall be reduced, if necessary, to provide a maximum 3" projection above finished grade. No changes will be made in quantities and no additional compensation will be allowed for this work.

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications. All reinforcing steel shall be Grade 60. Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise. All concrete shall be Class "C" and shall have a minimum compressive strength of 3600 psi. All reinforcing bars shall be adjusted to provide a minimum of 1 1/4" clear cover. When structure is founded on solid rock, depth of toewalls for culverts and wingwalls may be reduced or eliminated as directed by the Engineer. See BCS sheet for additional dimensions and information. The quantities for concrete and reinforcing steel resulting from the formulas given on this sheet are for Contractor's information only.

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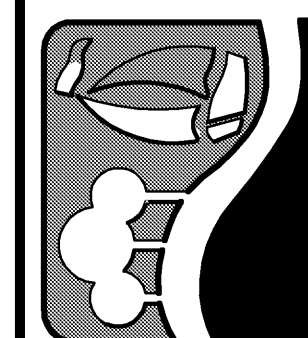
Texas Department of Transportation
Bridge Division

**CONCRETE WINGWALLS
WITH FLARED WINGS FOR
0° SKEW BOX CULVERTS**

FW-O

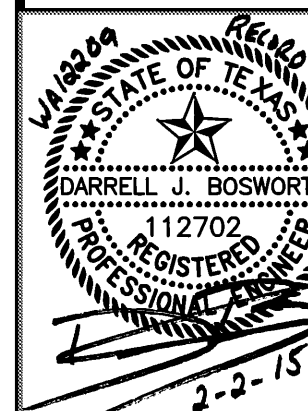
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©TxDOT February 2010	DISTRICT	FEDERAL AID PROJECT			SHEET
REVISIONS					
11-10: Add note for synthetic fibers.	COUNTY	CONTROL	SECT	JOB	HIGHWAY

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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**TXDOT CONCRETE
WINGWALLS (FW-0)**



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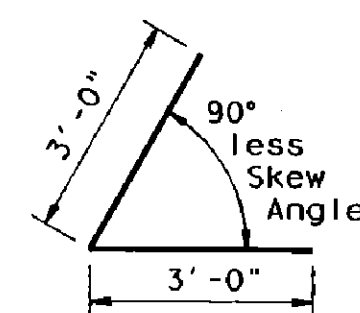
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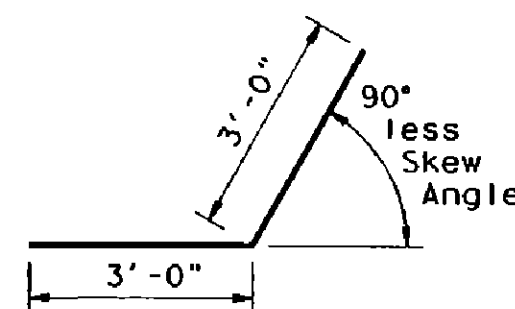
Dimensions					Variable Reinforcing		Estimated Quantities per ft of wing (2-Wings)	Estimated Quantities per ft of Toewall (1-Toewall)
Maximum Wingwall Height Hw	W	X	Y	Z	Bars J1	Bars J2	Reinf (Lb/Ft)	Reinf (Lb/Ft)
2'-6"	2'-10"	10"	1'-0"	7"	#4 1'-0"	#4 1'-0"	48.64	0.406
2'-9"	2'-10"	10"	1'-0"	7"	#4 1'-0"	#4 1'-0"	49.31	0.424
3'-0"	2'-10"	10"	1'-0"	7"	#4 1'-0"	#4 1'-0"	49.98	0.444
3'-3"	2'-10"	10"	1'-0"	7"	#4 1'-0"	#4 1'-0"	53.32	0.462
3'-6"	2'-10"	10"	1'-0"	7"	#4 1'-0"	#4 1'-0"	53.98	0.480
4'-0"	3'-2"	1'-2"	1'-0"	7"	#4 1'-0"	#4 1'-0"	55.77	0.532
4'-6"	3'-2"	1'-2"	1'-0"	7"	#4 1'-0"	#4 1'-0"	59.77	0.568
5'-0"	3'-9"	1'-7"	1'-2"	7"	#4 1'-0"	#4 1'-0"	63.45	0.632
5'-6"	3'-9"	1'-7"	1'-2"	7"	#4 1'-0"	#4 1'-0"	67.46	0.668
6'-0"	4'-4"	2'-0"	1'-4"	7"	#5 1'-0"	#5 1'-0"	80.67	0.730
6'-6"	4'-4"	2'-0"	1'-4"	7"	#5 1'-0"	#5 1'-0"	85.05	0.768
7'-0"	5'-0"	2'-3"	1'-9"	8"	#5 1'-0"	#5 1'-0"	92.15	0.864
7'-6"	5'-0"	2'-3"	1'-9"	8"	#5 1'-0"	#5 1'-0"	96.54	0.902
8'-0"	5'-6"	2'-8"	1'-10"	8"	#5 6"	#5 6"	139.04	0.962
8'-6"	5'-6"	2'-8"	1'-10"	8"	#5 6"	#5 6"	144.47	1.000
9'-0"	6'-0"	2'-10"	2'-2"	9"	#5 6"	#5 6"	156.93	1.136
10'-6"	6'-5"	3'-0"	2'-5"	9"	#6 6"	#5 6"	196.27	1.234
11'-6"	7'-2"	3'-6"	2'-8"	11"	#6 6"	#6 6"	230.13	1.438
12'-6"	7'-8"	3'-9"	2'-11"	7"	#6 6"	#6 6"	283.41	1.592
13'-6"	8'-2"	4'-0"	3'-2"	8"	#6 6"	#6 6"	348.72	1.804
14'-6"	8'-10"	4'-5"	3'-5"	9"	#6 6"	#6 6"	432.94	2.046
15'-6"	9'-6"	4'-10"	3'-8"	9"	#6 7"	#6 6"	489.52	2.302
16'-0"	9'-11"	5'-0"	3'-11"	9"	#6 7"	#6 6"	505.72	2.448

Bar	Size	No.	Spa
D1	#6	~	1'-0"
D2	#6	~	1'-0"
E1	#4	~	1'-0"
F	#4	~	1'-0"
G	#6	~	8"
M1	#4	4	~
P	#4	~	1'-0"
V	#4	~	1'-0"

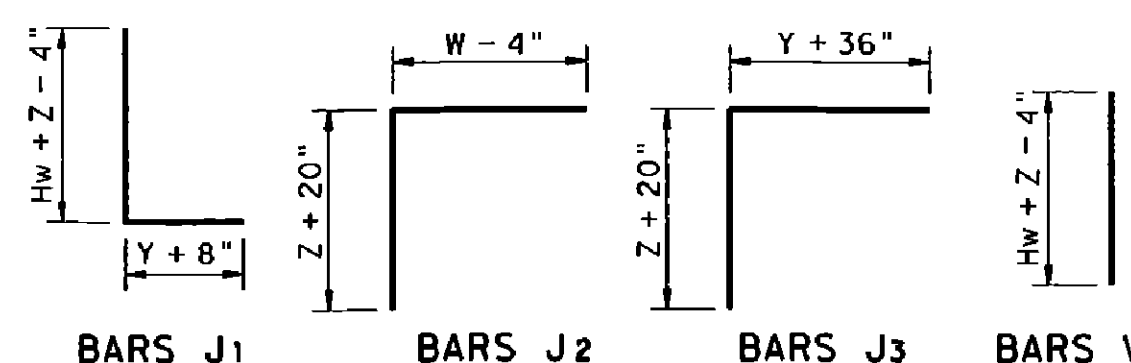
Bar	Size	No.	Spa
J3	#4	~	1'-0"
M2	#4	2	~
E2	#4	~	1'-0"



BARS D1



BARS D2



BARS J1

BARS J2

BARS J3

BARS V

WING DIMENSION CALCULATIONS:

Formulas: (All values are in Feet)

$$Hw = H + T + C$$

$$Lw = (Hw) (SL) \div \cosine \theta \text{ for Ty PW-1}$$

$$= (Hw - 1') (SL) \div \cosine \theta \text{ for Ty PW-2 and } Hw \geq 4'$$

$$= (Hw - 0.5') (SL) \div \cosine \theta \text{ for Ty PW-2 and } Hw < 4'$$

For Cast-in-place culverts:

$$Ltw = [(N) (S) + (N + 1) (U)] \div \cosine \theta$$

For Precast culverts:

$$Ltw = [(N) (2U + S) + (N - 1) (0.5')] \div \cosine \theta$$

$$\text{Total Wingwall Area (Two Wings) } \sim \text{SF}$$

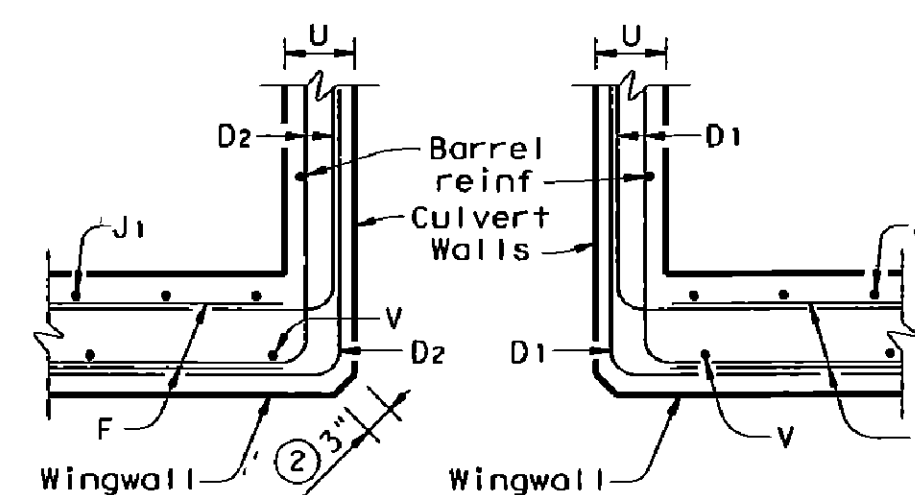
$$= (2) (Hw) (Lw) \text{ for Ty PW-1}$$

$$= (2) (Hw) (Lw) - 6 \text{ SF for Ty PW-2 and } Hw \geq 4'$$

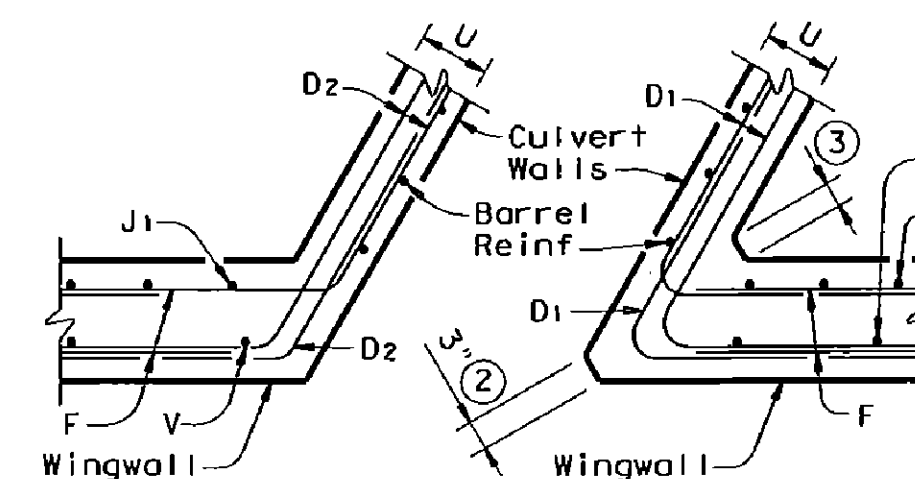
$$= (2) (Hw) (Lw) - 1.5 \text{ SF for Ty PW-2 and } Hw < 4'$$

Hw = Height of Wingwall
 Lw = Length of Wingwall
 Ltw = Culvert Toewall Length
 N = Number of Culvert Spans
 SL = Channel Slope ratio, (Horizontal: 1 Vertical, Usual value is 2:1)
 θ = Culvert Skew

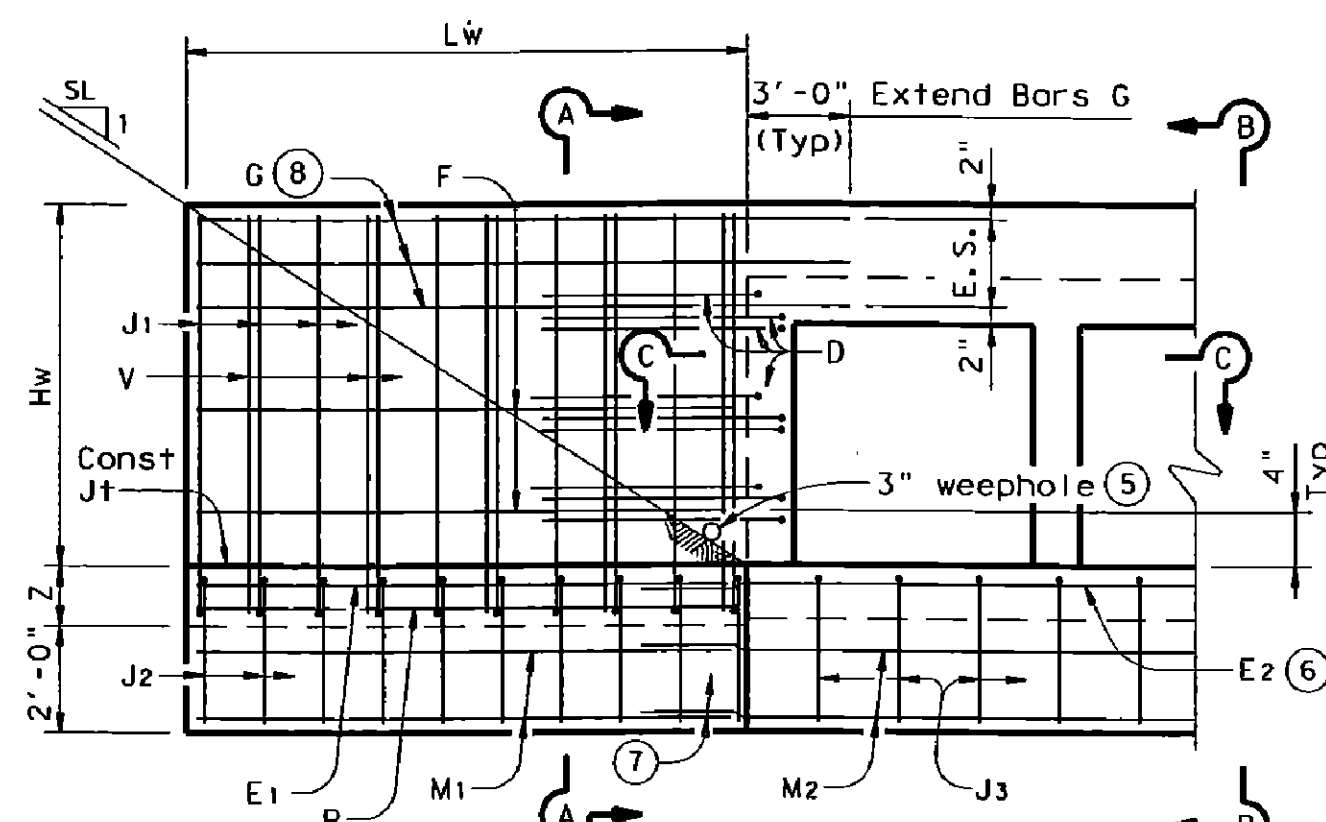
See applicable box culvert standard for S, H, T and U values.



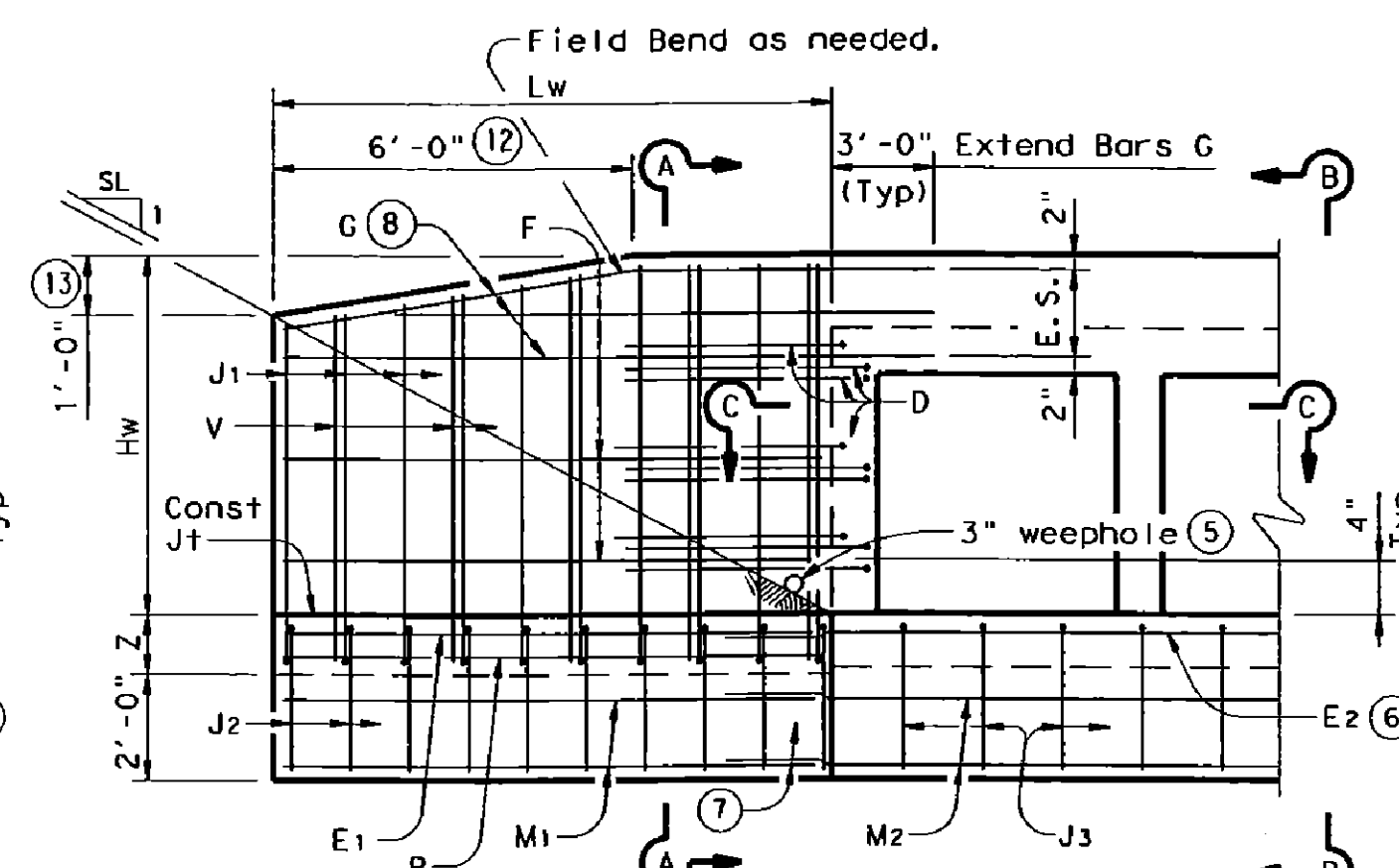
SECTION C-C



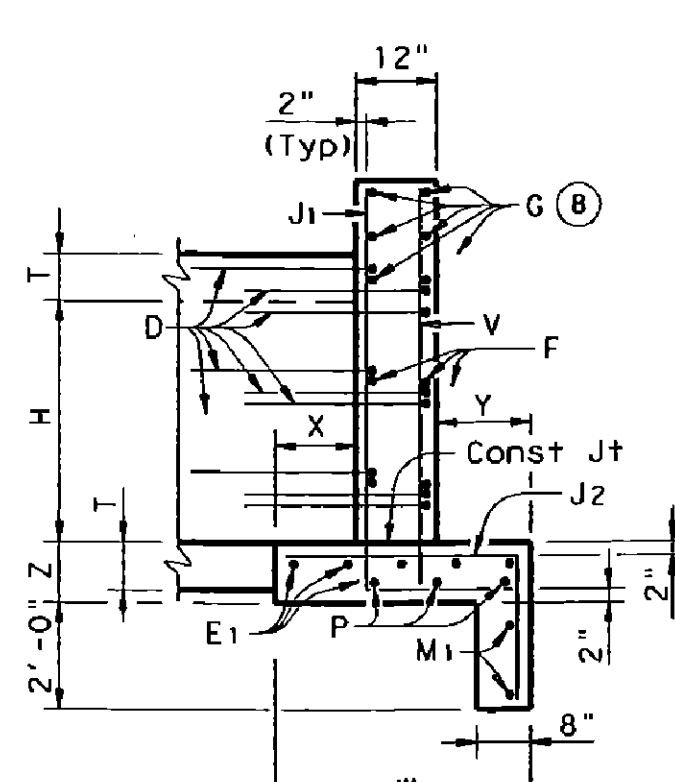
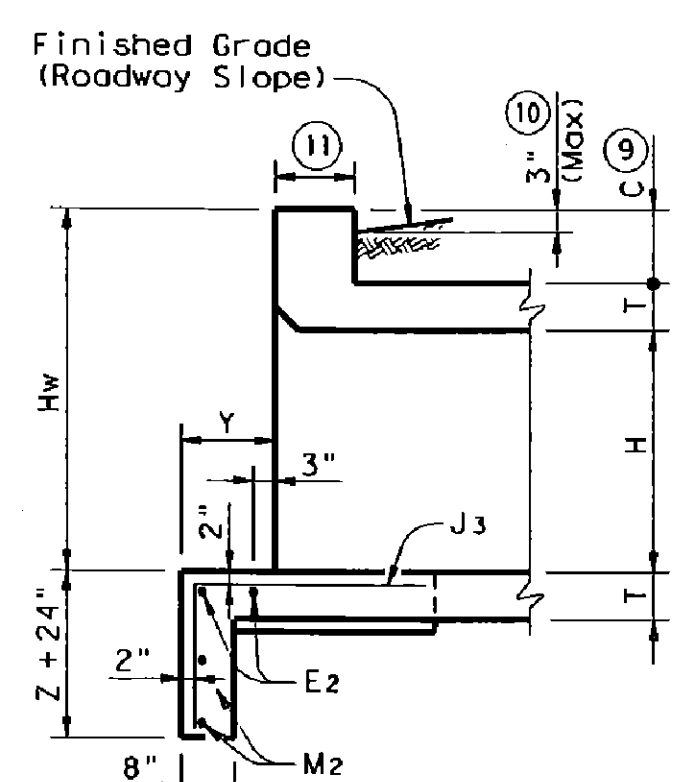
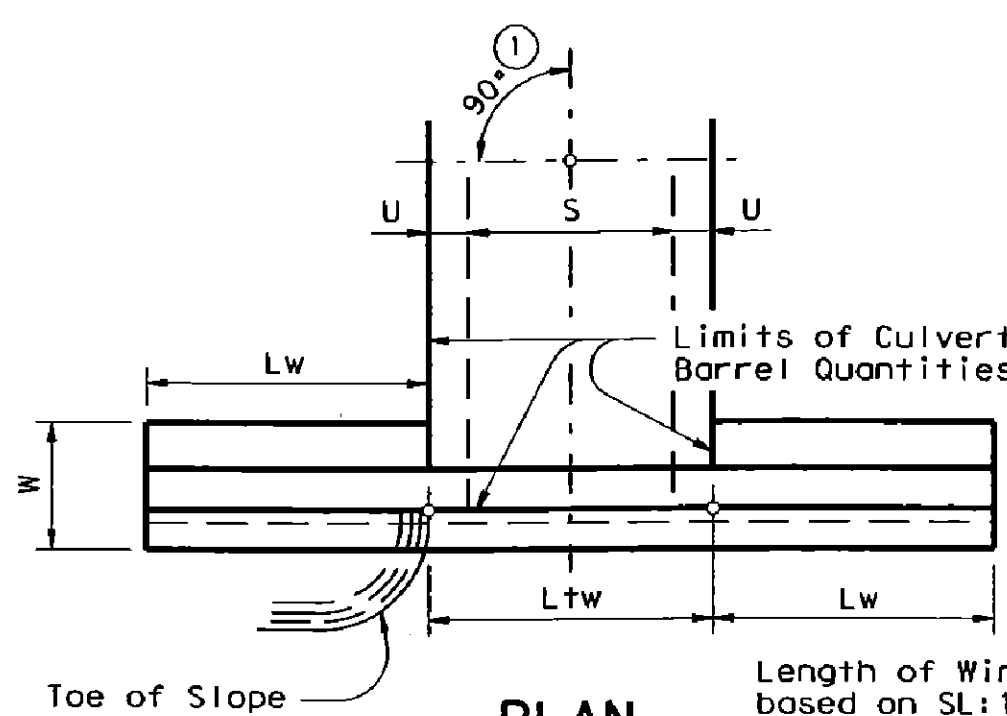
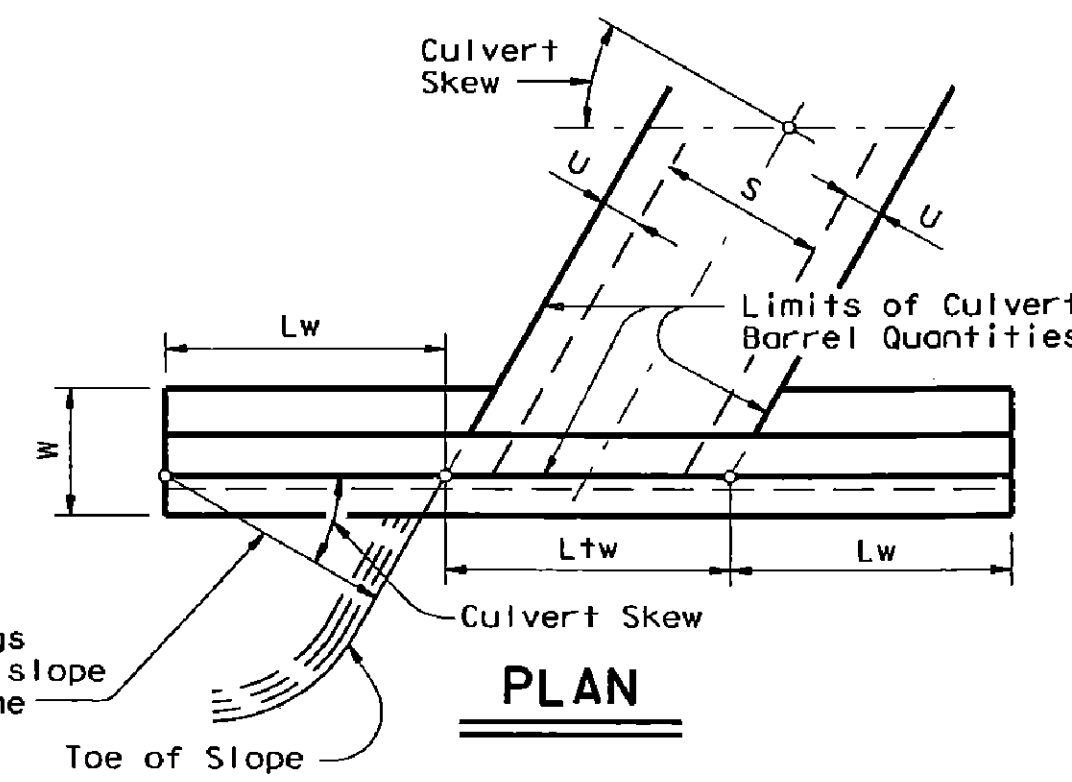
SECTION C-C



PARTIAL ELEVATION - PW-1



PARTIAL ELEVATION - PW-2

SECTION A-A
(Showing Wing Reinf)SECTION B-B
(Showing Wing Reinf)DETAILS FOR
NON-SKEWED BOX CULVERTSDETAILS FOR
SKEWED BOX CULVERTS
(Showing 30° Skew)

- Skew Angle = 0°
- At discharge end, chamfer may be 3/4".
- For 15° Skew ~ 1"
For 30° Skew ~ 2"
For 45° Skew ~ 3"
- Quantities shown are for two Type PW-1 wings. Adjust concrete volume for Type PW-2 wings. To determine estimated quantities for two wings, multiply the tabulated values by Lw. Quantities shown do not include weight of Bars D.
- Provide weepholes for Hw = 5'-0" and greater. Fill around weepholes with coarse gravel.
- Extend Bars E2 1'-6" minimum into the wingwall footing.
- Lap Bars M1 1'-6" minimum with Bars M2.
- Bars G equally spaced at 8" maximum, place as shown. Provide at least two pair Bars G per wing.
- 0" min to 5'-0" max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail, bicycle rail or curbs taller than 1'-0", refer to ECD standard. For structures with T6 bridge rail, refer to T6-CM standard. For structures with traffic rail, other than T6, refer to RAC standard.
- For vehicle safety, the following requirements must be met:
 - For structures without bridge rail, curbs cannot project more than 3" above finished grade.
 - For structures with bridge rail, build curbs flush with finished grade.
 Reduce curb heights, if necessary, to meet the above requirements. No changes will be made in quantities and no additional compensation will be allowed for this work.
- 1'-0" typical, 2'-0" typical when RAC standard is referenced elsewhere in the plans.
- 3'-0" for Hw < 4'.
- 6" for Hw < 4'.

GENERAL NOTES:

Designed in accordance with AASHTO LRFD Bridge Design Specifications.

Provide Class "C" Concrete (f'c = 3,600 psi Min) and Grade 60 reinforcing steel.

Provide 1/4" Min clear cover to reinforcing steel. Depth of toewalls for wingwalls and culverts may be reduced or eliminated when founded on solid rock, when directed by the Engineer.

See BCS sheet for wingwall type and additional dimensions and information.

The quantities for concrete and reinforcing steel resulting from the formulas given on this sheet are for the Contractor's information only.

DESIGNER NOTES:

Type PW-1 can be used for all applications and must be used if railing is to be mounted to the wingwall.

Type PW-2 can only be used for applications without a railing mounted to the wingwall.

Texas Department of Transportation
Bridge DivisionCONCRETE WINGWALLS
WITH PARALLEL WINGS FOR
BOX CULVERTS
TYPES PW-1 AND PW-2

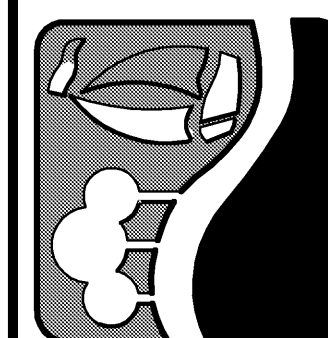
PW

FILE: dwstde01.dgn	DW: GAF	CK: CAT	DW: TXDOT	CK: GAF
DISTRICT	FEDERAL AID PROJECT	SHEET		
COUNTY	CONTROL SECT	JOB	HIGHWAY	

RECORD
DRAWING
02/02/2015

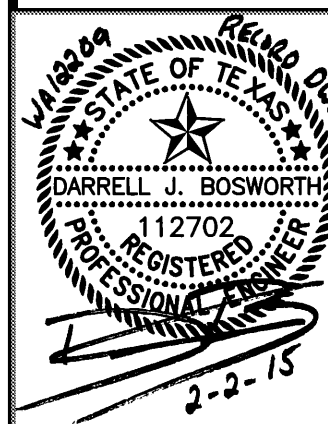
TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

PREPARED BY:
WIER & ASSOCIATES, INC.
 ENGINEERS SURVEYORS LAND PLANNERS
 701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
 Texas Firm Registration No. F-2776 www.WierAssociates.com



ROCKWALL
 TECHNOLOGY
 PARK
 PHASE IV

TXDOT CONCRETE
 WINGWALLS (PW)



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 LAST SHEET EDIT
 DATE: 06-26-2013
 WA# 12209

SHEET NO.
 D502



Approximate Weight = 245 lb



Connecting pipes should enter within 10° of normal to inlet wall. If necessary, pipe elbow or curved approach alignment should be used to stay within this limit.

Unless otherwise shown in the plans, payment will be made for each manhole of the type M.

When approved, precast inlets with equivalent structural capacity may be furnished. Sealed engineering calculations and drawings shall be submitted for approval prior to construction.

Shop drawings will not be required.

In areas of conflict between reinforcing steel, blockouts, pipes, anchor bolts or other reinforcing steel, the reinforcement shall be bent or adjusted to clear as directed by the Engineer.

The riser may be constructed of reinforced concrete as shown or of Reinforced Concrete Pipe, Class III, in accordance with ASTM Designation C-76. If pipe is used, joints shall conform to the Item "Reinforced Concrete Pipe Culverts". Precast Concrete Lift-Off Cover may be substituted for Ring and Cover.

The riser, either cast-in-place or concrete pipe, may be located in any corner.

All reinforcing steel shall be #4 unless otherwise noted.

Pipes may enter any or all walls. The maximum size of pipe that can be accommodated is 60". More than one pipe may enter a side, subject to the maximum box dimension shown. The clear distance between adjacent pipes should be 9" minimum.

ring and cover shall conform to the requirements of AASHTO M306, "Standard Specification for Drainage Structure Castings". Materials shall conform to ASTM A48, Class 35B for gray iron castings or ASTM A536, Grade 65-45-12 for ductile iron castings. Aluminum alloy castings shall not be permitted.

All concrete shall be Class "A" (f'c = 3000 psi).

All concrete shall be Class A ($f_c = 3000$ psi).



MANHOLE TYPE M
(JUNCTION BOX WITH ACCESS)
15' MAX FILL

MH-M

FILE: mh-mstde.dgn	DN: TxDOT	CK: TER	DN: MCB	CK:TER/GAF
© TxDOT February 2010	DISTRICT	FEDERAL AID PROJECT		
REVISIONS				SHEET
	COUNTY	CONTROL	SECT	JOB HIGHWAY

ROCKWALL TECHNOLOGY PARK PHASE IV

**TXDOT MANHOLE
TYPE M (MH-M)**



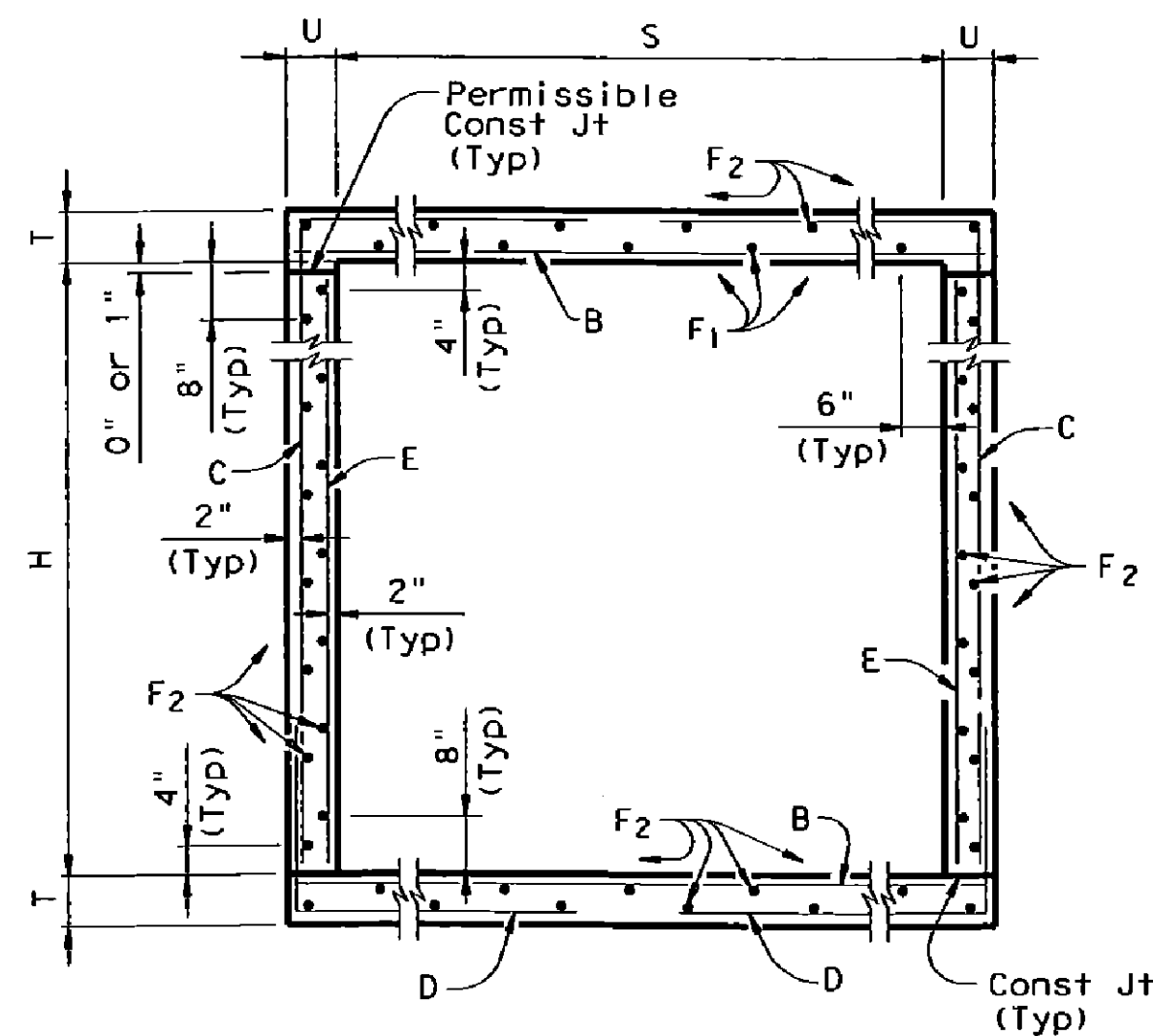
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DATE 09-23-2013
WA# 12209

SHEET NO.
D503

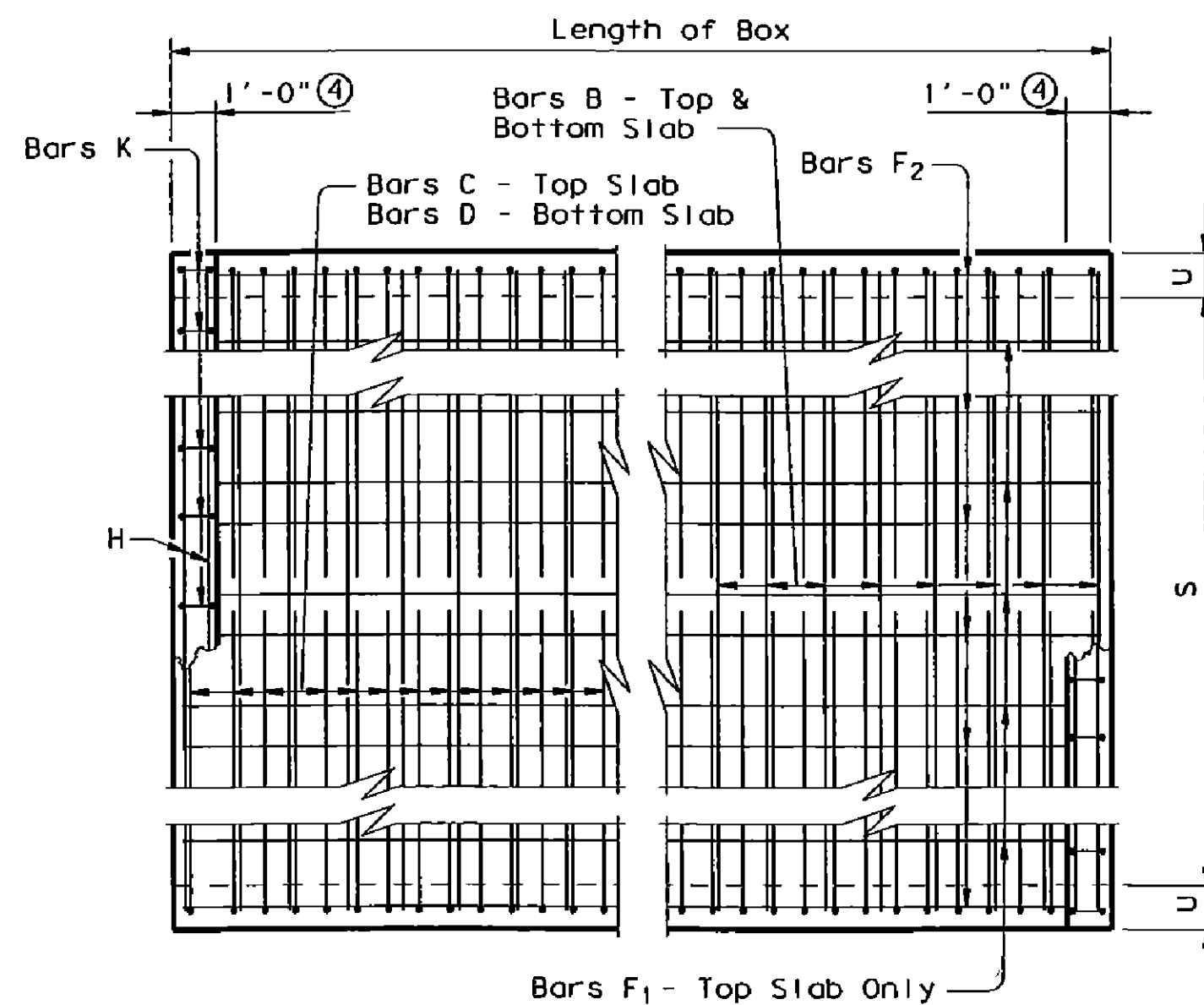
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS AND LAND PLANNERS
 PREPARED BY:
 701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817) 467-7700
 Registration No. T-2776 www.WierAssociates.com
 Texas Firm

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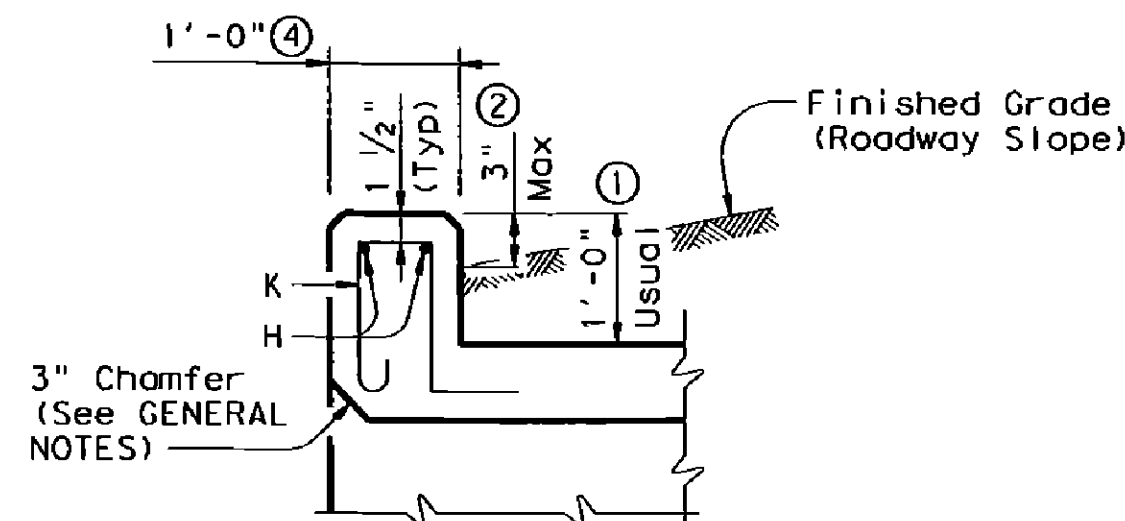
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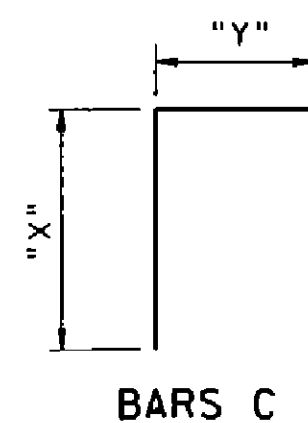
TYPICAL SECTION



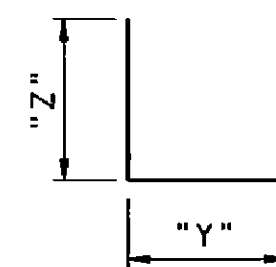
PLAN OF REINF STEEL



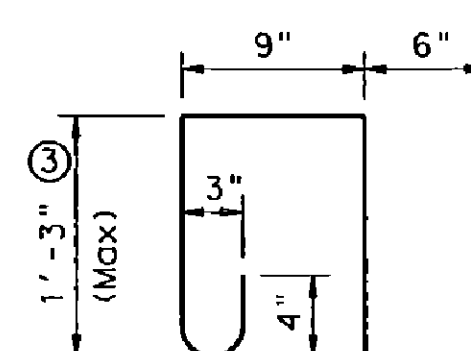
SECTION THRU CURB



BARS C



BARS D

BARS K ~ #4
(Spa = 1'-0" Max)
(Length = 4'-3")

- 0" min to 5'-0" max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail, bicycle rail or curbs taller than 1'-0", refer to ECD standard. For structures with T6 bridge rail, refer to T6-CM standard. For structures with traffic rail, other than T6, refer to RAC standard.
- For vehicle safety, the following requirements must be met:
 - For structures without bridge rail, curbs shall project no more than 3" above finished grade.
 - For structures with bridge rail, curbs shall be flush with finished grade.
 Curb heights shall be reduced, if necessary, to meet the above requirements. No changes will be made in quantities and no additional compensation will be allowed for this work.
- For curbs less than 1'-0" high, tilt bars K or reduce bar height as necessary to maintain cover. For curbs less than 3" high, bars K may be omitted.
- 1'-0" typical. 2'-0" when RAC standard is referred to elsewhere in the plans.

Deformed welded wire reinforcement (WWR) meeting the requirements of ASTM A1064 may be used to replace conventional reinforcement shown at the Contractor's option. The area of required reinforcement may be reduced by the ratio of 60 ksi / 70 ksi. Spacing of WWR is limited to 4" Min and 18" Max. When required, provide lap splices in the WWR of the same length required for the equivalent bar size, rounded up for wire sizes between conventional bar sizes.

Example Conversion: Replacement of No. 6 Gr 60 at 6" Spacing with WWR.
 WWR required = $(0.44 \text{ sq in/ } 0.5') \times (60 \text{ ksi}/70 \text{ ksi}) = 0.754 \text{ sq in/ft}$.
 If D30.6 wire is used to meet the 0.754 sq in/ft requirement in this example, the required spacing = $(0.306 \text{ sq in/ } 0.754 \text{ sq in/ft}) \times 12 \text{ in/ft} = 4.87"$ Max spacing.
 Required lap length for the provided D30.6 wire is 2'-2" (Lap required for uncoated No. 5 bars, as shown in Item 440).

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications. Designed to the maximum fill height shown.
 All reinforcing steel shall be Grade 60.
 All concrete shall be Class "C" with these exceptions: use Class "S" for top slabs of culverts with overlay, with 1-to-2 course surface treatment, or with the top slab as the final riding surface.
 Class "C" concrete shall have a minimum compressive strength of 3,600 psi. Class "S" concrete shall have a minimum compressive strength of 4,000 psi.
 The use of permanent forms is not allowed.
 The bottom edge of the top slab shall be chamfered 3" at the entrance.
 Reinforcing bars shall be adjusted to provide a minimum of 1 1/4" clear cover.
 Construction joints shown at the flow line may be raised a maximum of 6" at the Contractor's option. If this option is used, Bars E may be cut off or raised, and Bars C and D may be reversed.
 See standard SCC-MD for skewed ends, angle sections and lengthening details.

**RECORD
DRAWING
02/02/2015**

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HL93 LOADING

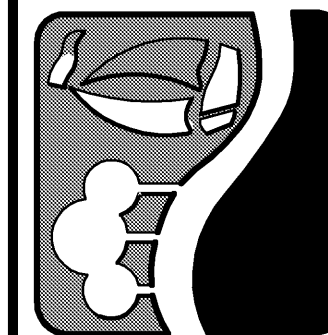
SHEET 1 OF 2

Texas Department of Transportation
Bridge Division

**SINGLE BOX CULVERTS
CAST-IN-PLACE
0' TO 30' FILL**

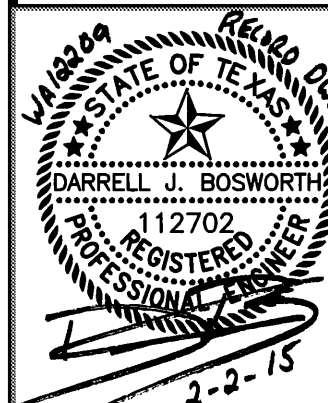
SCC-7

FILE: scc07ste.dgn	DW: GAF	CK: LMW	DN: BWH/TxDOT	CK: GAF
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REVISIONS				
10-12: Added WWR	COUNTY	CONTROL	SECT	JOB
				HIGHWAY



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**TXDOT SINGLE BOX
CULVERTS (SCC-7)**



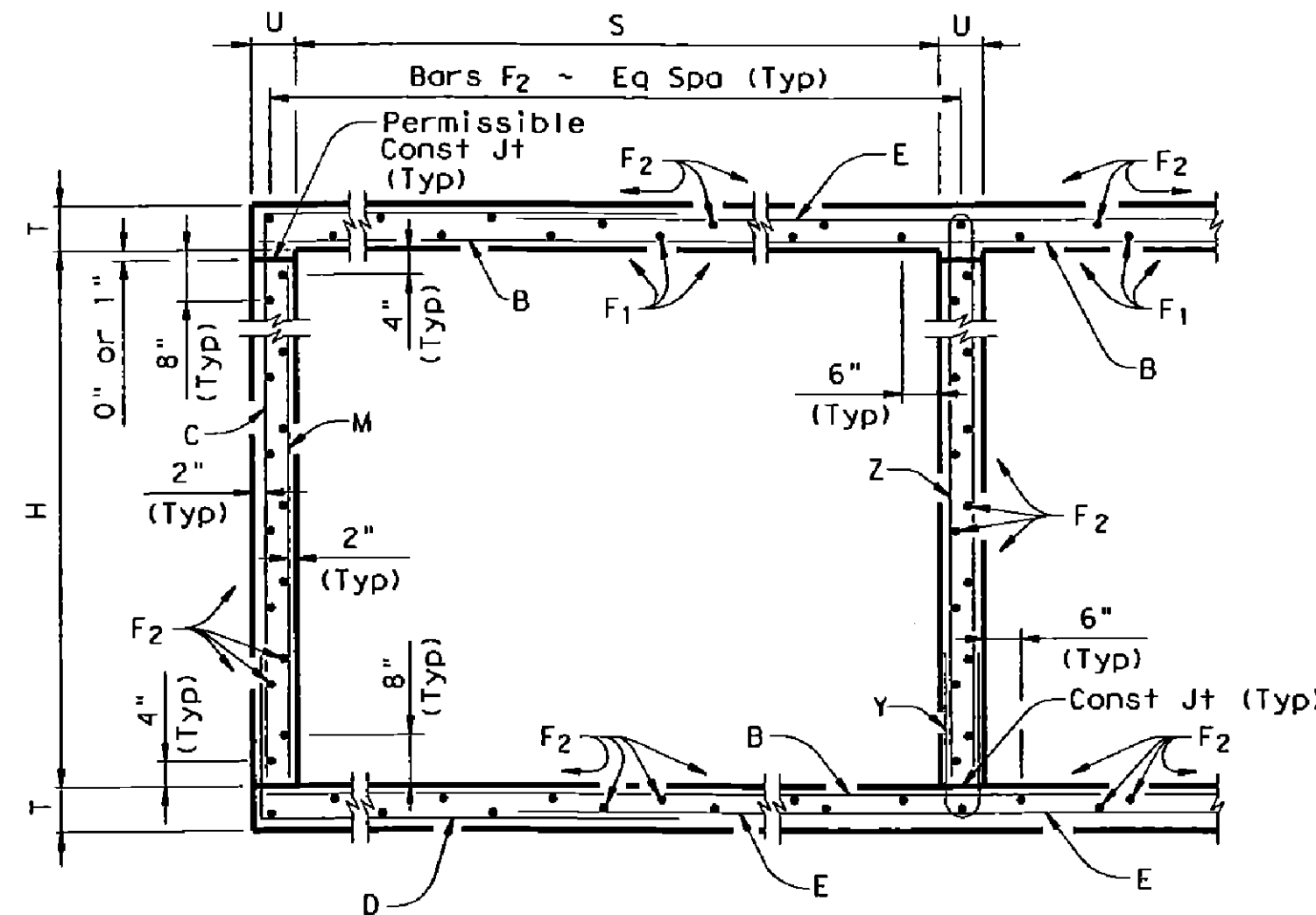
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LAST SHEET EDIT
DATE: 09-26-2013
WA# 12209

**SHEET NO.
D504**

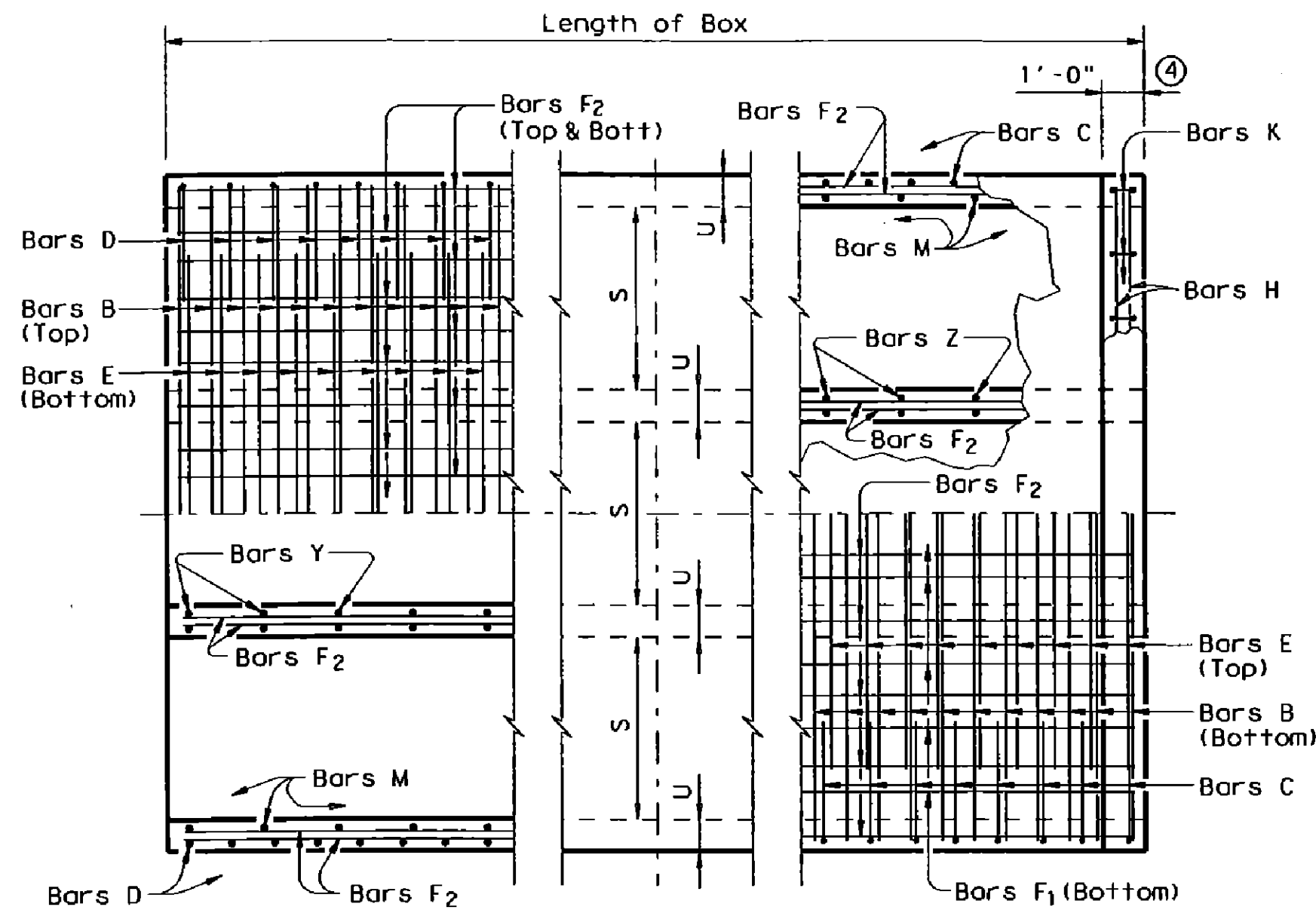
PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817) 467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

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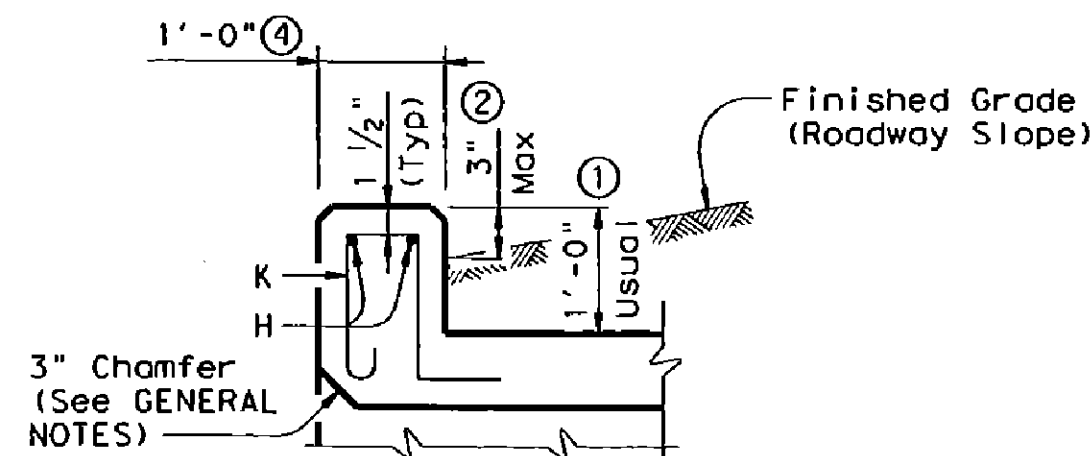
TYPICAL SECTION



BOTTOM SLAB

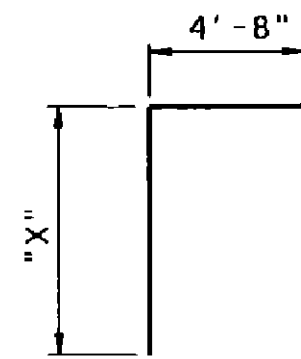
PART PLANS

TOP SLAB

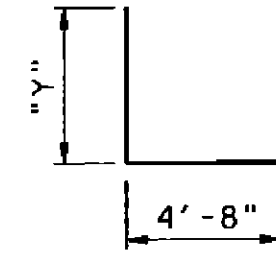


SECTION THRU CURB

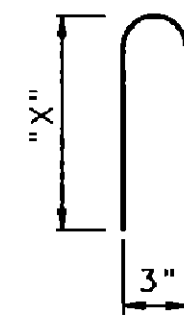
H	Bar Dimensions	
	"X"	"Y"
2'-0"	2'-5"	2'-2"
3'-0"	3'-5"	2'-2"
4'-0"	4'-5"	2'-2"
5'-0"	5'-5"	2'-2"



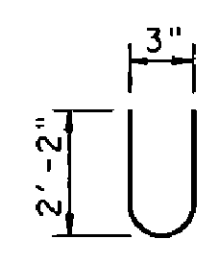
BAR C



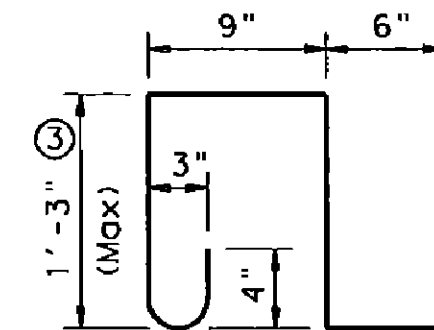
BAR D



BAR Z



BAR Y



BAR K ~ #4
(Spa = 1'-0" Max)
(Length = 4'-3")

RECORD
DRAWING
02/02/2015

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Reinforcing bars shall be adjusted to provide a minimum of 1 1/4" clear cover.
Construction joints shown at the flow line may be raised a maximum of 6" at the Contractor's option. If this option is used, Bars M may be cut off or raised, Bars C and D may be reversed, and Bars Y and Z may be reversed.
See standard MC-MD for skewed ends, angle sections and lengthening details.

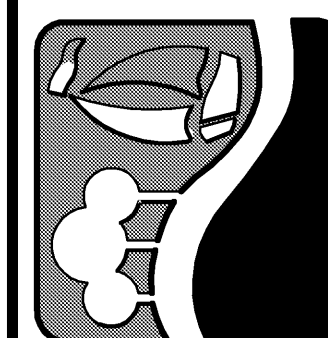
HL93 LOADING SHEET 1 OF 2

Texas Department of Transportation
Bridge Division

MULTIPLE BOX CULVERTS
CAST-IN-PLACE
5'-0" SPAN
0' TO 20' FILL

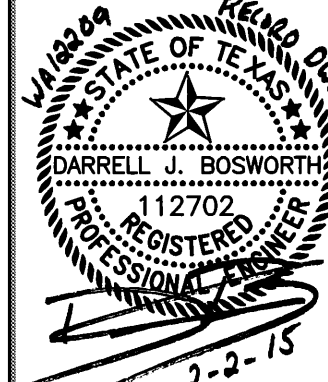
MC-5-20

FILE: mc520ste.dgn	DWG: GAF	CHK: LMM	DWG: BWH/TxDOT	CHK: GAF
© TXDOT February 2010	DISTRICT	FEDERAL AID PROJECT	SHEET	
REVISIONS	COUNTY	CONTROL SECT	JOB	HIGHWAY
10-121 Added WWR				



ROCKWALL
TECHNOLOGY
PARK
PHASE IV

TXDOT MULTIPLE
BOX CULVERTS
(MC-5-20)



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LAST SHEET EDIT
DATE: 09-26-2013
WA# 12209

SHEET NO.
D506

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701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

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NUMBER OF SPANS	SECTION DIMENSIONS				BILLS OF REINFORCING STEEL (For Box Length = 40 feet)																																QUANTITIES									
					Bars B				Bars C & D								Bars E				Bars F ₁ ~#4				Bars F ₂ ~#4 at 1'-6" Max		Bars M~#4 at 1'-6" Max		Bars Y & Z~#4 at 1'-0" Max				Bars H 4~#4		Bars K		Per foot of Barrel		Curb		Total					
	S	H	T	U	No.	Size	Spa	Length	Wt	No.	Size	Spa	Bar C		No.	Size	Spa	Length	Wt	No.	Spa	Length	Wt	No.	Length	Wt	No.	Length	Wt	No.	Bar Y Length	Bar Y Wt	Bar Z Length	Bar Z Wt	Length	Weight	No.	Weight	Conc (CY)	Reinf (LB)	Conc (CY)	Reinf (LB)	Conc (CY)	Reinf (LB)		
													Length	Wt																															Length	Wt
2	5'-0"	2'-0"	7"	7"	194	#4	5"	11'- 6"	1,490	162	#4	6"	7'- 1"	767	6'-10"	739	162	#5	6"	5'- 8"	957	14	8"	39'-9"	372	38	39'-9"	1,009	56	2'- 0"	75	41	4'- 6"	123	5'- 0"	137	11'- 6"	31	26	74	0.637	141.7	0.9	105	26.4	5,774
3	5'-0"	2'-0"	7"	7"	194	#4	5"	17'- 1"	2,214	162	#4	5"	7'- 1"	767	6'-10"	739	162	#5	6"	11'- 3"	1,901	21	8"	39'-9"	558	54	39'-9"	1,434	56	2'- 0"	75	82	4'- 6"	246	5'- 0"	274	17'- 1"	46	36	102	0.922	205.2	1.3	148	38.2	8,356
4	5'-0"	2'-0"	7"	7"	194	#4	5"	22'- 8"	2,937	162	#4	6"	7'- 1"	767	6'-10"	739	162	#5	6"	16'-10"	2,844	28	8"	39'-9"	743	70	39'-9"	1,859	56	2'- 0"	75	123	4'- 6"	370	5'- 0"	411	22'- 8"	61	48	136	1.206	268.6	1.7	197	49.9	10,942
5	5'-0"	2'-0"	7"	7"	194	#4	5"	28'- 3"	3,661	162	#4	6"	7'- 1"	767	6'-10"	739	162	#5	6"	22'- 5"	3,788	35	8"	39'-9"	929	86	39'-9"	2,284	56	2'- 0"	75	164	4'- 6"	493	5'- 0"	548	28'- 3"	75	60	170	1,491	332.1	2.1	245	61.7	13,529
6	5'-0"	2'-0"	7"	7"	194	#4	5"	33'-10"	4,385	162	#4	6"	7'- 1"	767	6'-10"	739	162	#5	6"	28'- 0"	4,731	42	8"	39'-9"	1,115	102	39'-9"	2,708	56	2'- 0"	75	205	4'- 6"	616	5'- 0"	685	33'-10"	90	70	199	1,775	395.5	2.5	289	73.5	16,110
2	5'-0"	3'-0"	7"	7"	194	#4	5"	11'- 6"	1,490	138	#4	7"	8'- 1"	745	6'-10"	630	162	#5	6"	5'- 8"	957	14	8"	39'-9"	372	44	39'-9"	1,168	56	3'- 0"	112	41	4'- 6"	123	7'- 0"	192	11'- 6"	31	26	74	0.702	144.7	0.9	105	29.0	5,894
3	5'-0"	3'-0"	7"	7"	194	#4	5"	17'- 1"	2,214	138	#4	7"	8'- 1"	745	6'-10"	630	162	#5	6"	11'- 3"	1,901	21	8"	39'-9"	558	62	39'-9"	1,646	56	3'- 0"	112	82	4'- 6"	246	7'- 0"	383	17'- 1"	46	36	102	1.008	210.9	1.3	148	41.6	8,583
4	5'-0"	3'-0"	7"	7"	194	#4	5"	22'- 8"	2,937	138	#4	7"	8'- 1"	745	6'-10"	630	162	#5	6"	16'-10"	2,844	28	8"	39'-9"	743	80	39'-9"	2,124	56	3'- 0"	112	123	4'- 6"	370	7'- 0"	575	22'- 8"	61	48	136	1.314	277.0	1.7	197	54.3	11,277
5	5'-0"	3'-0"	7"	7"	194	#4	5"	28'- 3"	3,661	138	#4	7"	8'- 1"	745	6'-10"	630	162	#5	6"	22'- 5"	3,788	35	8"	39'-9"	929	98	39'-9"	2,602	56	3'- 0"	112	164	4'- 6"	493	7'- 0"	767	28'- 3"	75	60	170	1,620	343.2	2.1	245	66.9	13,972
6	5'-0"	3'-0"	7"	7"	194	#4	5"	33'-10"	4,385	138	#4	7"	8'- 1"	745	6'-10"	630	162	#5	6"	28'- 0"	4,731	42	8"	39'-9"	1,115	116	39'-9"	3,080	56	3'- 0"	112	205	4'- 6"	616	7'- 0"	959	33'-10"	90	70	199	1,926	409.3	2.5	289	79.5	16,662
2	5'-0"	4'-0"	7"	7"	194	#4	5"	11'- 6"	1,490	138	#4	7"	9'- 1"	837	6'-10"	630	162	#5	6"	5'- 8"	957	14	8"	39'-9"	372	44	39'-9"	1,168	56	4'- 0"	150	41	4'- 6"	123	9'- 0"	246	11'- 6"	31	26	74	0.767	149.3	0.9	105	31.6	6,078
3	5'-0"	4'-0"	7"	7"	194	#4	5"	17'- 1"	2,214	138	#4	7"	9'- 1"	837	6'-10"	630	162	#5	6"	11'- 3"	1,901	21	8"	39'-9"	558	62	39'-9"	1,646	56	4'- 0"	150	82	4'- 6"	246	9'- 0"	493	17'- 1"	46	36	102	1.095	216.9	1.3	148	45.1	8,823
4	5'-0"	4'-0"	7"	7"	194	#4	5"	22'- 8"	2,937	138	#4	7"	9'- 1"	837	6'-10"	630	162	#5	6"	16'-10"	2,844	28	8"	39'-9"	743	80	39'-9"	2,124	56	4'- 0"	150	123	4'- 6"	370	9'- 0"	739	22'- 8"	61	48	136	1.422	284.4	1.7	197	58.6	11,571
5	5'-0"	4'-0"	7"	7"	194	#4	5"	28'- 3"	3,661	138	#4	7"	9'- 1"	837	6'-10"	630	162	#5	6"	22'- 5"	3,788	35	8"	39'-9"	929	98	39'-9"	2,602	56	4'- 0"	150	164	4'- 6"	493	9'- 0"	986	28'- 3"	75	60	170	1,750	351.9	2.1	245	72.1	14,321
6	5'-0"	4'-0"	7"	7"	194	#4	5"	33'-10"	4,385	138	#4	7"	9'- 1"	837	6'-10"	630	162	#5	6"	28'- 0"	4,731	42	8"	39'-9"	1,115	116	39'-9"	3,080	56	4'- 0"	150	205	4'- 6"	616	9'- 0"	1,232	33'-10"	90	70	199	2,078	419.4	2.5	289	85.6	17,065
2	5'-0"	5'-0"	7"	7"	194	#4	5"	11'- 6"	1,490	138	#4	7"	10'- 1"	930	6'-10"	630	162	#5	6"	5'- 8"	957	14	8"	39'-9"	372	50	39'-9"	1,328	56	5'- 0"	187	41	4'- 6"	123	11'- 0"	301	11'- 6"	31	26	74	0.832	158.0	0.9	105	34.2	6,423
3	5'-0"	5'-0"	7"	7"	194	#4	5"	17'- 1"	2,214	138	#4	7"	10'- 1"	930	6'-10"	630	162	#5	6"	11'- 3"	1,901	21	8"	39'-9"	558	70	39'-9"	1,859	56	5'- 0"	187	82	4'- 6"	246	11'- 0"	603	17'- 1"	46	36	102	1.181	228.2	1.3	148	48.5	9,276
4	5'-0"	5'-0"	7"	7"	194	#4	5"	22'- 8"	2,937	138	#4	7"	10'- 1"	930	6'-10"	630	162	#5	6"	16'-10"	2,844	28	8"	39'-9"	743	90	39'-9"	2,390	56	5'- 0"	187	123	4'- 6"	370	11'- 0"	904	22'- 8"	61	48	136	1,530	298.4	1.7	197	62.9	12,132
5	5'-0"	5'-0"	7"	7"	194	#4	5"	28'- 3"	3,661	138	#4	7"	10'- 1"	930	6'-10"	630	162	#5	6"	22'- 5"	3,788	35	8"	39'-9"	929	110	39'-9"	2,921	56	5'- 0"	187	164	4'- 6"	493	11'- 0"	1,205	28'- 3"	75	60	170	1,880	368.6	2.1	245	77.3	14,989
6	5'-0"	5'-0"	7"	7"	194	#4	5"	33'-10"	4,385	138	#4	7"	10'- 1"	930	6'-10"	630	162	#5	6"	28'- 0"	4,731	42	8"	39'-9"	1,115	130	39'-9"	3,452	56	5'- 0"	187	205	4'- 6"	616	11'- 0"	1,506	33'-10"	90	70	199	2,229	438.8	2.5	289	91.7	17,841

Deformed welded wire reinforcement (WWR) meeting the requirements of ASTM A1064 may be used to replace conventional reinforcement shown at the Contractor's option. The area of required reinforcement may be reduced by the ratio of 60 ksi / 70 ksi. Spacing of WWR is limited to 4' Min and 18" Max. When required, provide lap splices in the WWR of the same length required for the equivalent bar size, rounded up for wire sizes between conventional bar sizes.

Example Conversion: Replacement of No. 6 Gr 60 at 6" Spacing with WWR.
WWR required = (0.44 sq in/ 0.5') x (60 ksi/70 ksi) = 0.754 sq in/ft.
If D30.6 wire is used to meet the 0.754 sq in/ft requirement in this example, the required spacing = (0.306 sq in/ 0.754 sq in/ft) x 12 in/ft = 4.87" Max spacing.
Required lap length for the provided D30.6 wire is 2'-2" (Lap required for uncoated No. 5 bars, as shown in Item 440).

RECORD
DRAWING
02/02/2015

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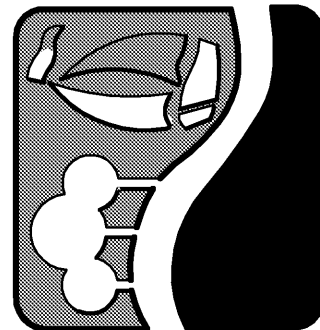
HL93 LOADING SHEET 2 OF 2

Texas Department of Transportation
Bridge Division

MULTIPLE BOX CULVERTS
CAST-IN-PLACE
5'-0" SPAN
0' TO 20' FILL

MC-5-20

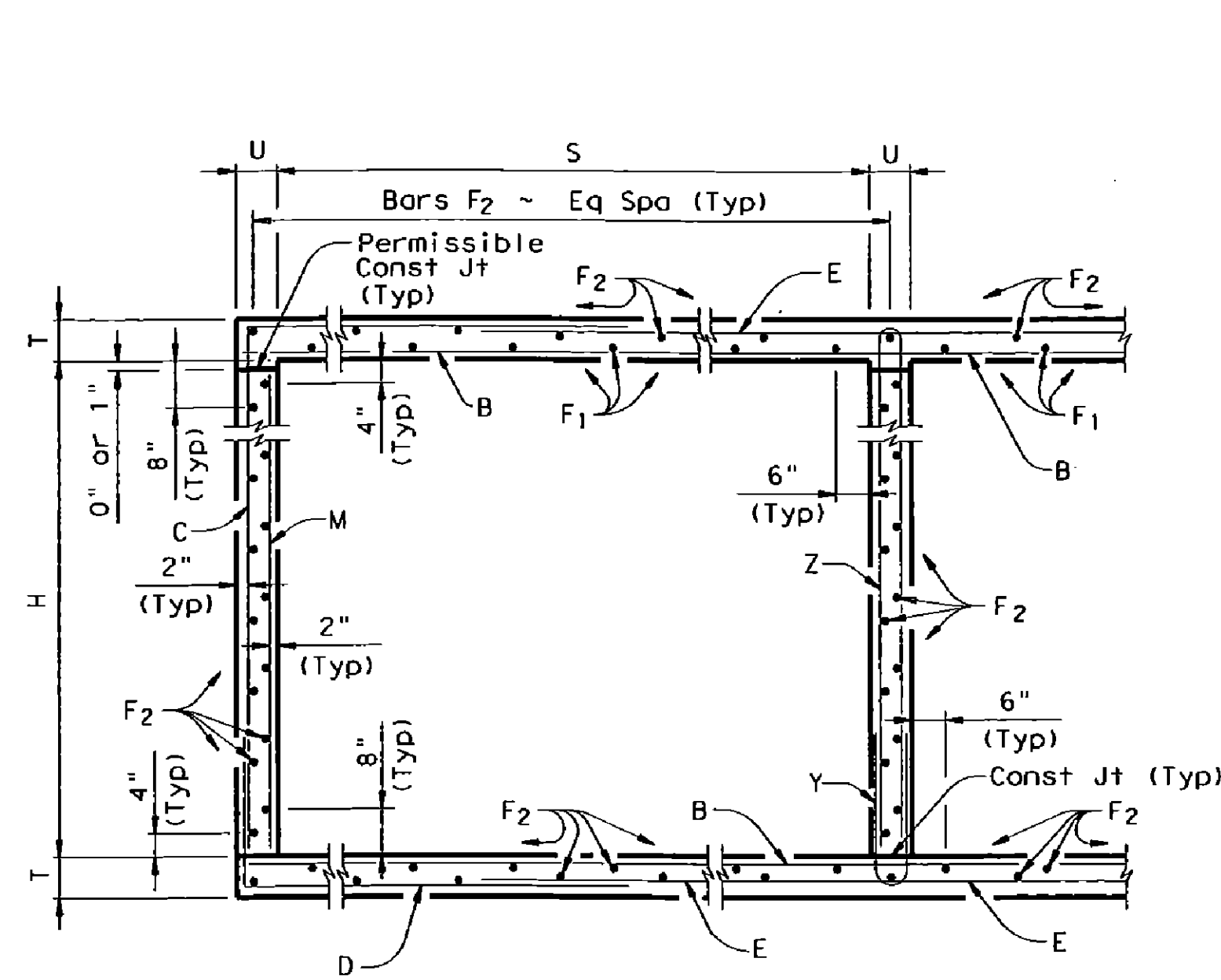
FILE: mc520ste.dgn	DN: GAF	CK: LMN	DN: BWH/TxDOT	CK: GAF	
©TxDOT February 2010	DISTRICT	FEDERAL AID PROJECT			SHEET
REVISIONS					
10-12: Added WWR	COUNTY	CONTROL	SECT	JOB	HIGHWAY



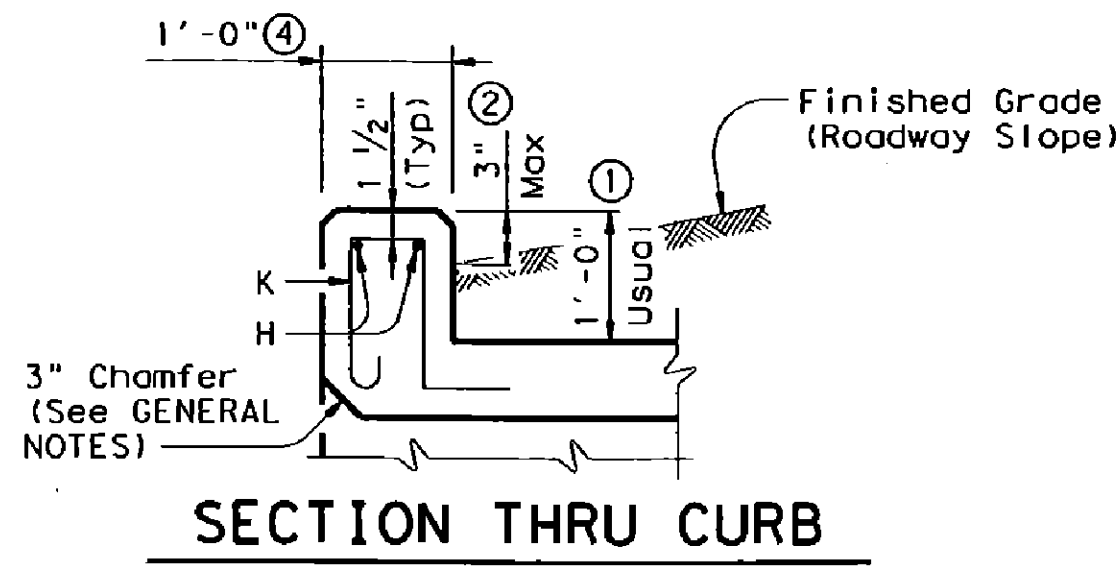
ROCKW

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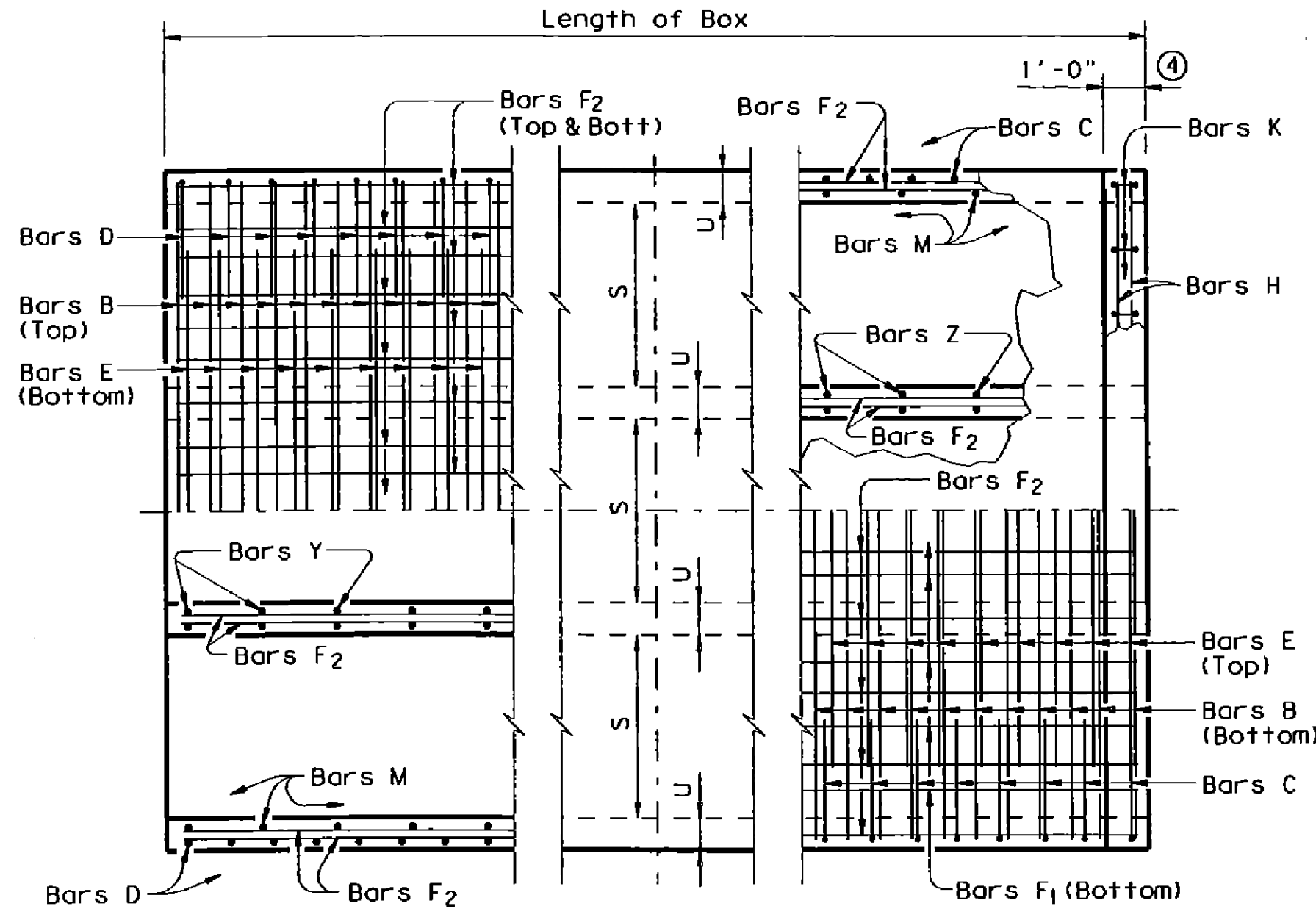
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TYPICAL SECTION



SECTION THRU CURB

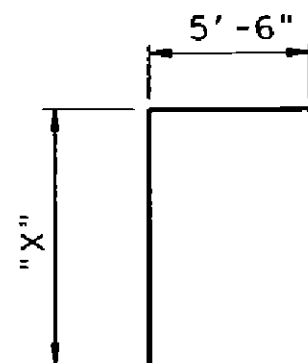


BOTTOM SLAB

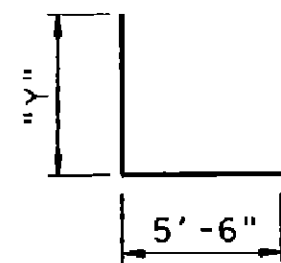
PART PLANS

TOP SLAB

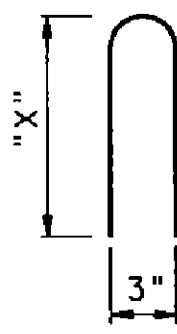
H	Bar Dimensions	
	"X"	"Y"
3'-0"	3'-5"	2'-2"
4'-0"	4'-5"	2'-2"
5'-0"	5'-5"	2'-2"
6'-0"	6'-5"	2'-2"



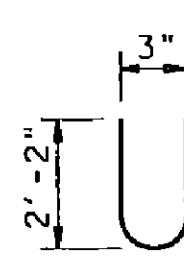
BARS C



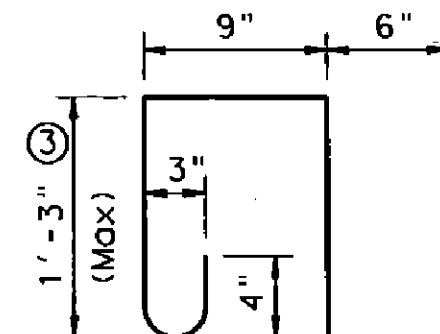
BARS D



BARS Z



BARS Y



BARS K ~ #4
(Spa = 1'-0" Max)
(Length = 4'-3")

- 0" min to 5'-0" max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail, bicycle rail or curbs taller than 1'-0", refer to ECD standard. For structures with T6 bridge rail, refer to T6-CM standard. For structures with traffic rail, other than T6, refer to RAC standard.
- For vehicle safety, the following requirements must be met:
 - For structures without bridge rail, curbs shall project no more than 3" above finished grade.
 - For structures with bridge rail, curbs shall be flush with finished grade.Curb heights shall be reduced, if necessary, to meet the above requirements. No changes will be made in quantities and no additional compensation will be allowed for this work.
- For curbs less than 1'-0" high, tilt bars K or reduce bar height as necessary to maintain cover. For curbs less than 3" high, bars K may be omitted.
- 1'-0" typical. 2'-0" when RAC standard is referred to elsewhere in the plans.

Deformed welded wire reinforcement (WWR) meeting the requirements of ASTM A1064 may be used to replace conventional reinforcement shown at the Contractor's option. The area of required reinforcement may be reduced by the ratio of 60 ksi / 70 ksi. Spacing of WWR is limited to 4" Min and 18" Max. When required, provide lap splices in the WWR of the same length required for the equivalent bar size, rounded up for wire sizes between conventional bar sizes.

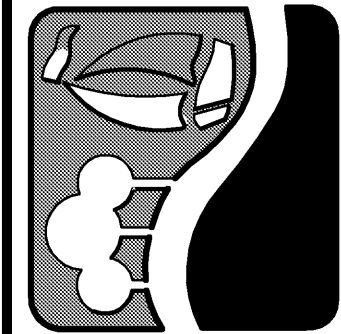
Example Conversion: Replacement of No. 6 Gr 60 at 6" Spacing with WWR.
WWR required = $(0.44 \text{ sq in/ } 0.5') \times (60 \text{ ksi}/70 \text{ ksi}) = 0.754 \text{ sq in/ft}$.
If D30.6 wire is used to meet the 0.754 sq in/ft requirement in this example, the required spacing = $(0.306 \text{ sq in/ } 0.754 \text{ sq in/ft}) \times 12 \text{ in/ft} = 4.87"$ Max spacing.
Required lap length for the provided D30.6 wire is 2'-2" (Lap required for uncoated No. 5 bars, as shown in Item 440).

GENERAL NOTES:
Designed according to AASHTO LRFD Specifications.
Designed to the maximum fill height shown.
All reinforcing steel shall be Grade 60.
All concrete shall be Class "C" with these exceptions: use Class "S" for top slabs of culverts with overlay, with 1-to-2 course surface treatment, or with the top slab as the final riding surface.
Class "C" concrete shall have a minimum compressive strength of 3,600 psi. Class "S" concrete shall have a minimum compressive strength of 4,000 psi.
The use of permanent forms is not allowed.
The bottom edge of the top slab shall be chamfered 3" at the entrance.
Reinforcing bars shall be adjusted to provide a minimum of 1 1/4" clear cover.
Construction joints shown at the flow line may be raised a maximum of 6" at the Contractor's option. If this option is used, Bars M may be cut off or raised, Bars C and D may be reversed, and Bars Y and Z may be reversed.
See standard MC-MD for skewed ends, angle sections and lengthening details.

**RECORD
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02/02/2015**
TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

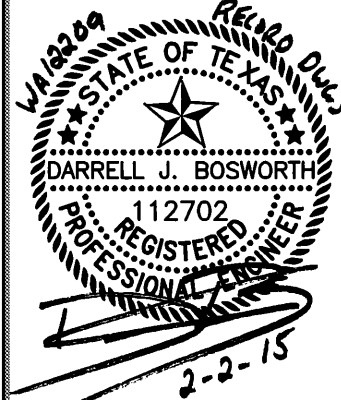
HL93 LOADING SHEET 1 OF 2

Texas Department of Transportation Bridge Division				
MULTIPLE BOX CULVERTS CAST-IN-PLACE 6'-0" SPAN 0' TO 16' FILL				
MC-6-16				
FILE: mc616ste.dgn	DN: GAF	CK: LMW	DN: BWH/TxDOT	CK: GAF
©TxDOT February 2010		DISTRICT FEDERAL A10 PROJECT SHEET		
REVISIONS				
10-12: Added WWR	COUNTY	CONTROL	SECT	JOB HIGHWAY



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**TXDOT MULTIPLE
BOX CULVERTS
(MC-6-16)**



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WIER & ASSOCIATES, INC.
LAST SHEET EDIT
DATE: 09-26-2013
WA# 12209
**SHEET NO.
D508**

PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

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NUMBER OF SPANS	SECTION DIMENSIONS				BILLS OF REINFORCING STEEL (For Box Length = 40 feet)																																								QUANTITIES					
					Bars B				Bars C & D								Bars E				Bars F ₁ ~#4				Bars F ₂ ~#4 at 1'-6" Max				Bars M~#4 at 1'-6" Max				Bars Y & Z~#4 at 1'-0" Max				Bars H 4~#4		Bars K		Per foot of Barrel		Curb		Total					
	Conc (CY)		Reinf (Lb)																																						Conc (CY)		Reinf (Lb)		Conc (CY)		Reinf (Lb)			
	S	H	T	U	No.	Size	Spd	Length	Wt	No.	Size	Spd	Bar C		Bar D		No.	Size	Spd	Length	Wt	No.	Spd	Length	Wt	No.	Length	Wt	No.	Bar Y Length	Bar Y Wt	Bar Z Length	Bar Z Wt	Length	Weight	No.	Weight	Conc (CY)	Reinf (Lb)	Conc (CY)	Reinf (Lb)	Conc (CY)	Reinf (Lb)							
2	6'-0"	3'-0"	7"	7"	162	#5	6"	13'- 6"	2,281	162	#4	6"	8'-11"	965	7'- 8"	830	194	#5	5"	6'- 0"	1,214	16	9"	39'-9"	425	50	39'-9"	1,328	56	3'- 0"	112	41	4'- 6"	123	7'- 0"	192	13'- 6"	36	30	85	0.789	186.8	1.0	121	32.6	7,591				
3	6'-0"	3'-0"	7"	7"	162	#5	6"	20'- 1"	3,393	162	#4	6"	8'-11"	965	7'- 8"	830	194	#5	5"	12'- 7"	2,546	24	9"	39'-9"	637	71	39'-9"	1,885	56	3'- 0"	112	82	4'- 6"	246	7'- 0"	383	20'- 1"	54	42	119	1.138	274.9	1.5	173	47.0	11,170				
4	6'-0"	3'-0"	7"	7"	162	#5	6"	26'- 8"	4,506	162	#4	6"	8'-11"	965	7'- 8"	830	194	#5	5"	19'- 2"	3,878	32	9"	39'-9"	850	92	39'-9"	2,443	56	3'- 0"	112	123	4'- 6"	370	7'- 0"	575	26'- 8"	71	56	159	1.487	363.2	2.0	230	61.5	14,759				
5	6'-0"	3'-0"	7"	7"	162	#5	6"	33'- 3"	5,618	162	#4	6"	8'-11"	965	7'- 8"	830	194	#5	5"	25'- 9"	5,210	40	9"	39'-9"	1,062	113	39'-9"	3,000	56	3'- 0"	112	164	4'- 6"	493	7'- 0"	767	33'- 3"	89	70	199	1.836	451.4	2.5	288	75.9	18,345				
6	6'-0"	3'-0"	7"	7"	162	#5	6"	39'-10"	6,730	162	#4	6"	8'-11"	965	7'- 8"	830	194	#5	5"	32'- 4"	6,542	48	9"	39'-9"	1,275	134	39'-9"	3,558	56	3'- 0"	112	205	4'- 6"	616	7'- 0"	959	39'-10"	106	82	233	2.186	539.7	3.0	339	90.4	21,926				
2	6'-0"	4'-0"	7"	7"	162	#5	6"	13'- 6"	2,281	162	#4	6"	9'-11"	1,073	7'- 8"	830	194	#5	5"	6'- 0"	1,214	16	9"	39'-9"	425	50	39'-9"	1,328	56	4'- 0"	150	41	4'- 6"	123	9'- 0"	246	13'- 6"	36	30	85	0.853	191.8	1.0	121	35.1	7,791				
3	6'-0"	4'-0"	7"	7"	162	#5	6"	20'- 1"	3,393	162	#4	6"	9'-11"	1,073	7'- 8"	830	194	#5	5"	12'- 7"	2,546	24	9"	39'-9"	637	71	39'-9"	1,885	56	4'- 0"	150	82	4'- 6"	246	9'- 0"	493	20'- 1"	54	42	119	1.224	281.3	1.5	173	50.5	11,426				
4	6'-0"	4'-0"	7"	7"	162	#5	6"	26'- 8"	4,506	162	#4	6"	9'-11"	1,073	7'- 8"	830	194	#5	5"	19'- 2"	3,878	32	9"	39'-9"	850	92	39'-9"	2,443	56	4'- 0"	150	123	4'- 6"	370	9'- 0"	739	26'- 8"	71	56	159	1.595	371.0	2.0	230	65.8	15,069				
5	6'-0"	4'-0"	7"	7"	162	#5	6"	33'- 3"	5,618	162	#4	6"	9'-11"	1,073	7'- 8"	830	194	#5	5"	25'- 9"	5,210	40	9"	39'-9"	1,062	113	39'-9"	3,000	56	4'- 0"	150	164	4'- 6"	493	9'- 0"	986	33'- 3"	89	70	199	1.966	460.6	2.5	288	81.1	18,710				
6	6'-0"	4'-0"	7"	7"	162	#5	6"	39'-10"	6,730	162	#4	6"	9'-11"	1,073	7'- 8"	830	194	#5	5"	32'- 4"	6,542	48	9"	39'-9"	1,275	134	39'-9"	3,558	56	4'- 0"	150	205	4'- 6"	616	9'- 0"	1,232	39'-10"	106	82	233	2.337	550.2	3.0	339	96.5	22,345				
2	6'-0"	5'-0"	7"	7"	162	#5	6"	13'- 6"	2,281	162	#4	6"	10'-11"	1,181	7'- 8"	830	194	#5	5"	6'- 0"	1,214	16	9"	39'-9"	425	56	39'-9"	1,487	56	5'- 0"	187	41	4'- 6"	123	11'- 0"	301	13'- 6"	36	30	85	0.918	200.7	1.0	121	37.7	8,150				
3	6'-0"	5'-0"	7"	7"	162	#5	6"	20'- 1"	3,393	162	#4	6"	10'-11"	1,181	7'- 8"	830	194	#5	5"	12'- 7"	2,546	24	9"	39'-9"	637	79	39'-9"	2,098	56	5'- 0"	187	82	4'- 6"	246	11'- 0"	603	20'- 1"	54	42	119	1.311	293.0	1.5	173	53.9	11,894				
4	6'-0"	5'-0"	7"	7"	162	#5	6"	26'- 8"	4,506	162	#4	6"	10'-11"	1,181	7'- 8"	830	194	#5	5"	19'- 2"	3,878	32	9"	39'-9"	850	102	39'-9"	2,708	56	5'- 0"	187	123	4'- 6"	370	11'- 0"	904	26'- 8"	71	56	159	1.703	385.4	2.0	230	70.1	15,644				
5	6'-0"	5'-0"	7"	7"	162	#5	6"	33'- 3"	5,618	162	#4	6"	10'-11"	1,181	7'- 8"	830	194	#5	5"	25'- 9"	5,210	40	9"	39'-9"	1,062	125	39'-9"	3,319	56	5'- 0"	187	164	4'- 6"	493	11'- 0"	1,205	33'- 3"	89	70	199	2.096	477.6	2.5	288	86.3	19,393				
6	6'-0"	5'-0"	7"	7"	162	#5	6"	39'-10"	6,730	162	#4	6"	10'-11"	1,181	7'- 8"	830	194	#5	5"	32'- 4"	6,542	48	9"	39'-9"	1,275	148	39'-9"	3,930	56	5'- 0"	187	205	4'- 6"	616	11'- 0"	1,506	39'-10"	106	82	233	2.488	569.9	3.0	339	102.5	23,136				
2	6'-0"	6'-0"	7"	7"	162	#5	6"	13'- 6"	2,281	194	#4	5"	11'-11"	1,544	7'- 8"	994	194	#5	5"	6'- 0"	1,214	16	9"	39'-9"	425	62	39'-9"	1,646	56	6'- 0"	224	41	4'- 6"	123	13'- 0"	356	13'- 6"	36	30	85	0.983	220.2	1.0	121	40.3	8,928				
3	6'-0"	6'-0"	7"	7"	162	#5	6"	20'- 1"	3,393	194	#4	5"	11'-11"	1,544	7'- 8"	994	194	#5	5"	12'- 7"	2,546	24	9"	39'-9"	637	87	39'-9"	2,310	56	6'- 0"	224	82	4'- 6"	246	13'- 0"	712	20'- 1"	54	42	119	1.397	315.2	1.5	173	57.4	12,779				
4	6'-0"	6'-0"	7"	7"	162	#5	6"	26'- 8"	4,506	194	#4	5"	11'-11"	1,544	7'- 8"	994	194	#5	5"	19'- 2"	3,878	32	9"	39'-9"	850	112	39'-9"	2,974	56	6'- 0"	224	123	4'- 6"	370	13'- 0"	1,068	26'- 8"	71	56	159	1.811	410.2	2.0	230	74.4	16,638				
5	6'-0"	6'-0"	7"	7"	162	#5	6"	33'- 3"	5,618	194	#4	5"	11'-11"	1,544	7'- 8"	994	194	#5	5"	25'- 9"	5,210	40	9"	39'-9"	1,062	137	39'-9"	3,638	56	6'- 0"	224	164	4'- 6"	493	13'- 0"	1,424	33'- 3"	89	70	199	2.225	505.2	2.5	288	91.5	20,495				
6	6'-0"	6'-0"	7"	7"	162	#5	6"	39'-10"	6,730	194	#4	5"	11'-11"	1,544	7'- 8"	994	194	#5	5"	32'- 4"	6,542	48	9"	39'-9"	1,275	162	39'-9"	4,302	56	6'- 0"	224	205	4'- 6"	616	13'- 0"	1,780	39'-10"	106	82	233	2.639	600.2	3.0	339	108.6	24,346				

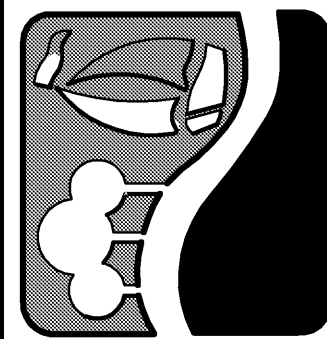
Deformed welded wire reinforcement (WWR) meeting the requirements of ASTM A1064 may be used to replace conventional reinforcement shown at the Contractor's option. The area of required reinforcement may be reduced by the ratio of 60 ksi / 70 ksi. Spacing of WWR is limited to 4" Min and 18" Max. When required, provide lap splices in the WWR of the same length required for the equivalent bar size, rounded up for wire sizes between conventional bar sizes.

Example Conversion: Replacement of No. 6 Gr 60 at 6" Spacing with WWR.
WWR required = (0.44 sq in/ 0.5') x (60 ksi/70 ksi) = 0.754 sq in/ft.
If D30.6 wire is used to meet the 0.754 sq in/ft requirement in this example, the required spacing = (0.306 sq in/ 0.754 sq in/ft) x 12 in/ft = 4.87" Max spacing.
Required lap length for the provided D30.6 wire is 2'-2" (Lap required for uncoated No. 5 bars, as shown in Item 440).

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02/02/2015**

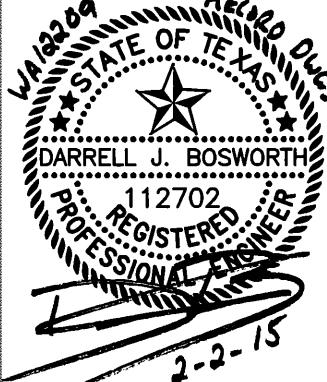
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HL93 LOADING		SHEET 2 OF 2	
MULTIPLE BOX CULVERTS CAST-IN-PLACE 6'-0" SPAN 0' TO 16' FILL			
MC-6-16			
FILE: mc616ste.dgn	DW: GAF	CK: LMW	DW: BWH/TXDOT
©TXDOT February 2010	DISTRICT	FEDERAL AID PROJECT	SHEET
REVISIONS		COUNTY	
10-12: Added WWR		CONTROL	SECT
		JOB	HIGHWAY



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**TXDOT MULTIPLE
BOX CULVERTS
(MC-6-16)**



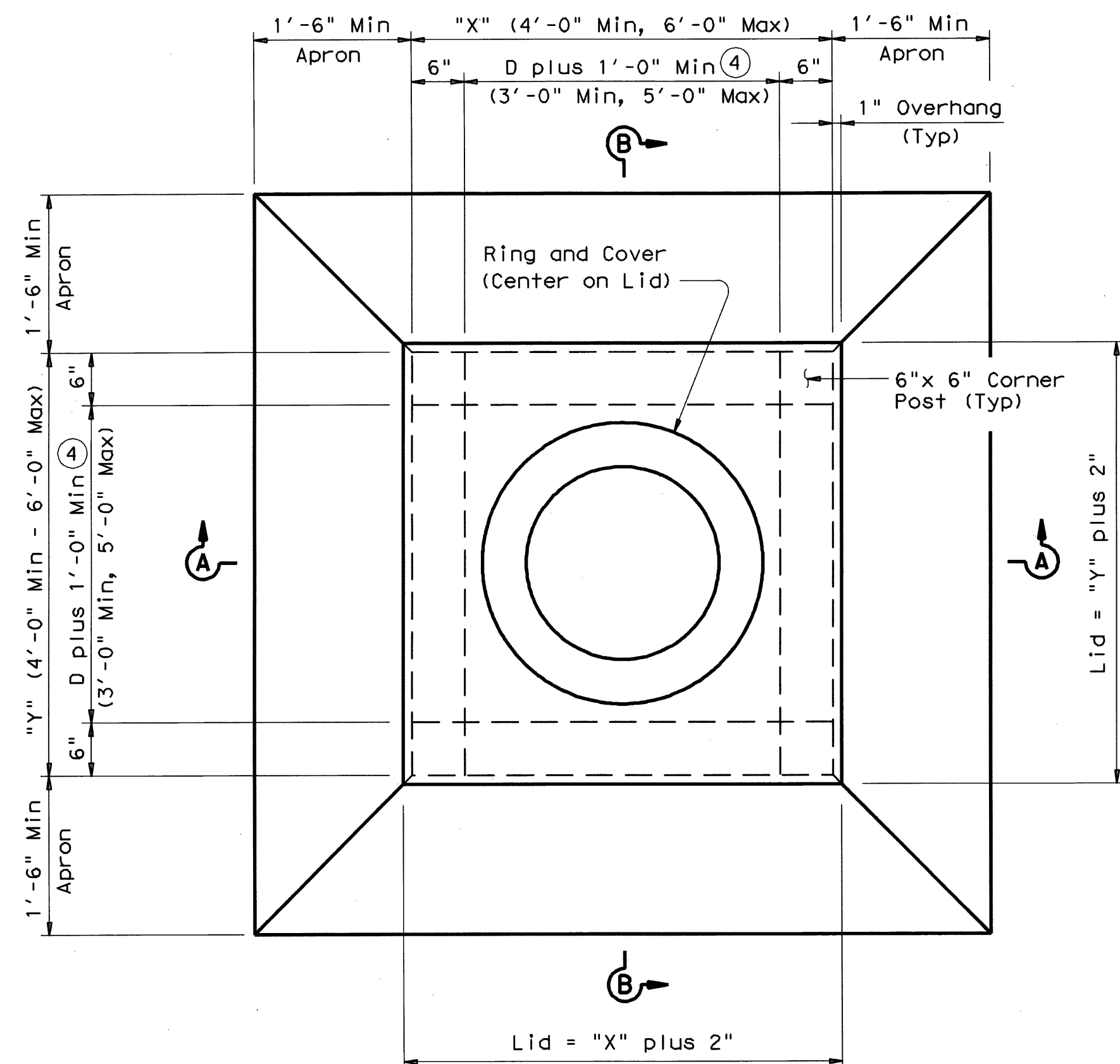
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**SHEET NO.
D509**

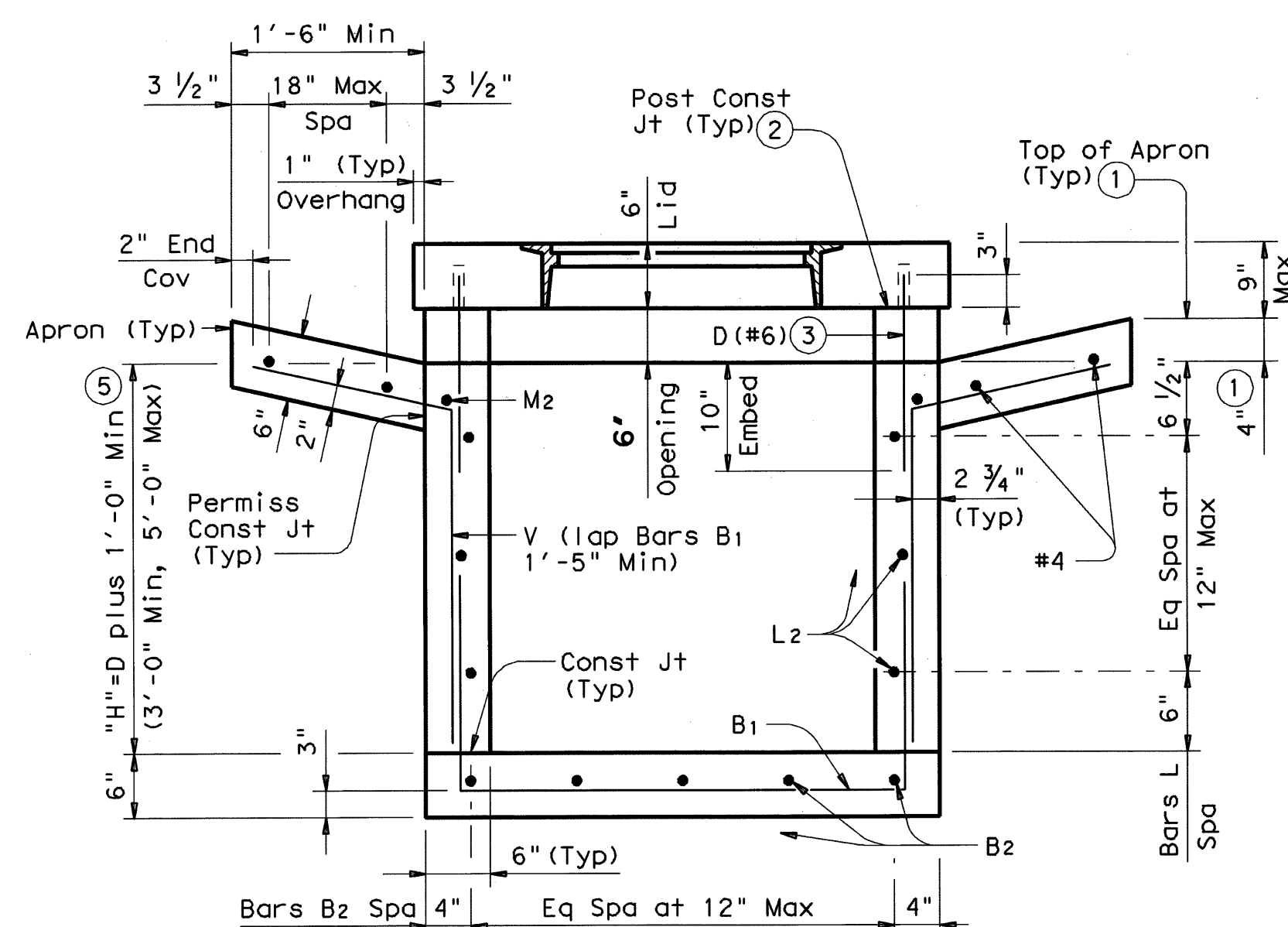
PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

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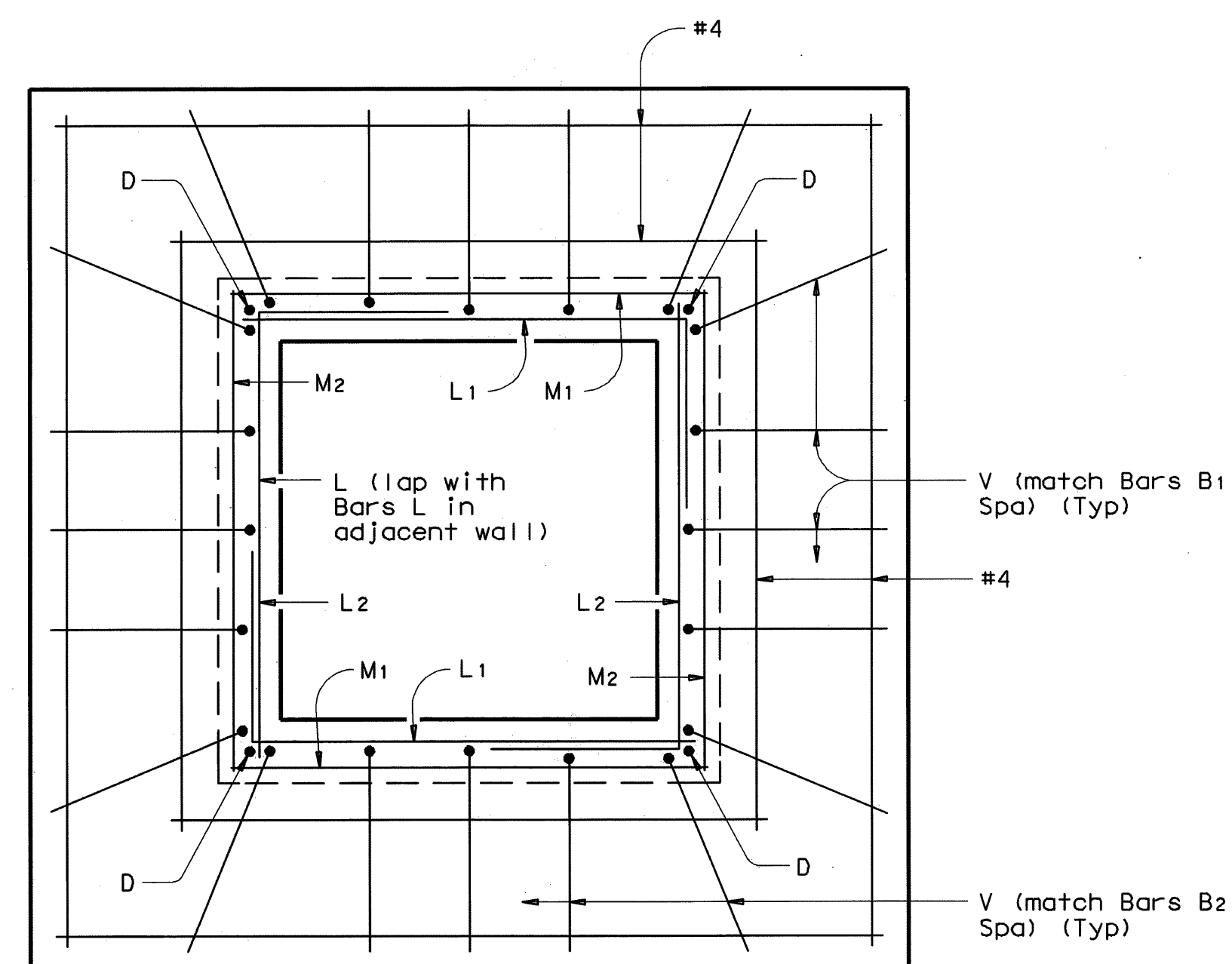
LEVELS DISPLAYED	PATH:
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PLAN

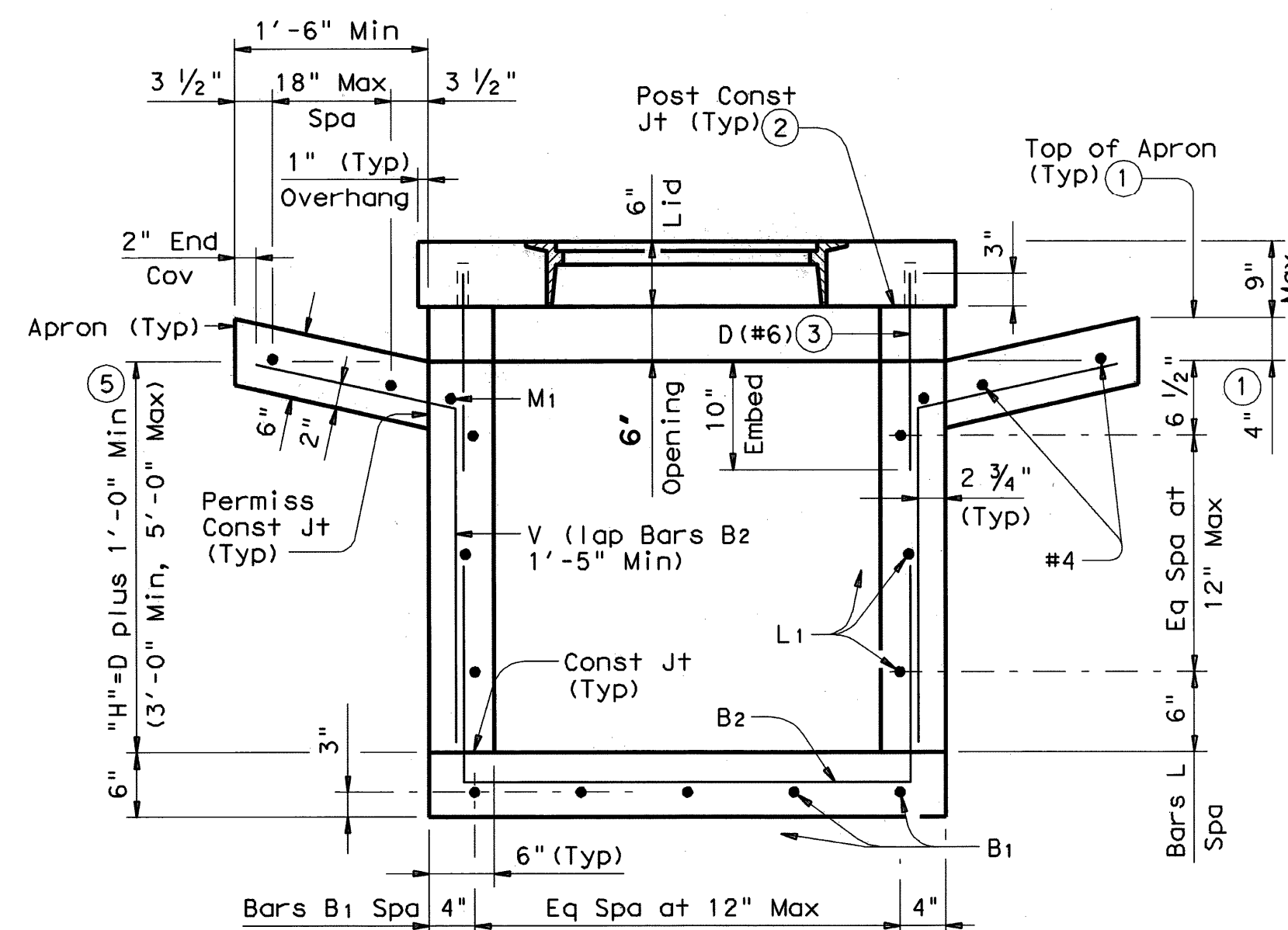


SECTION A-A



TYPICAL APRON PLAN

(Lid not shown for clarity. Showing reinforcing in walls and in apron.)



SECTION B-B

- ① May be changed as directed by the Engineer.
- ② Place layer of grout between lid and corner posts to provide stable seating of lid.
- ③ Center Dowels D in corner posts. (Typ)
- ④ D equals the maximum inside diameter of any pipe entering the wall shown or the opposite wall.
- ⑤ D equals the maximum inside diameter of any pipe entering the inlet.

**RECORD
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02/02/2015**

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SHEET 1 OF 2

 Texas Department of Transportation
Bridge Division

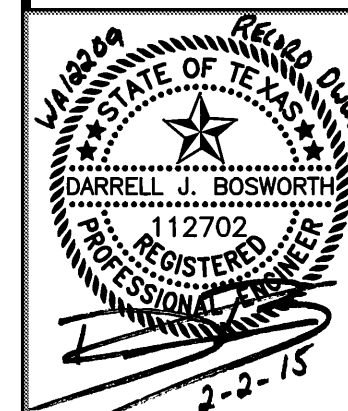
**HORIZONTAL INLET
TYPE H WITH LID
(MAX 48" DIA PIPES)**

IL-H-L

FILE: ihlste02.dgn	DN: TxDOT	CK: TxDOT	DN: TxDOT	CK: TxDOT
© TxDOT February 2010	DISTRICT	FEDERAL AID PROJECT		
REVISIONS				SHEET
	COUNTY	CONTROL	SECT	JOB
				HIGHWAY

ROCKWALL TECHNOLOGY PARK PHASE IV

**TXDOT
HORIZONTAL INLET
(IL-H-L)**



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SHEET NO.
D510

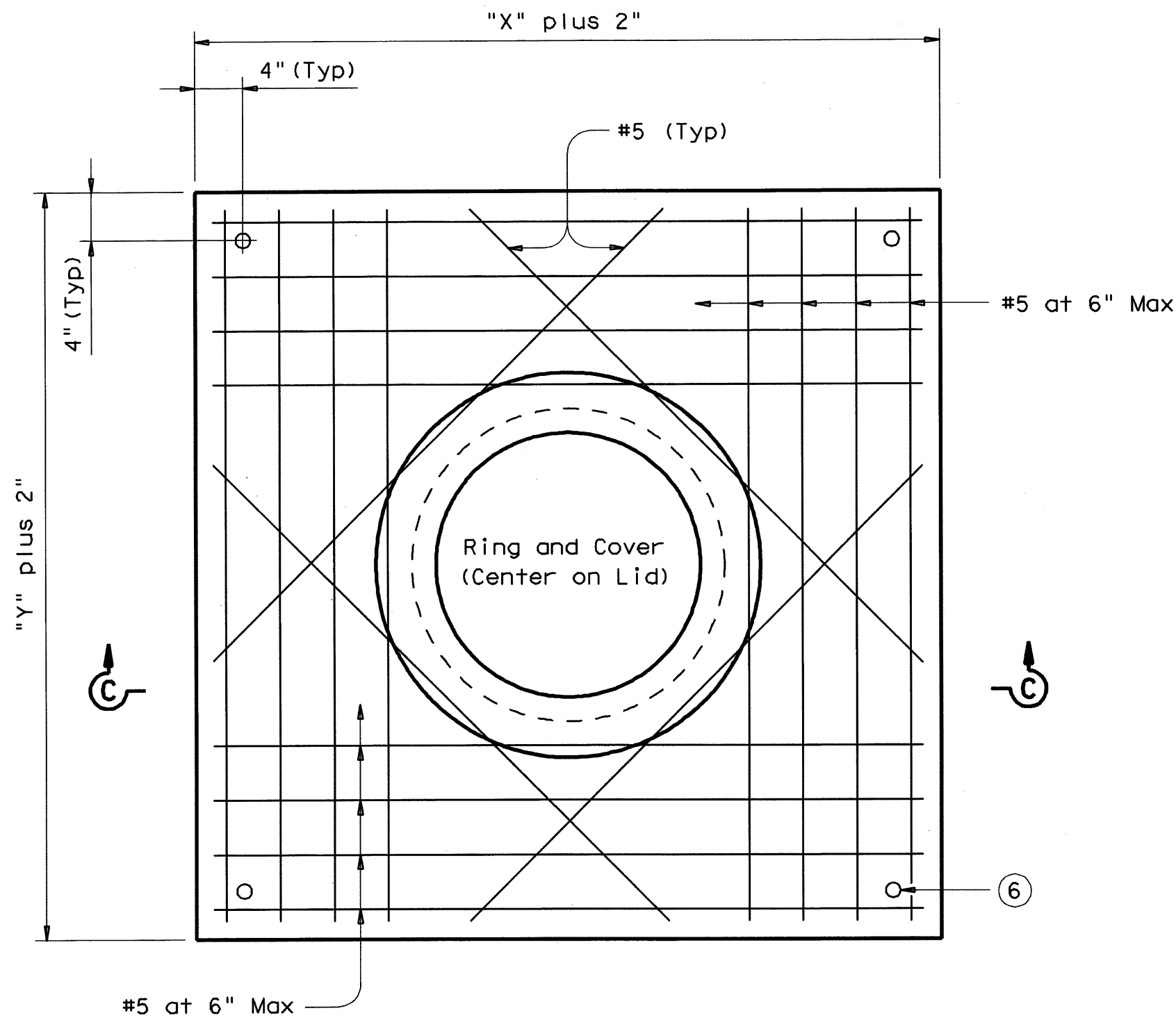
WIA WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS

PREPARED BY:

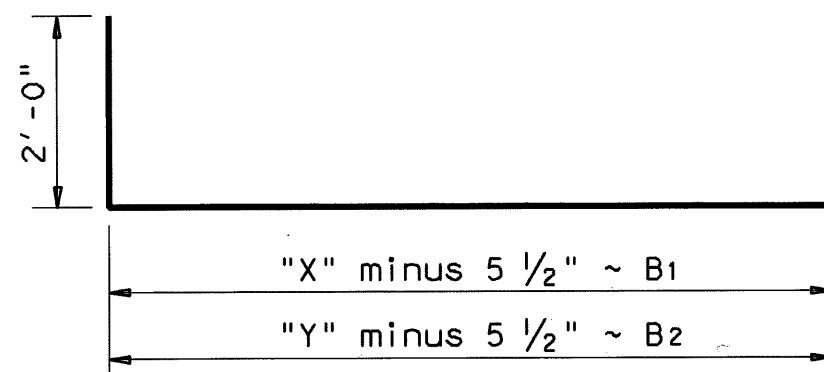
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LEVELS DISPLAYED	PATH:
1	

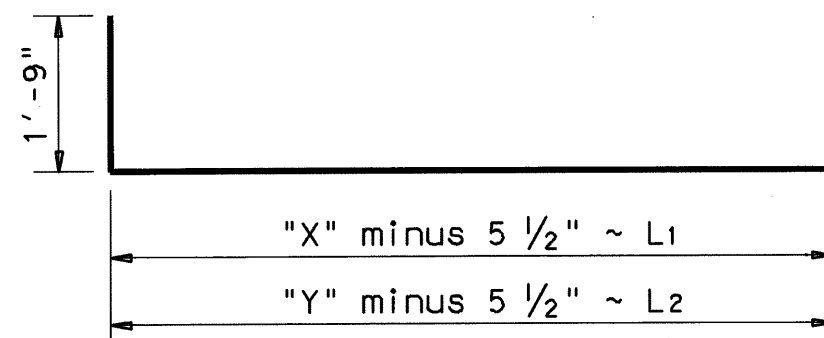
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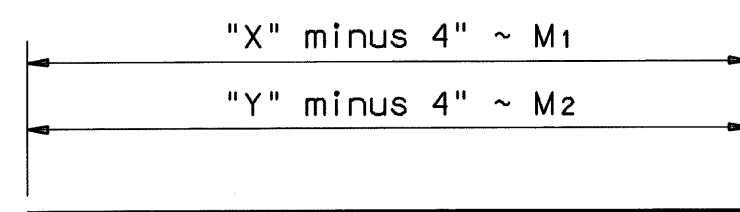
TYPICAL PRECAST LID PLAN



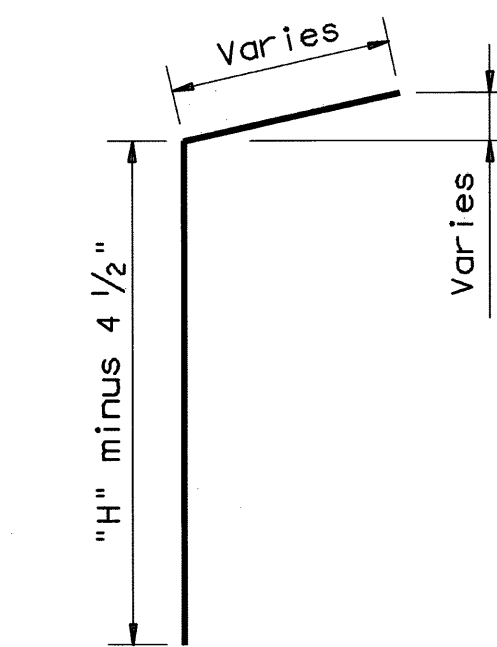
BARS B (#4)



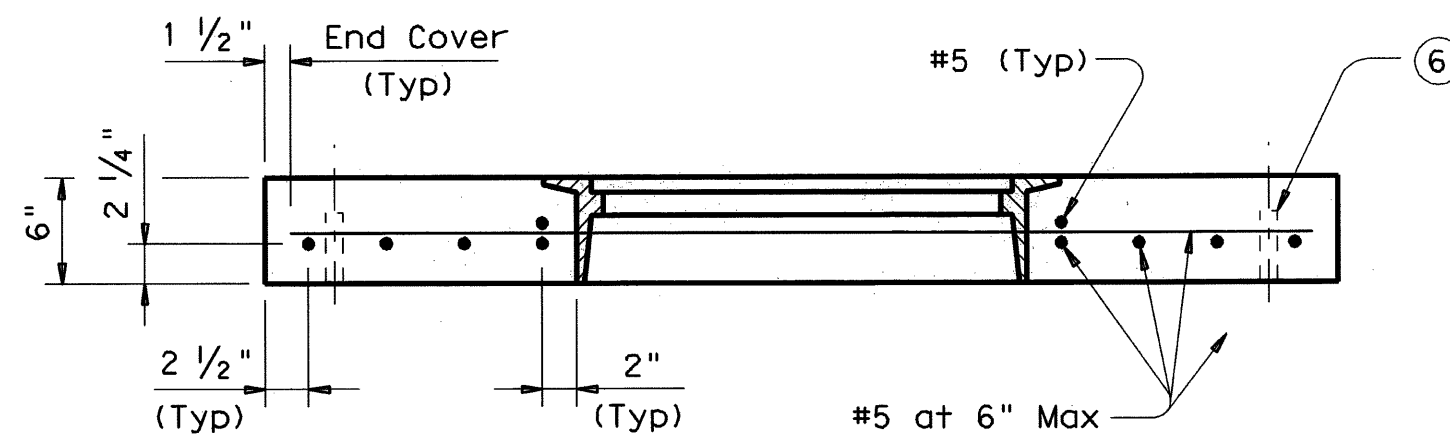
BARS L (#4)



BARS M (#4)

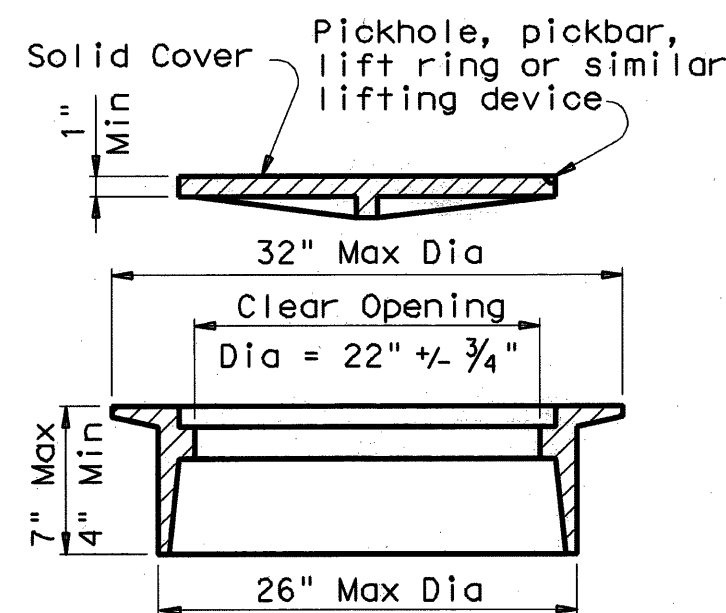


BARS V (#4)



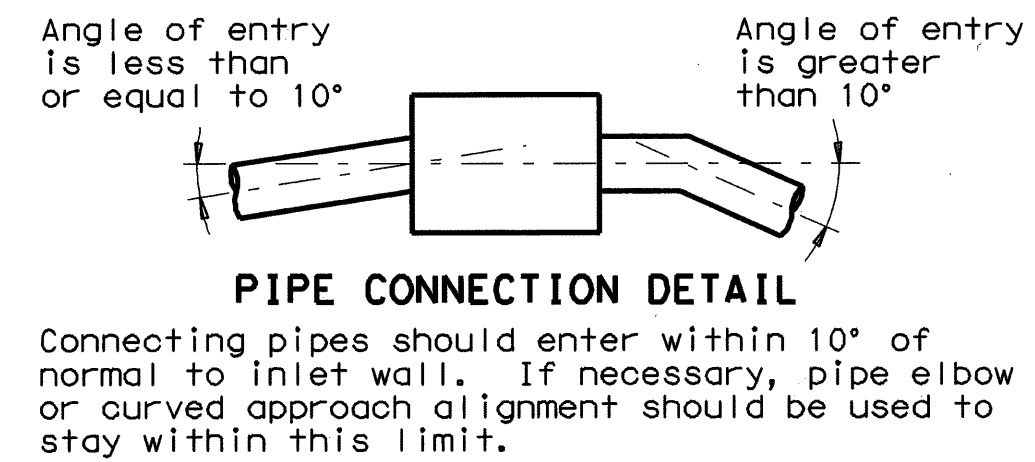
SECTION C-C

⑥ Form holes in lid for Dowels D using 1" Dia x 4" PVC Pipe (SCH 40) (Typ)



RING AND COVER DETAILS

Approximate Weight = 245 lb



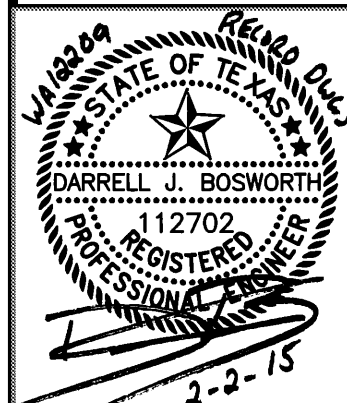
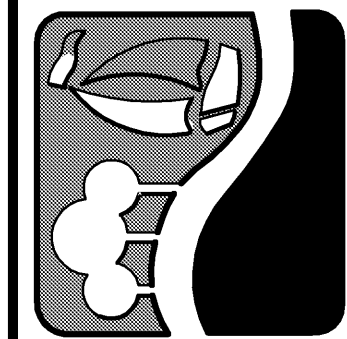
GENERAL NOTES:

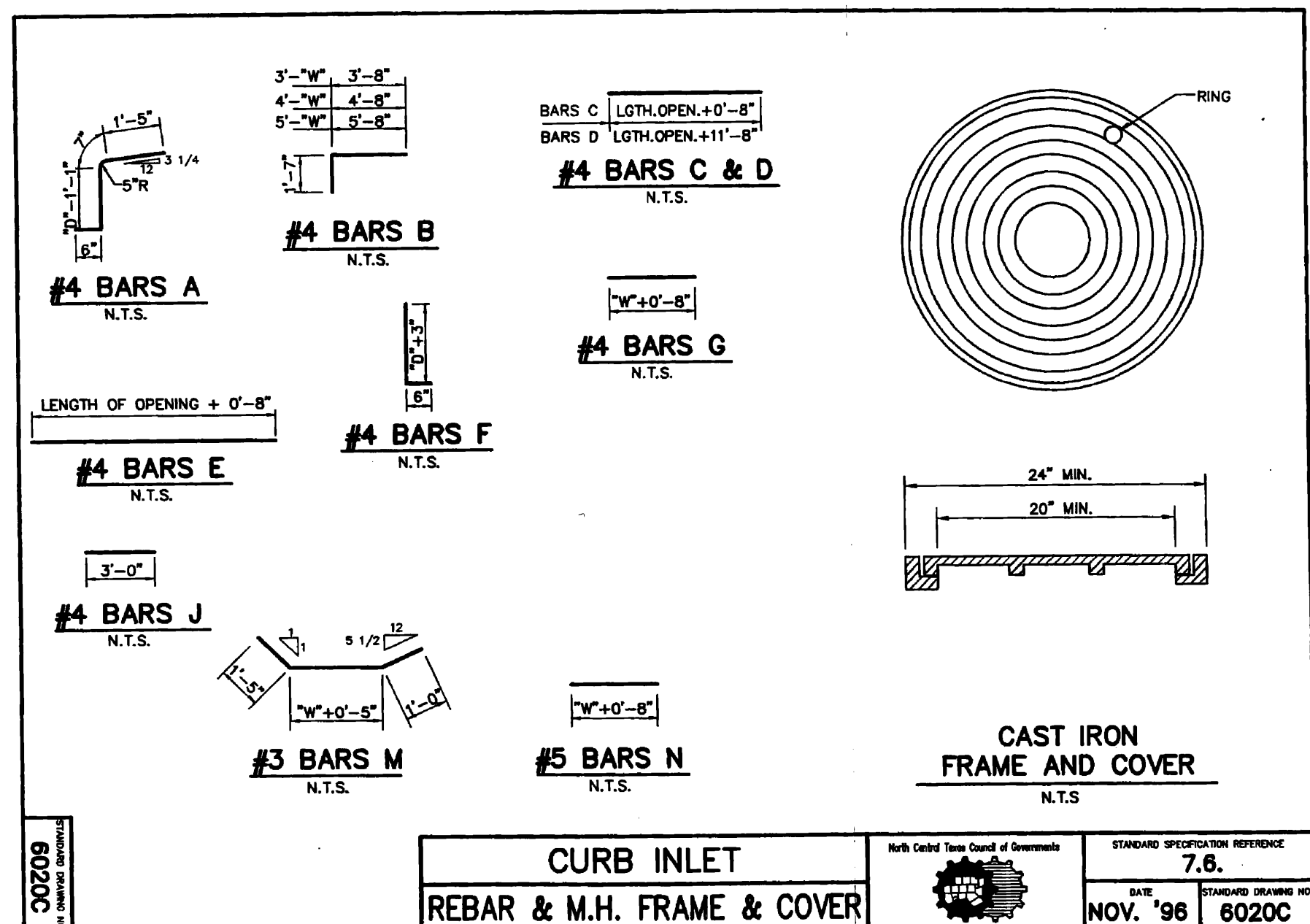
When approved, precast inlets with equivalent structural capacity may be furnished. Sealed engineering calculations and drawings shall be submitted for approval prior to construction. Shop drawings will not be required. Apron will be cast-in-place. Lid will be precast. In areas of conflict between reinforcing steel, blockouts, pipes, anchor bolts or other reinforcing steel, the reinforcement shall be bent or adjusted to clear as directed by the Engineer. Structural Steel for grates shall conform to the requirements of ASTM Designation A-36 or AISI Designation M1010-M1020. All reinforcing steel shall be Grade 60 unless otherwise noted. All concrete shall be Class "A" (f'c = 3,000 psi). All steel components except reinforcing, shall be galvanized after fabrication. Galvanizing damaged during transport or construction shall be repaired in accordance with the specifications. Inlet is to be used in ditches and medians away from the roadway.

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TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

SHEET 2 OF 2				
Texas Department of Transportation Bridge Division				
HORIZONTAL INLET TYPE H WITH LID (MAX 48" DIA PIPES)				
IL-H-L				
FILE: i1hste02.dgn	DN: TxDOT	CK: TxDOT	DN: TxDOT	CK: TxDOT
© TxDOT February 2010	DISTRICT	FEDERAL AID PROJECT	SHEET	
REVISIONS				
COUNTY	CONTROL	SECT	JOB	HIGHWAY

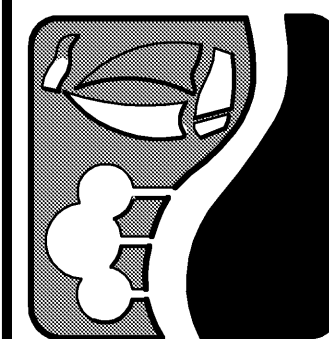


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RECORD DRAWING 02/02/2015

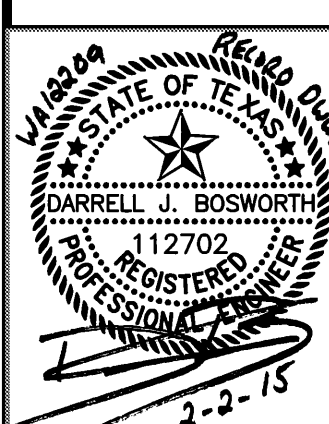
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 PREPARED BY:
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ROCKWALL TECHNOLOGY PARK PHASE IV

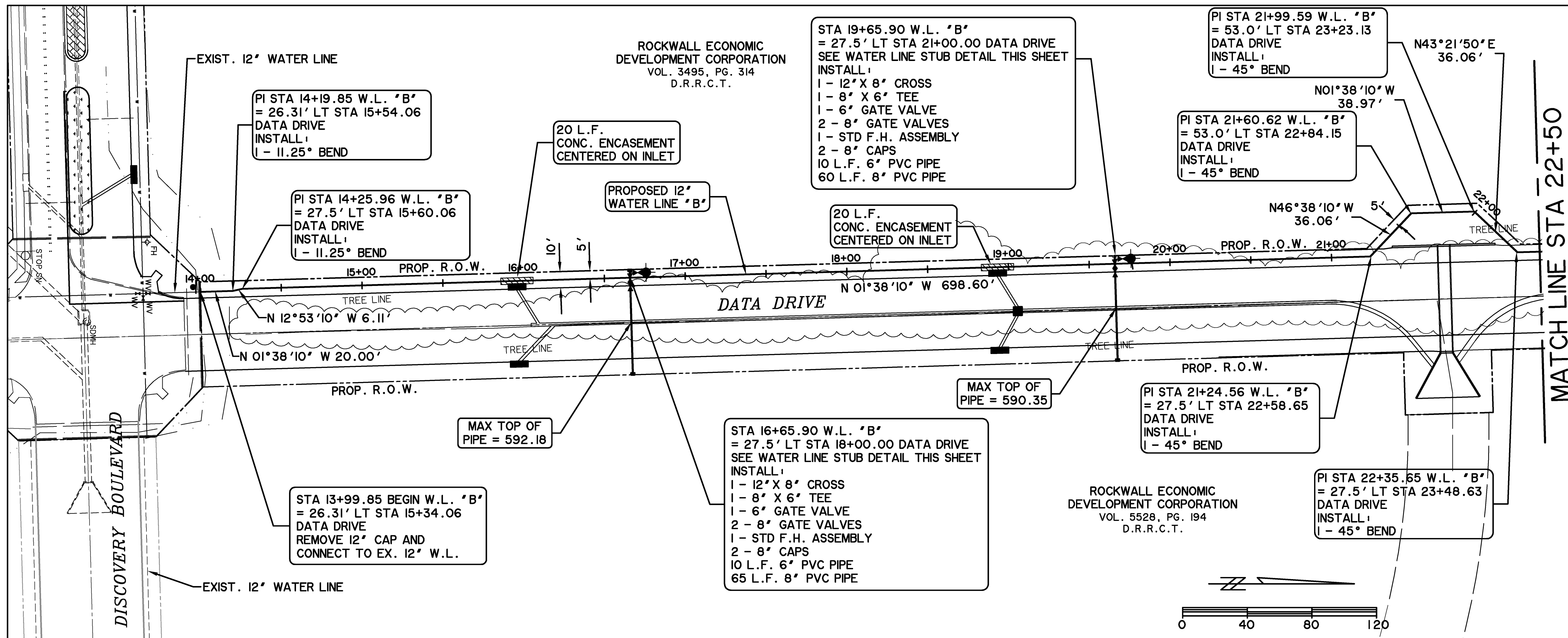
CURB INLET DETAILS



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SHEET NO
D512

TIME 11/15/15
FILE: U101-WATER LINE B-12209.dwg

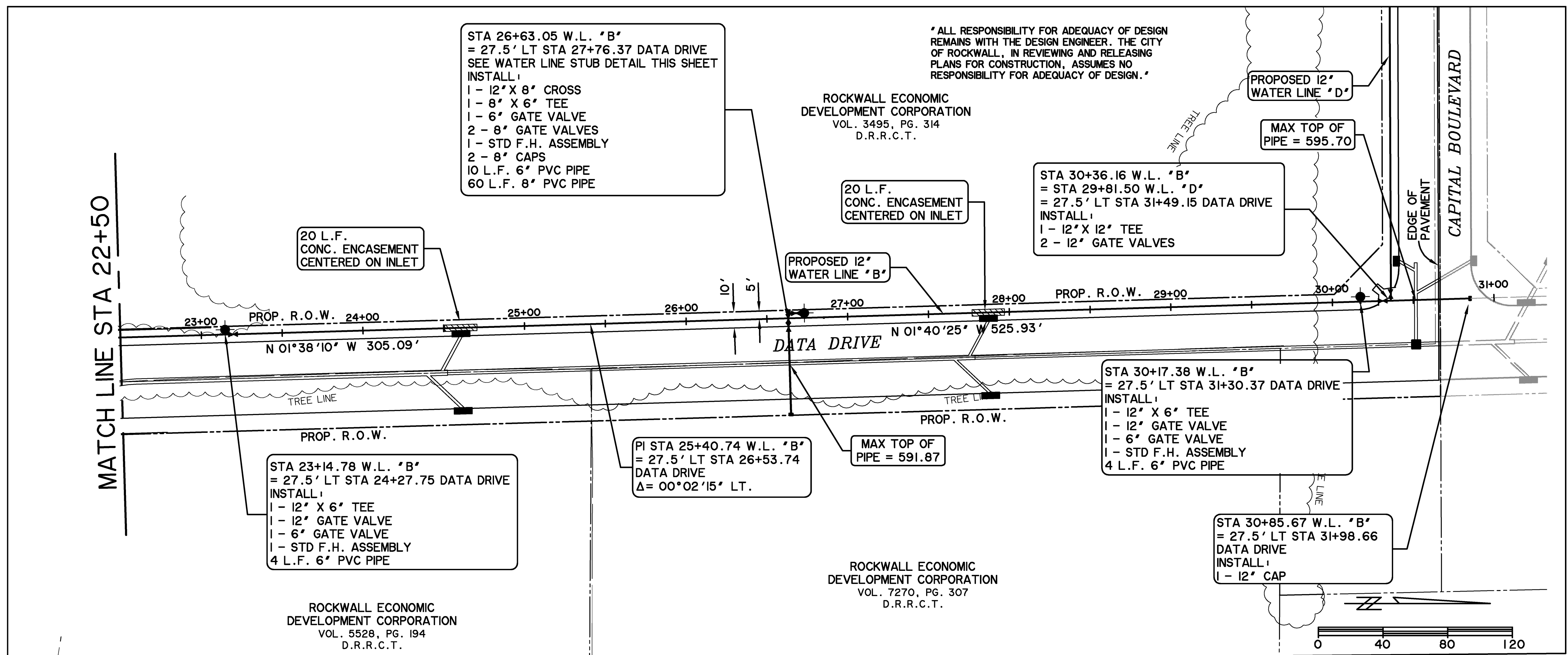
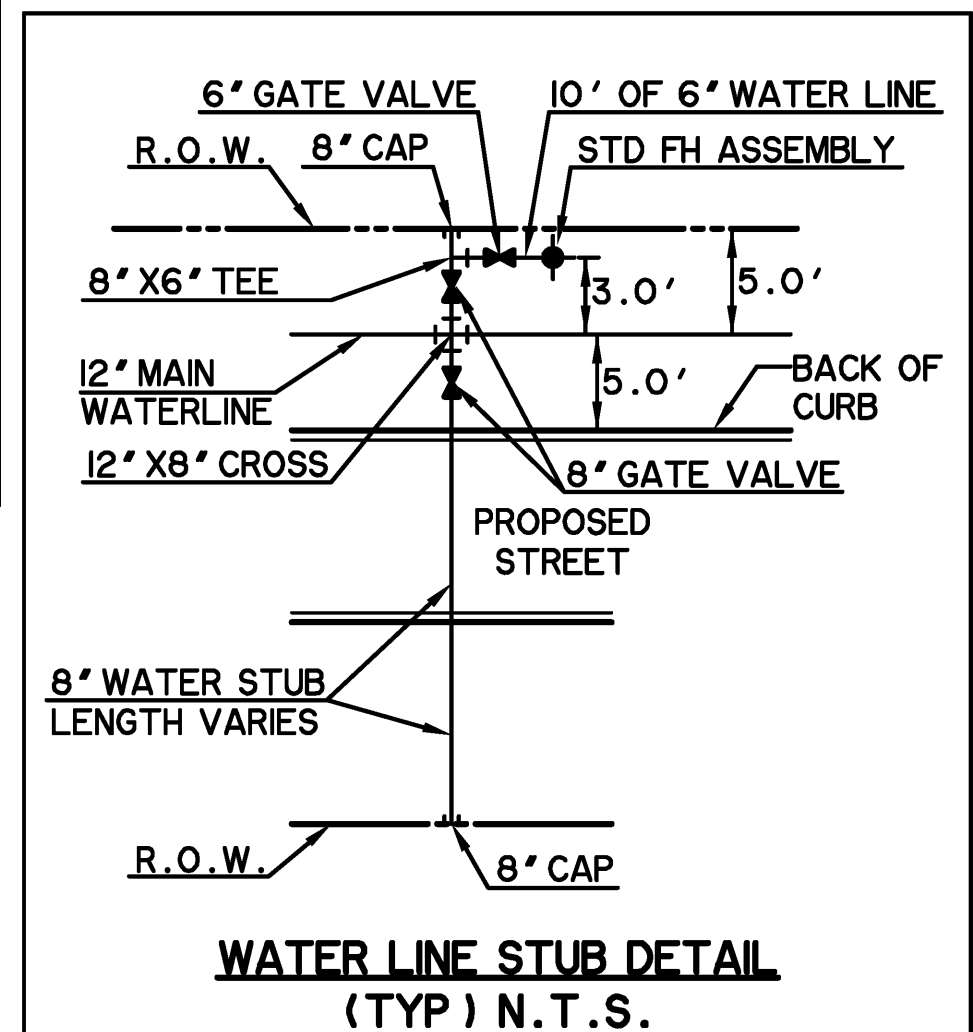


NOTES:

1. CONTRACTOR SHALL ADJUST LOCATION OF PROPOSED WATER LINES AS REQUIRED TO AVOID CONFLICTS WITH STORM SEWER OR OTHER UTILITIES.
2. ALL WATER LINES SHALL HAVE A MINIMUM COVER OF 48" MEASURED FROM TOP OF PIPE UNLESS A GREATER DEPTH IS REQUIRED BY CITY STANDARDS.
3. INSTALL FIRE HYDRANTS 8' FROM BACK OF CURB.

CAUTION !!

EXISTING UTILITIES ARE INDICATED ON THE PLANS FROM AVAILABLE INFORMATION. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE LOCATION OF ALL UTILITIES, TO NOTIFY ALL UTILITY COMPANIES OF THE CONTRACTORS OPERATIONS, TO PROTECT ALL UTILITIES FROM DAMAGE, TO REPAIR ALL UTILITIES DAMAGED DUE TO THE CONTRACTORS OPERATIONS, AND TO NOTIFY THE ENGINEER PROMPTLY OF ALL CONFLICTS OF THE WORK WITH EXISTING UTILITIES.



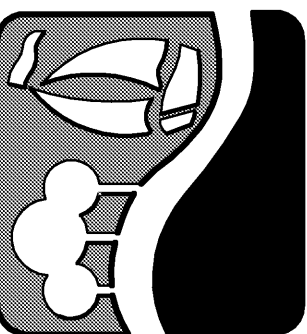
* BENCH MARKS *

- BM A - AN "X" CUT IN THE BACK OF CURB
LOCATED AT THE SOUTH RIGHT-OF-WAY LINE
OF SPRINGER ROAD ±2470' EAST OF THE
INTERSECTION OF SPRINGER ROAD AND F.M.
549. 598.80 FT.
- BM B - AN "X" CUT IN THE BACK OF CURB
LOCATED AT THE NORTH RIGHT-OF-WAY
LINE OF DISCOVERY BOULEVARD ±580' EAST
OF THE INTERSECTION OF DISCOVERY
BOULEVARD AND F.M. 549. 599.82 FT.
- BM C - AN "□" CUT IN DISCOVERY
BOULEVARD IN A MEDIAN NOSE ±60' WEST OF
THE INTERSECTION OF DISCOVERY BOULEVARD
AND F.M. 549. 598.20 FT.

RECORD
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02/02/2015

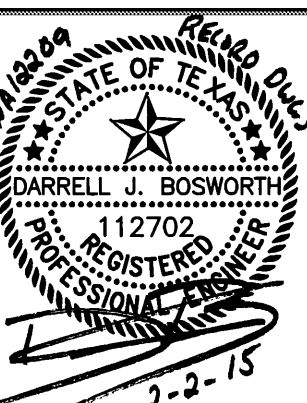
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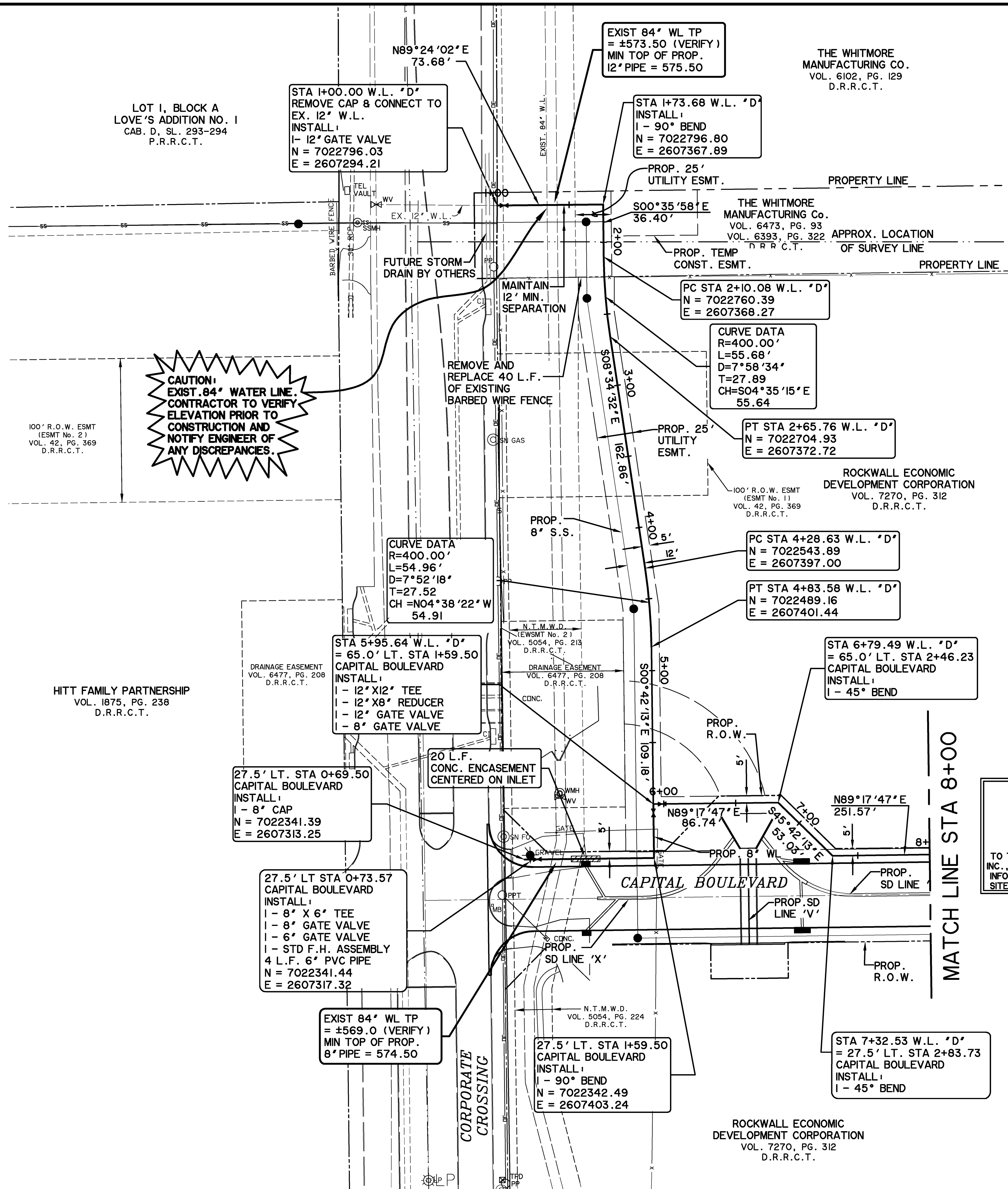
ROCKWALL
TECHNOLOGY
PARK
PHASE IV

WATER LINE "B"
PLAN VIEW
STA 13+99.85 TO END



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SHEET NO.
U101

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NOTES:

1. CONTRACTOR SHALL ADJUST LOCATION OF PROPOSED WATER LINES AS REQUIRED TO AVOID CONFLICTS WITH STORM SEWER OR OTHER UTILITIES.
2. ALL WATER LINES SHALL HAVE A MINIMUM COVER OF 48" MEASURED FROM TOP OF PIPE UNLESS NOTED OTHERWISE OR A GREATER DEPTH IS REQUIRED BY CITY STANDARDS.
3. INSTALL FIRE HYDRANTS 8' FROM BACK OF CURB.

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02/02/2015**

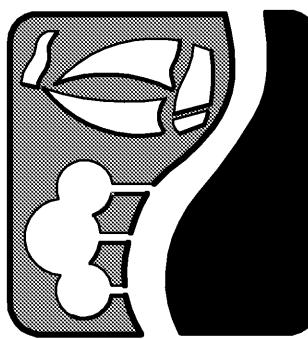
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*** BENCH MARKS ***

BM A - AN "X" CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.

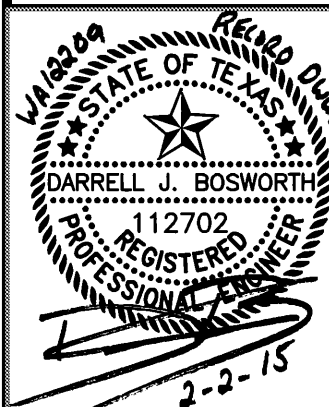
BM B - AN "X" CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.

BM C - AN "X" CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

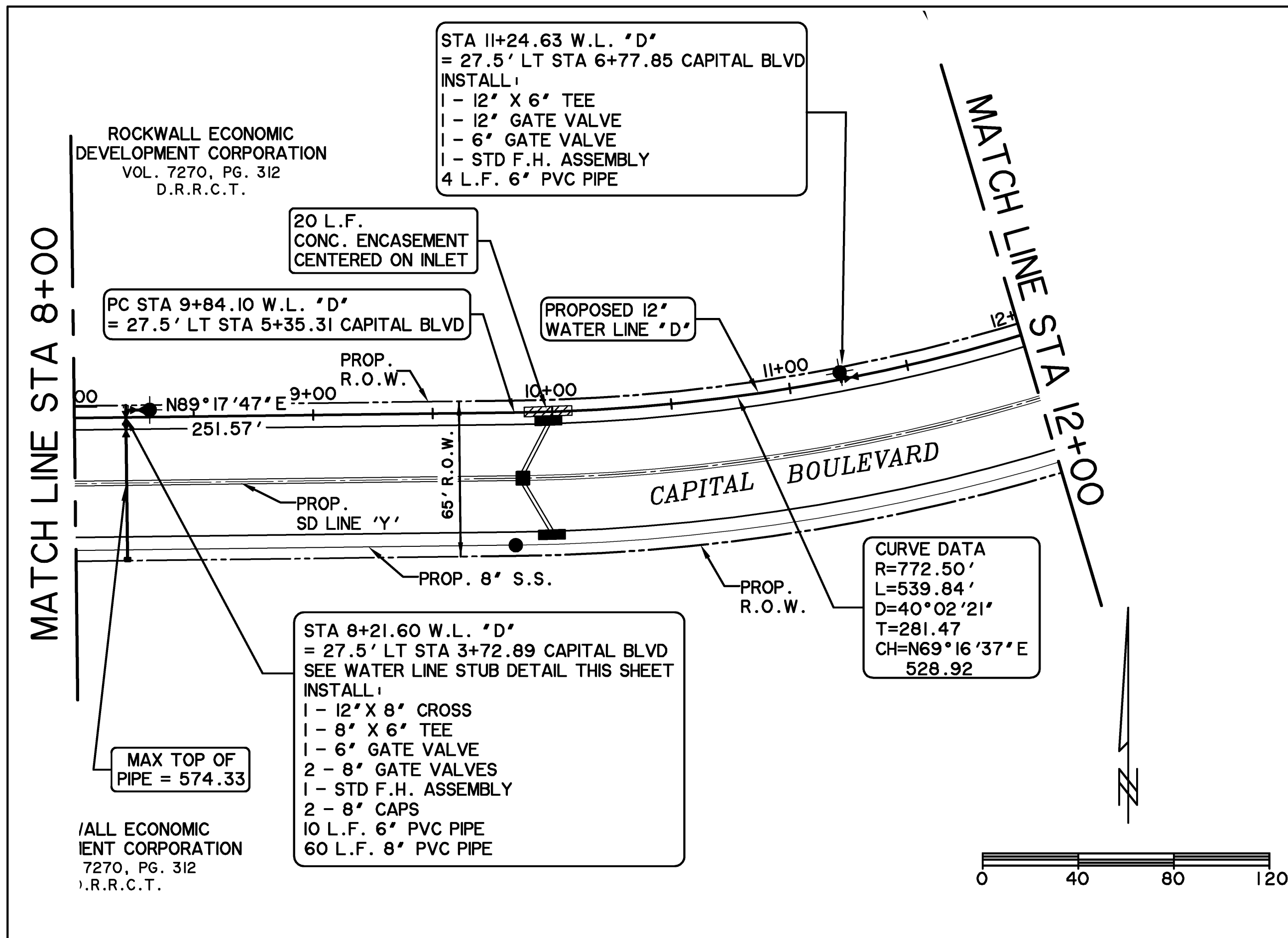
**WATER LINE 'D'
PLAN VIEW
STA 0+00 TO STA 8+00**



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**SHEET NO.
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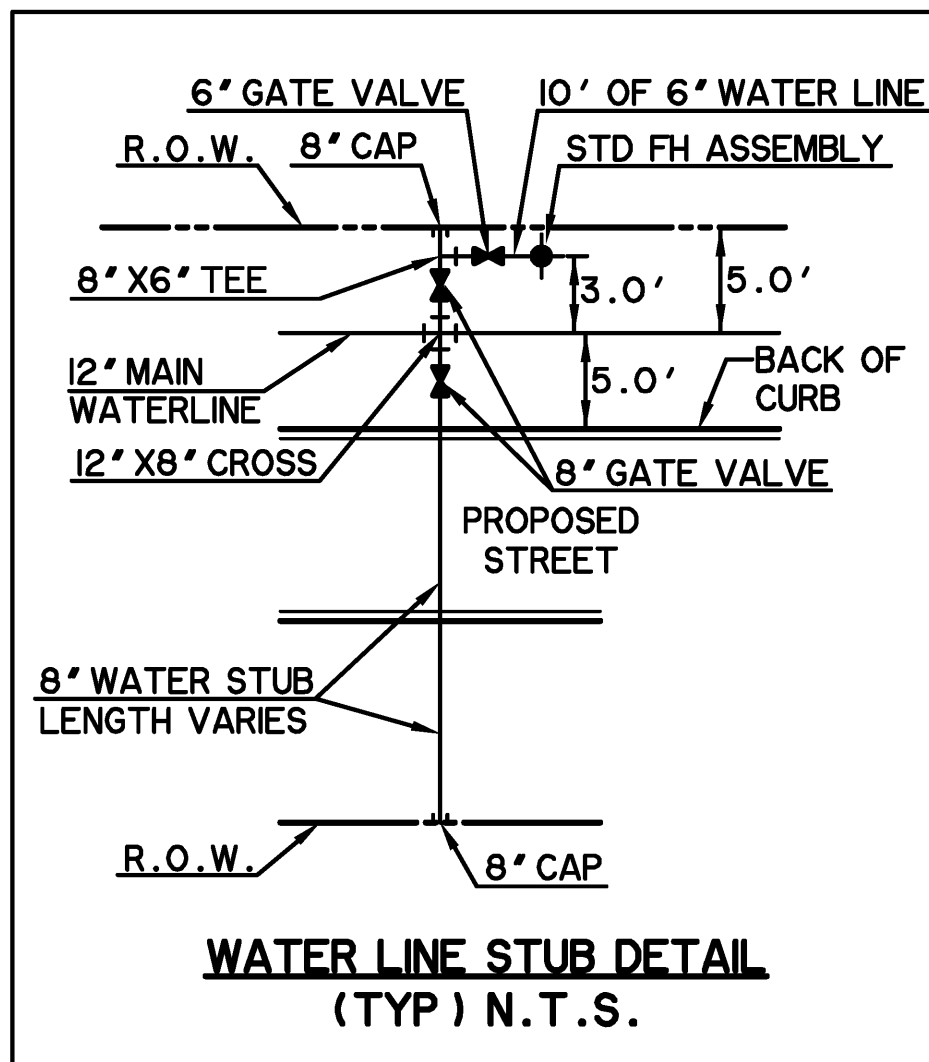


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NOTES:

- CONTRACTOR SHALL ADJUST LOCATION OF PROPOSED WATER LINES AS REQUIRED TO AVOID CONFLICTS WITH STORM SEWER OR OTHER UTILITIES.
- ALL WATER LINES SHALL HAVE A MINIMUM COVER OF 48" MEASURED FROM TOP OF PIPE UNLESS A GREATER DEPTH IS REQUIRED BY CITY STANDARDS.
- ON THE NORTH SIDE OF CAPITAL BOULEVARD, BETWEEN STA 13+50 & 15+50, THE WATER LINE SHALL BE INSTALLED AT A DEPTH THAT WILL PROVIDE AT LEAST 48" OF COVER BOTH NOW AND IN THE FUTURE WHEN THE NORTH SIDE OF CAPITAL BOULEVARD IS CONSTRUCTED. SEE TYPICAL SECTION ON SHEET POOL FOR CAPITAL BOULEVARD.
- INSTALL FIRE HYDRANTS 8' FROM BACK OF CURB.



*** BENCH MARKS ***

BM A - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.

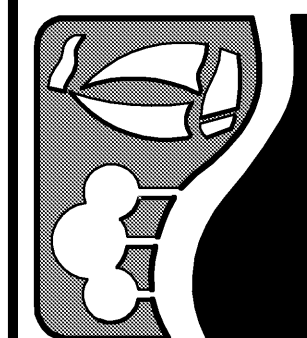
BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.

BM C - AN 'X' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

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02/02/2015**

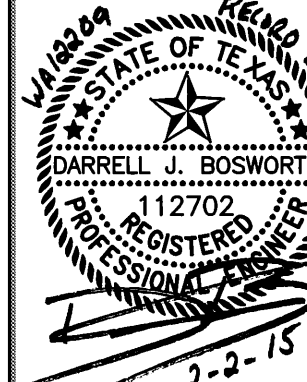
TO THE BEST OF OUR KNOWLEDGE WIER & ASSOCIATES, INC., HERBY STATES THAT THIS PLAN IS AS-BUILT. THIS INFORMATION PROVIDED IS BASED ON SURVEYING AT THE SITE AND INFORMATION PROVIDED BY THE CONTRACTOR.

PREPARED BY:
WIER & ASSOCIATES, INC.
ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
www.WierAssociates.com
Texas Firm Registration No. F-2776



**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

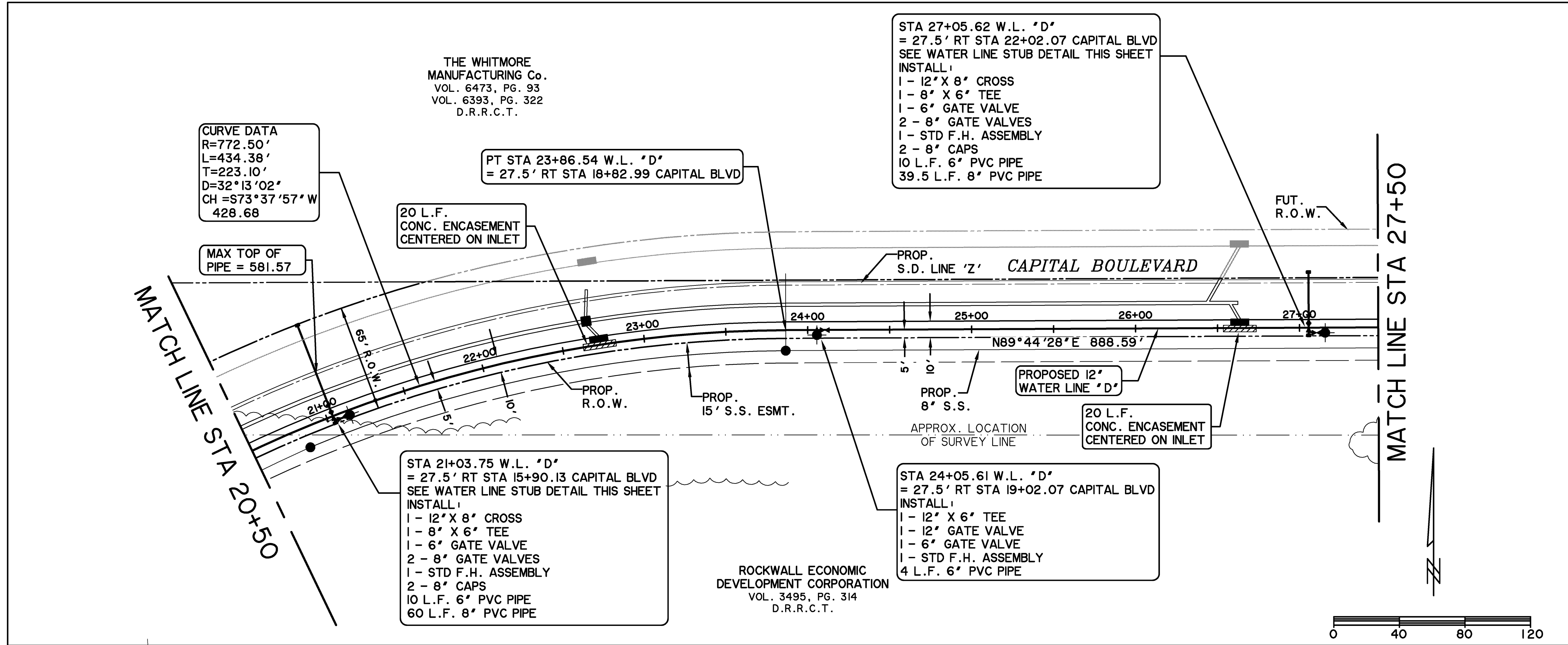
**WATER LINE 'D'
PLAN VIEW
STA 8+00 TO STA 20+50**



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LAST SHEET EDIT
DATE: 10-02-2013
WA# 12209

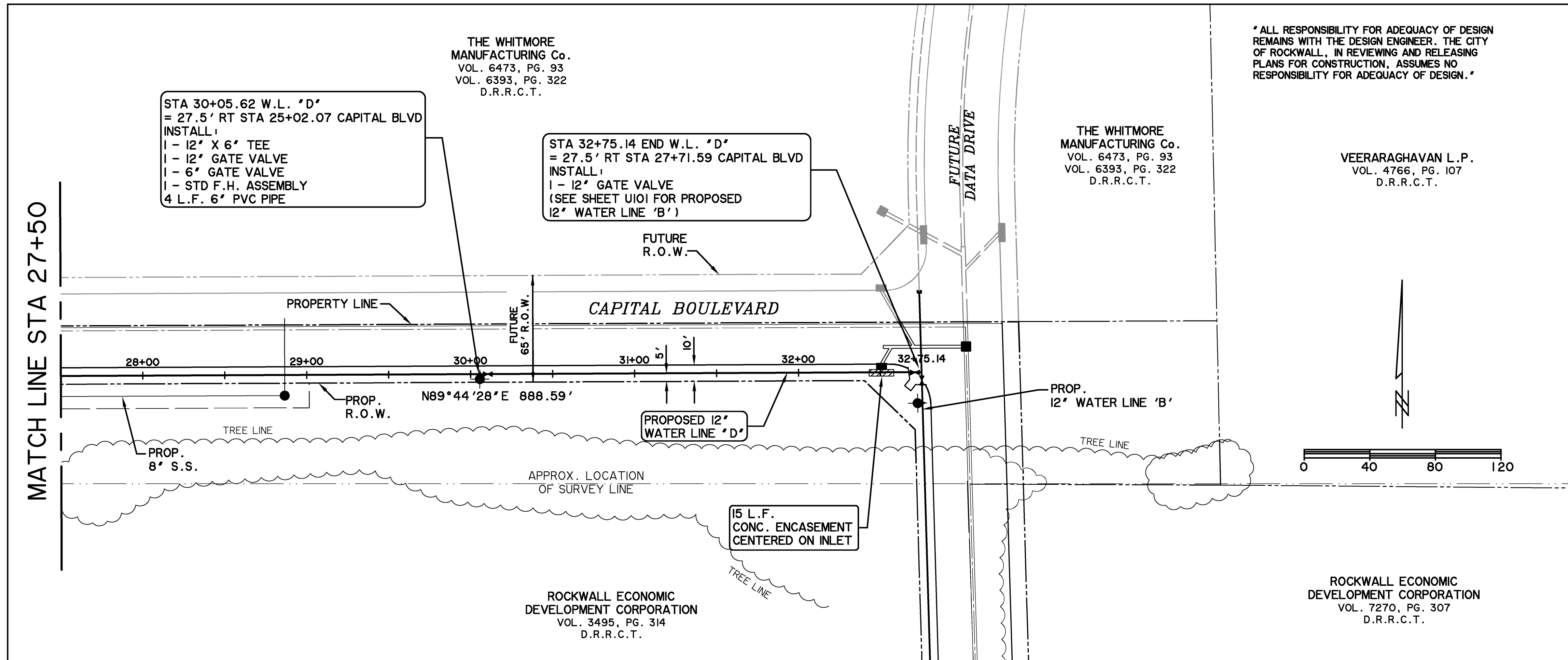
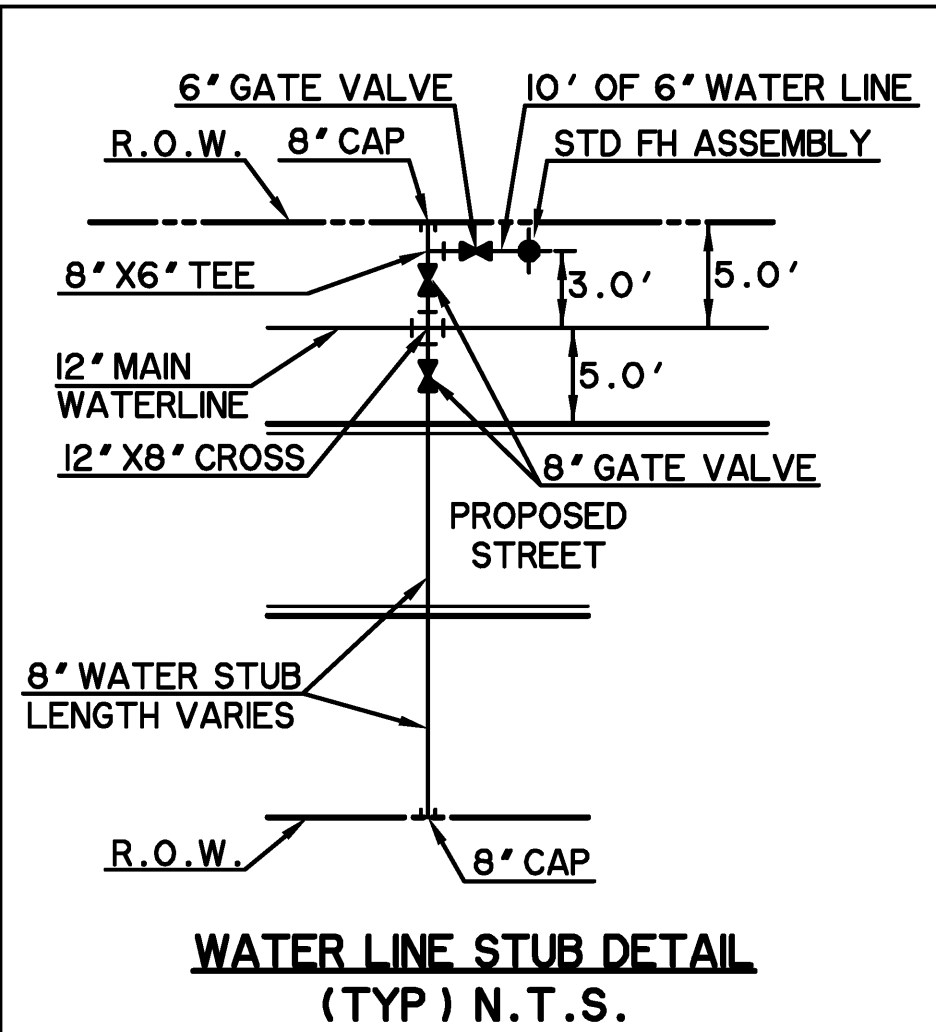
**SHEET NO.
U103**

TIME 12:13
FILE: U104-WATER LINE D-3-12209.dwg



CAUTION !!
EXISTING UTILITIES ARE INDICATED ON THE PLANS FROM AVAILABLE INFORMATION. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE LOCATION OF ALL UTILITIES, TO NOTIFY ALL UTILITY COMPANIES OF THE CONTRACTORS OPERATIONS, TO PROTECT ALL UTILITIES FROM DAMAGE, TO REPAIR ALL UTILITIES DAMAGED DUE TO THE CONTRACTORS OPERATIONS, AND TO NOTIFY THE ENGINEER PROMPTLY OF ALL CONFLICTS OF THE WORK WITH EXISTING UTILITIES.

- NOTES:
- CONTRACTOR SHALL ADJUST LOCATION OF PROPOSED WATER LINES AS REQUIRED TO AVOID CONFLICTS WITH STORM SEWER OR OTHER UTILITIES.
 - ALL WATER LINES SHALL HAVE A MINIMUM COVER OF 48" MEASURED FROM TOP OF PIPE UNLESS A GREATER DEPTH IS REQUIRED BY CITY STANDARDS.
 - INSTALL FIRE HYDRANTS 8' FROM BACK OF CURB.

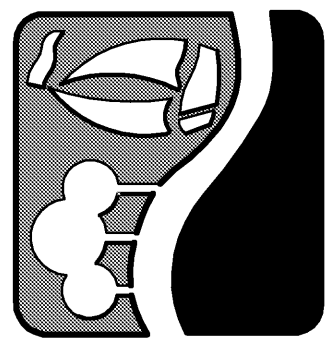


* ALL RESPONSIBILITY FOR ADEQUACY OF DESIGN REMAINS WITH THE DESIGN ENGINEER. THE CITY OF ROCKWALL, IN REVIEWING AND RELEASING PLANS FOR CONSTRUCTION, ASSUMES NO RESPONSIBILITY FOR ADEQUACY OF DESIGN. *

- * BENCH MARKS *
- BM A - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549. 598.80 FT.
- BM B - AN 'X' CUT IN THE BACK OF CURB LOCATED AT THE NORTH RIGHT-OF-WAY LINE OF DISCOVERY BOULEVARD ±580' EAST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 599.82 FT.
- BM C - AN '□' CUT IN DISCOVERY BOULEVARD IN A MEDIAN NOSE ±60' WEST OF THE INTERSECTION OF DISCOVERY BOULEVARD AND F.M. 549. 598.20 FT.

**RECORD
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02/02/2015**

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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**WATER LINE 'D'
PLAN VIEW
STA 20+50 TO END**

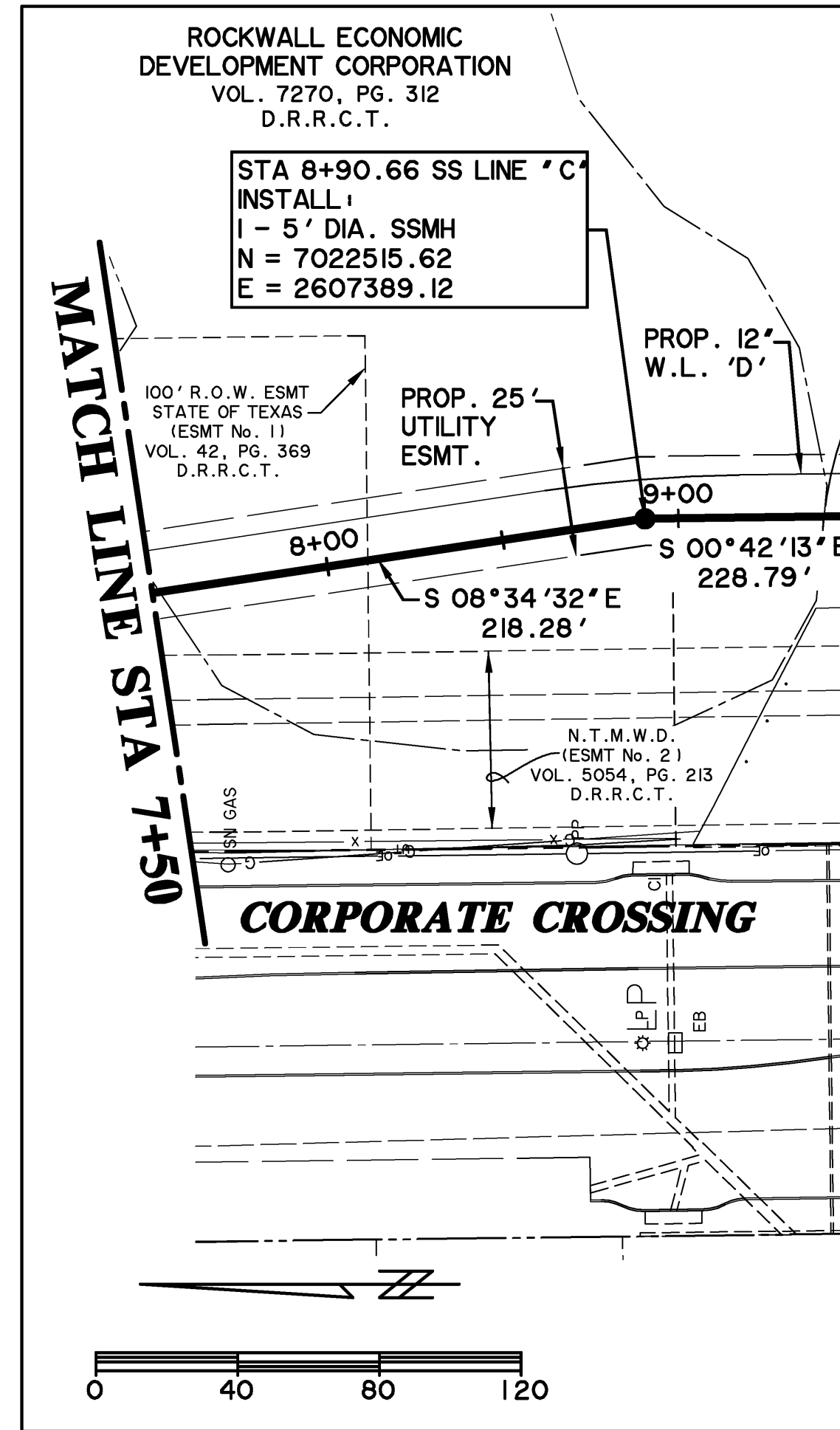
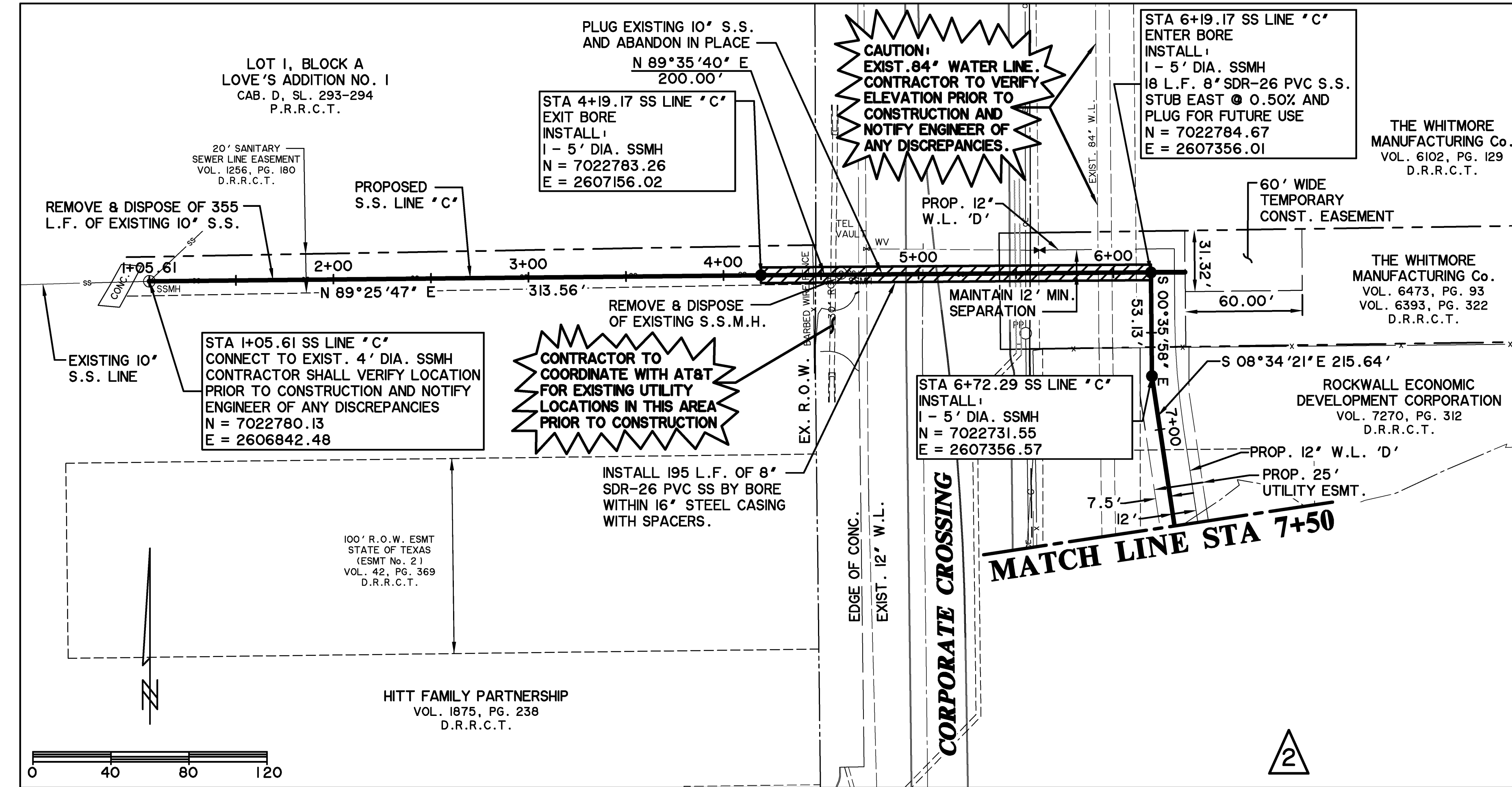


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DATE 10-02-2015
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**SHEET NO.
U104**

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ENGINEERS SURVEYORS LAND PLANNERS
701 HIGHLANDER BLVD., SUITE 300 ARLINGTON, TEXAS 76015 METRO (817)467-7700
Texas Firm Registration No. F-2776 www.WierAssociates.com

TIME 13:25 FILE: U201-SS PLAN PROFILE LINE C-I-12209_REV1.dwg



REVISIONS	
1	4/10/2014: CHANGED S.S.M.H. RIM ELEVATION TO 580.00
2	4/10/2014: REMOVED R FOR 8" STUB TO THE EAST AND ADDED TO S.S.M.H. @ STA 6+19.17

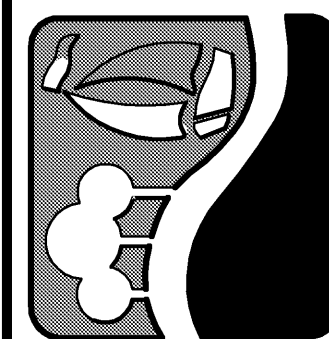
CAUTION 1.1
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BENCH MARKS	
BM A - AN "X" CUT IN THE BACK OF CURB LOCATED AT THE SOUTH RIGHT-OF-WAY LINE OF SPRINGER ROAD ±2470' EAST OF THE INTERSECTION OF SPRINGER ROAD AND F.M. 549.	598.80 FT.
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**RECORD
DRAWING
02/02/2015**

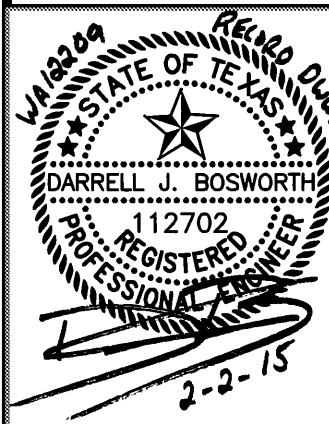
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**ROCKWALL
TECHNOLOGY
PARK
PHASE IV**

**SANITARY SEWER
LINE 'C'
PLAN AND PROFILE
STA 0+00 TO STA 9+50**

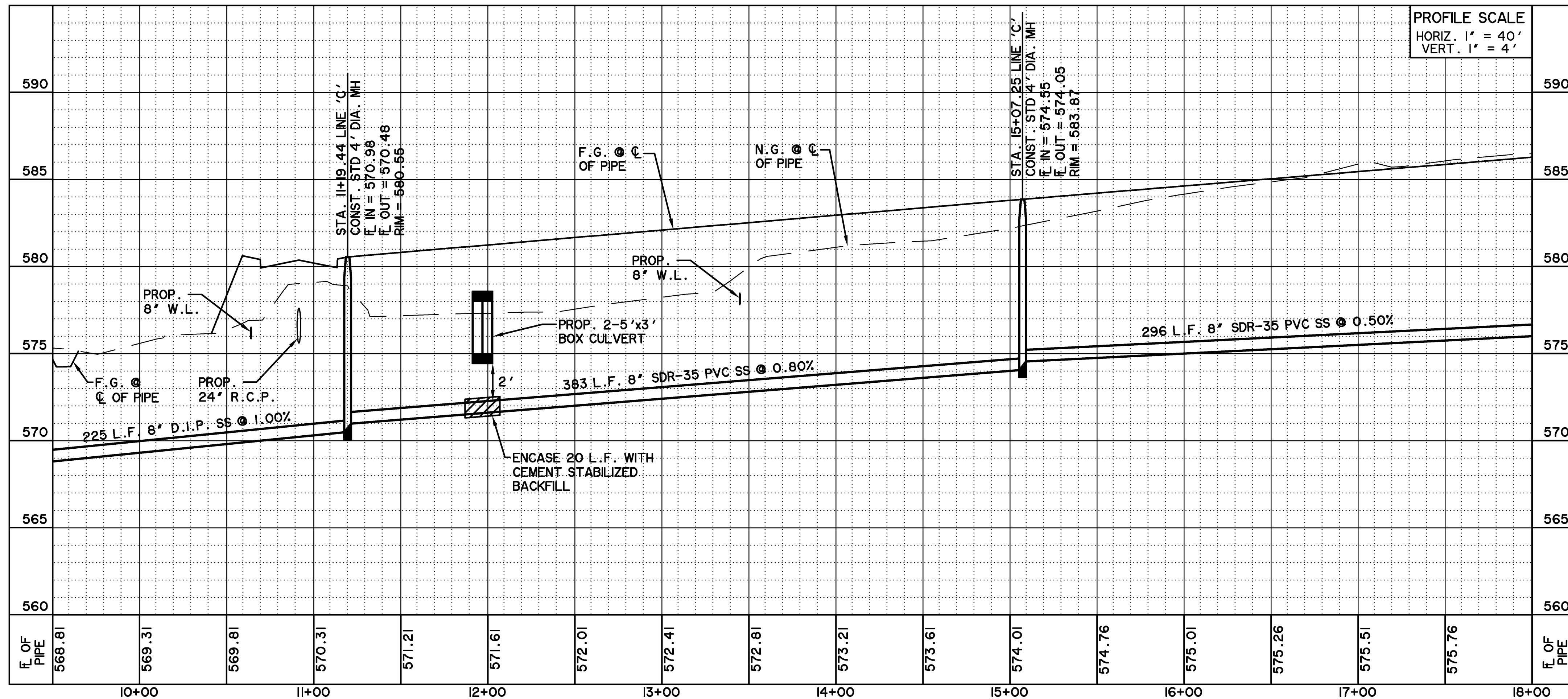
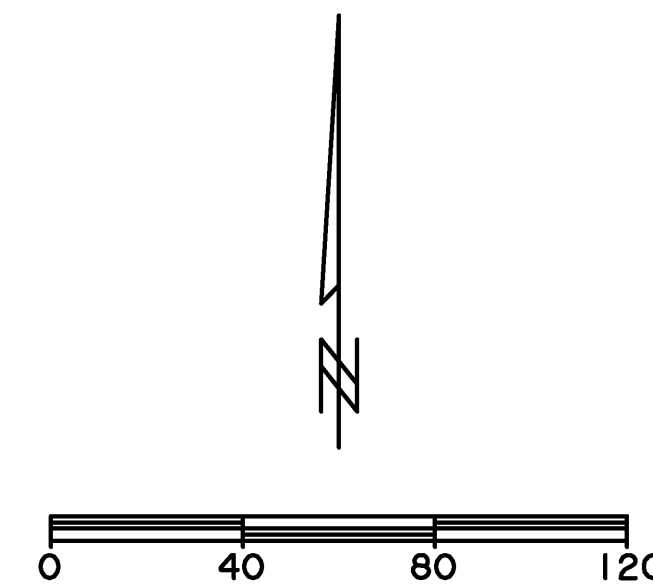
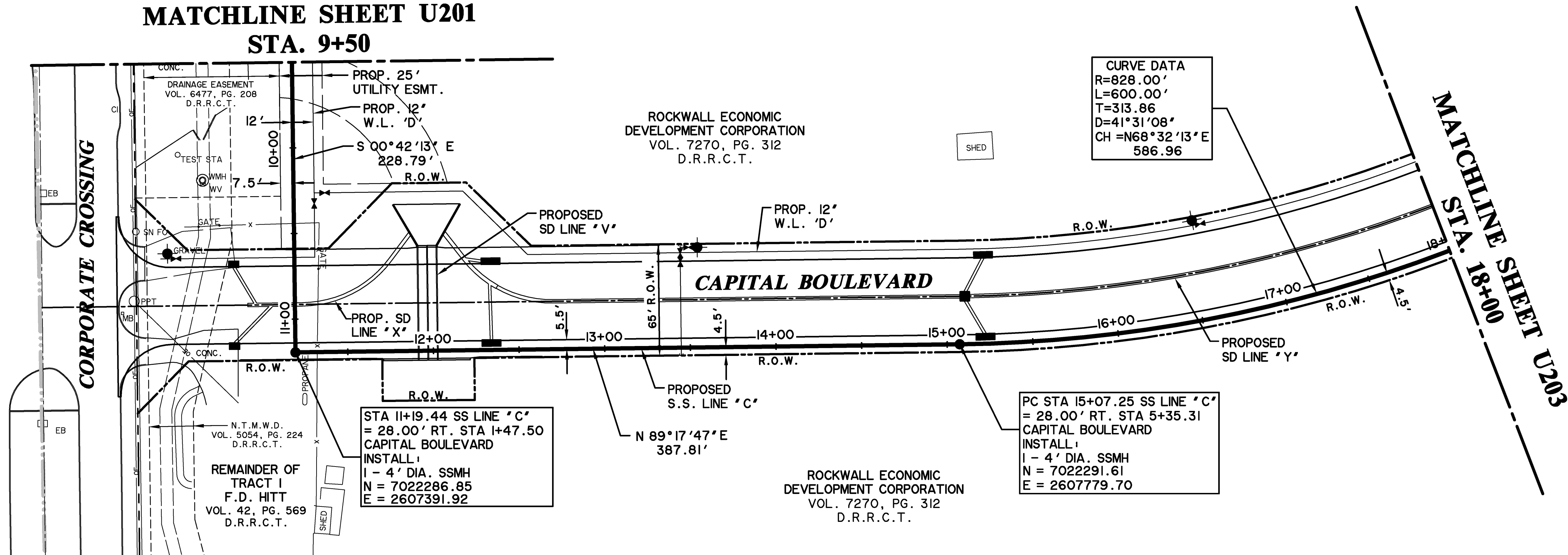


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DATE: 04-10-2014
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**SHEET NO.
U201**

TIME 141.04 FILE 1202-SS PLAN PROFILE LINE C-2-12209.dwg

MATCHLINE SHEET U201

STA. 9+50



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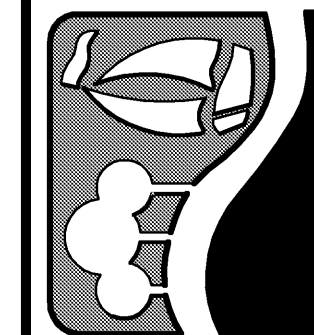
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**RECORD
DRAWING
02/02/2015**

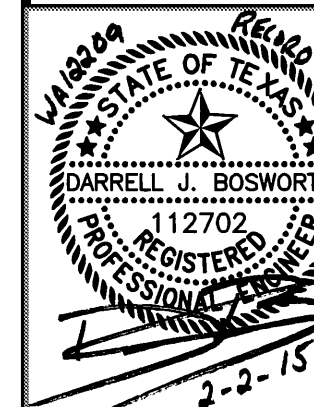
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**ROCKWALL
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PHASE IV**

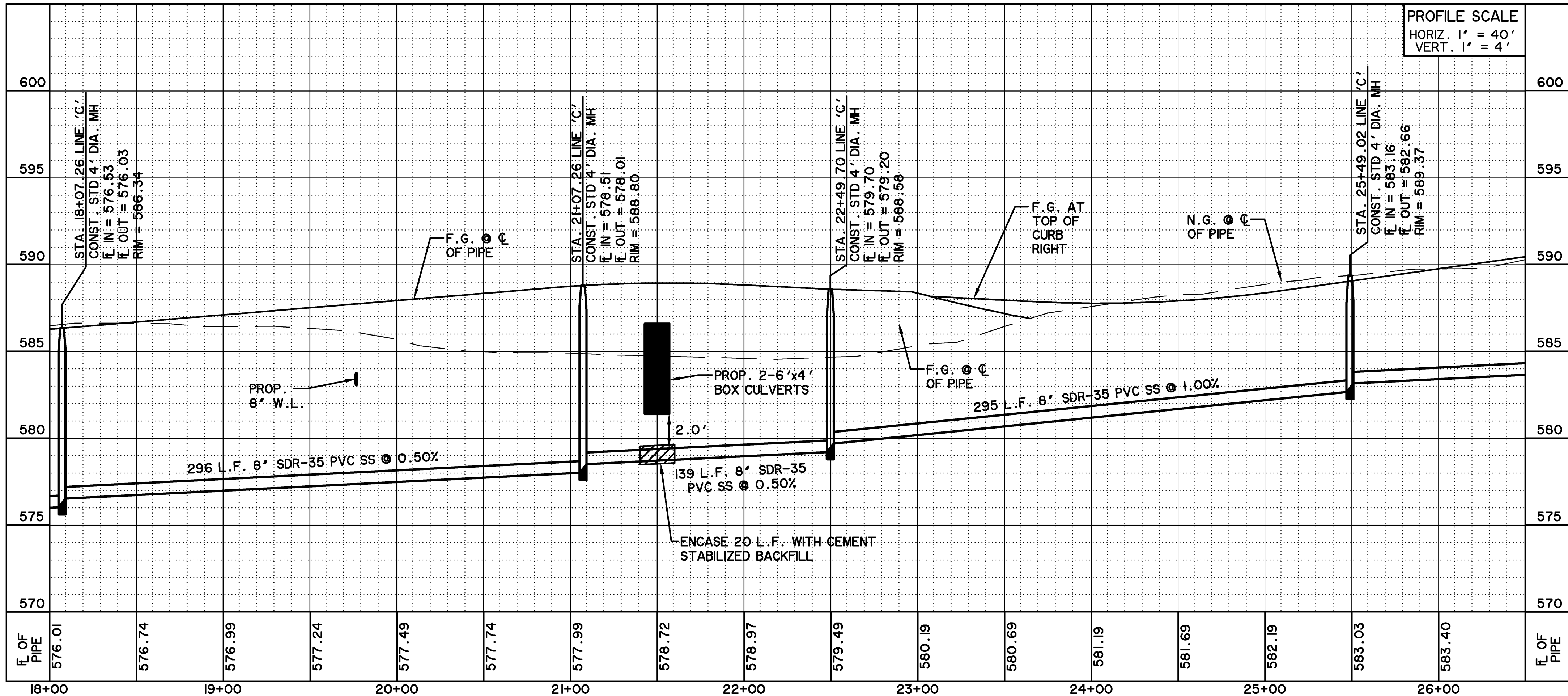
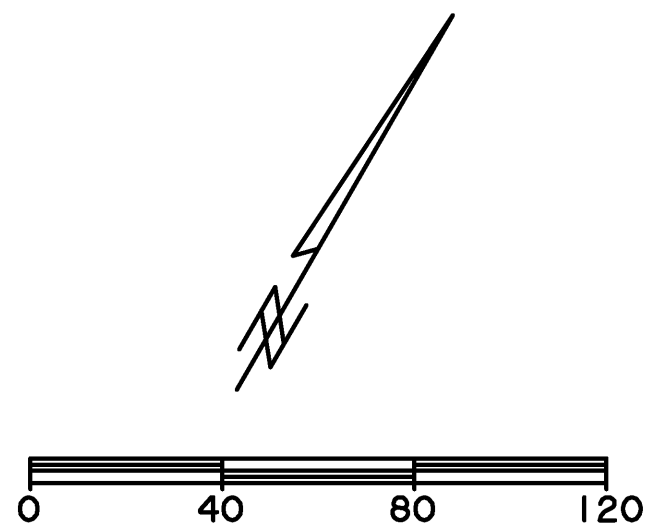
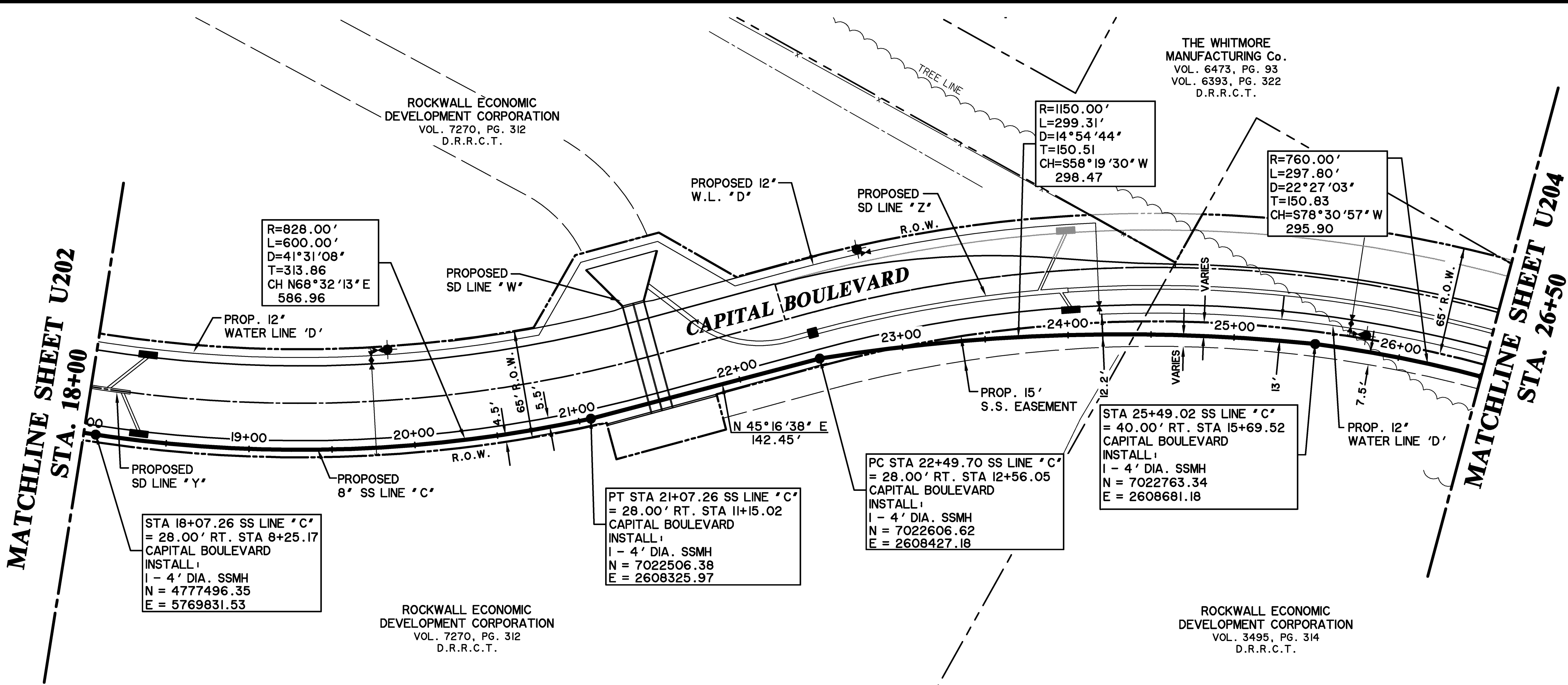
**SANITARY SEWER
LINE 'C'
PLAN AND PROFILE
STA 9+50 TO STA 18+00**



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**SHEET NO.
U202**

TIME 141.08 FILE I:\203-SS PLAN PROFILE LINE C-3-12209.dwg



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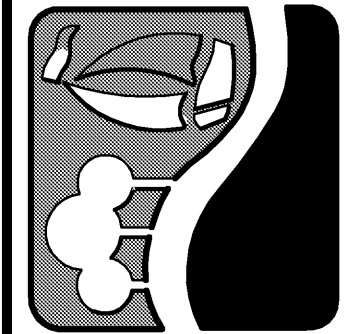
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**RECORD
DRAWING
02/02/2015**

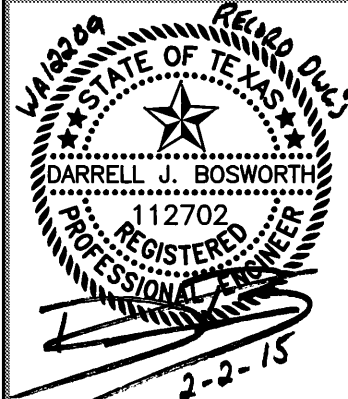
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www.WierAssociates.com
Texas Firm Registration No. F-2776



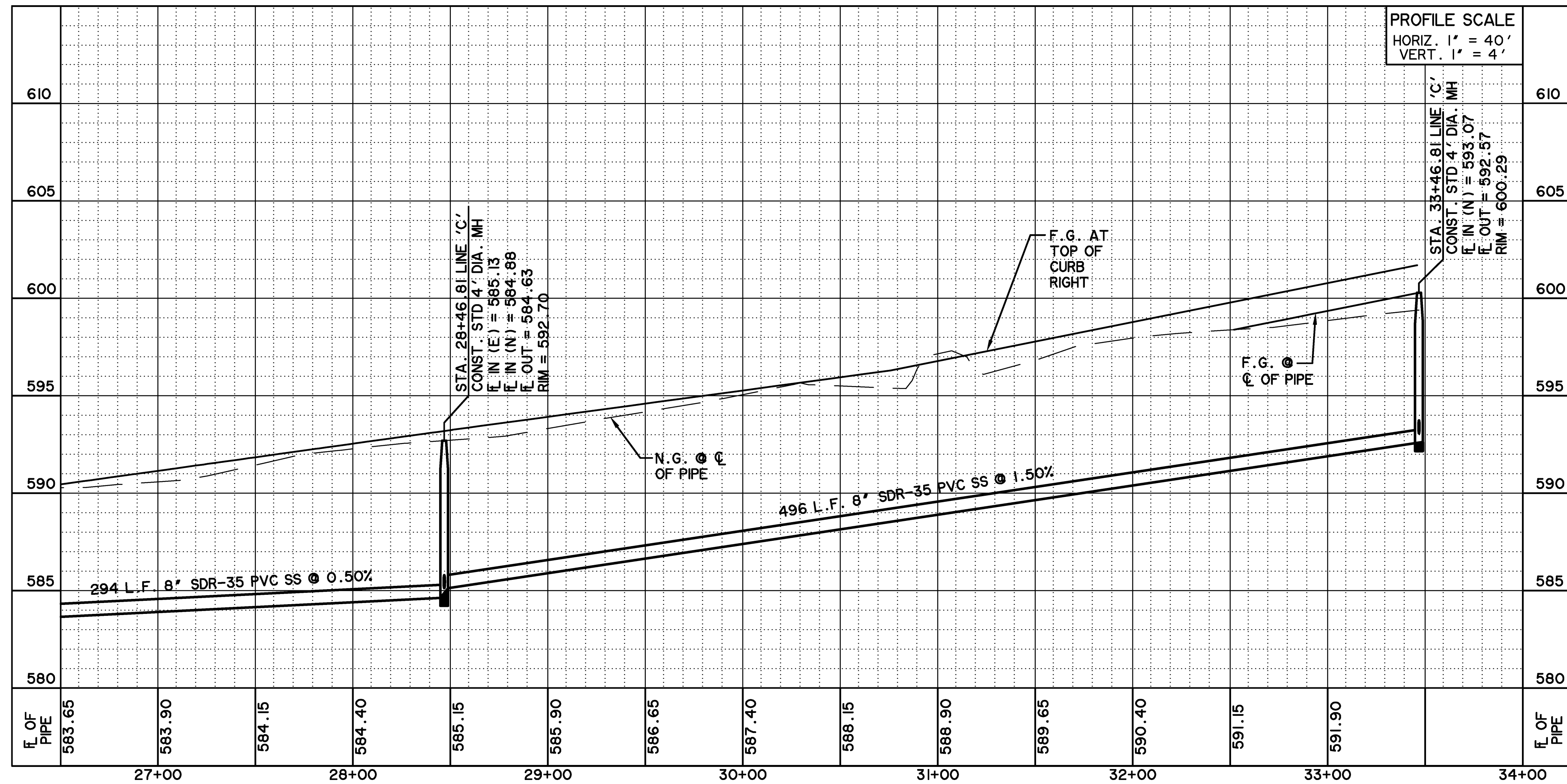
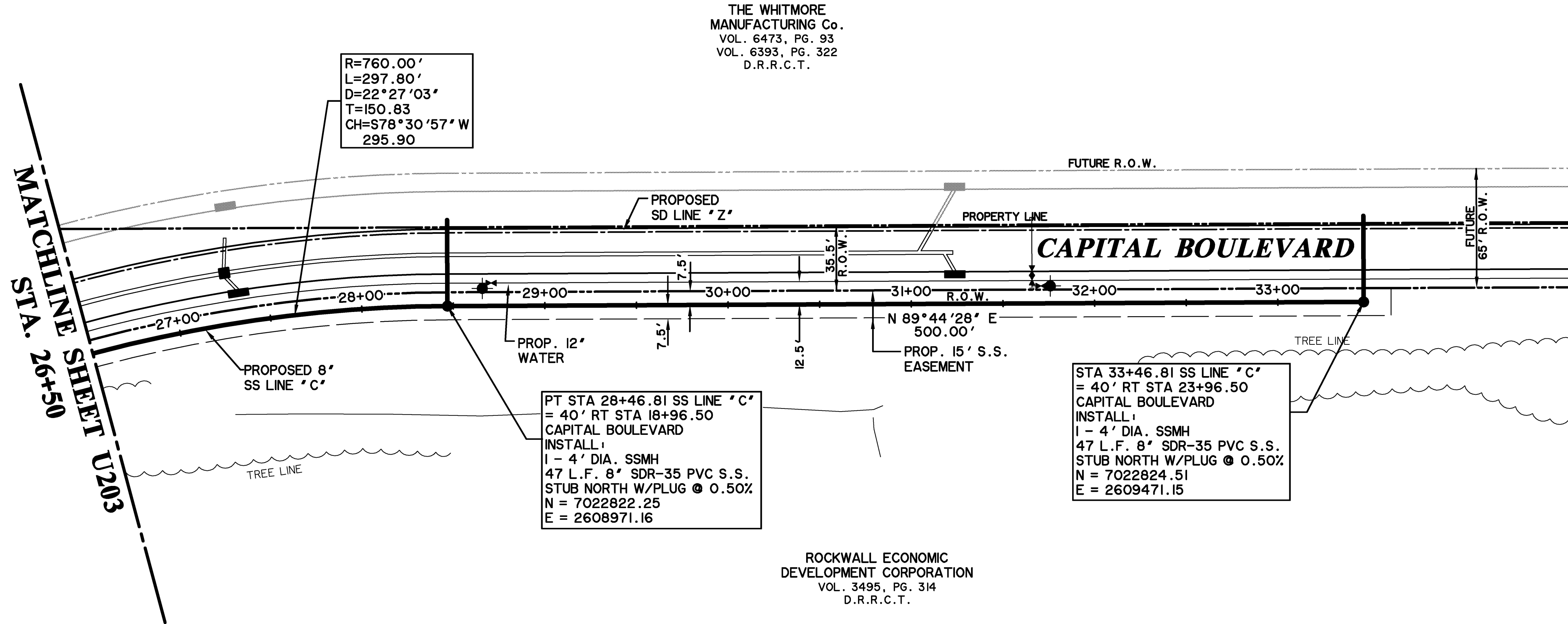
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**SANITARY SEWER
LINE 'C'
PLAN AND PROFILE
STA 18+00 TO STA 26+50**



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**SHEET NO.
U203**

TIME 14:12 FILE: I204-SS PLAN PROFILE LINE C-4-12209.dwg



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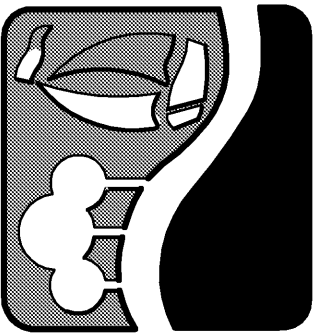
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**SANITARY SEWER
LINE 'C'
PLAN AND PROFILE
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